Preliminary Engineering Report

for

Water System Improvements Project Village of Wellsville, Allegany County, New York

January 2017 HUNT 1861-034



Prepared by:
Hunt Engineers, Architects & Land Surveyors, PC
Airport Corporate Park
100 Hunt Center
Horseheads, NY 14845-1019
Phone (607) 358-1000, Fax (607) 358-1800

Preliminary Engineering Report

For

Water System Improvements Project Village of Wellsville, Allegany County, New York

HUNT 1861-034

January 2017

TABLE OF CONTENTS

I.	Background and Introduction	5
II.	Project Planning	5
A.	Location	5
B.	Environmental Resources Present	5
C.	Population Trends	6
D.	Community Engagement	8
III.	Existing Water System	8
A.	History	8
B.	Service Area	8
C.	Water Supply, Demands and Unaccounted-for-Water	9
D.	Water Distribution and Transmission	. 12
E.	Finished Water Storage	
F.	System Operations	. 19
G.		
Н.	Equivalent Dwelling Units	. 20
IV.	Regulatory Compliance	.21
Α.	Water Source and Treatment	
В.		
C.		
V.	Asset Management	
Α.		
B.	Asset Management Plan	. 23
VI.	Financial Status of Existing Facilities	
Α.		
B.	Water Fund Revenues	. 25
C.	Current Water Debt	. 25
D.	Proposed Water Rate with Asset Management	. 26
E.		
F.	Rate Maintenance	. 27
VII.	Leak Detection and Meter Testing	. 28
Α.	Review of System Leak Repairs	. 28
B.	Leak Detection Survey	. 28
C.		
VIII.	Need for the Project	
A.	Health, Sanitation, and Security	. 33
B.	Aging Infrastructure	
C.	Reasonable Growth	. 33
IX.	Proposed Water System Improvement Alternatives	.34
A.		.34
В.	· · · · · · · · · · · · · · · · · · ·	
C.	Distribution System	.36
Χ.	Proposed Project (Recommended Alternative)	. 52

Figure 2: Treatment plant discharge data from January 2005 to December 2015. Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015. Figure 4: Water distribution system, pipe material. Figure 5: Water distribution system, composition by pipe diameter. Figure 6: Composition of Wellsville Water Customer, March 2015-2016. Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016. Figure 8: Village billing for Argenteiri Laundry.	52 52 54 55 55 10
D. Existing Cost to Customers E. Annual Operating Budget F. Review of Funding Opportunities XI. Conclusions and Recommendations Figure 1: Population data for the Village of Wellsville Figure 2: Treatment plant discharge data from January 2005 to December 2015 Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015 Figure 4: Water distribution system, pipe material Figure 5: Water distribution system, composition by pipe diameter Figure 6: Composition of Wellsville Water Customer, March 2015-2016 Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016. Figure 8: Village billing for Argenteiri Laundry	53 54 55 7 10 15 15 15
D. Existing Cost to Customers E. Annual Operating Budget F. Review of Funding Opportunities XI. Conclusions and Recommendations FIGURES Figure 1: Population data for the Village of Wellsville. Figure 2: Treatment plant discharge data from January 2005 to December 2015. Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015. Figure 4: Water distribution system, pipe material. Figure 5: Water distribution system, composition by pipe diameter. Figure 6: Composition of Wellsville Water Customer, March 2015-2016. Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016. Figure 8: Village billing for Argenteiri Laundry.	53 54 55 7 10 15 15 15
F. Review of Funding Opportunities XI. Conclusions and Recommendations	54 55 10 12 15 21 25 30
Figure 1: Population data for the Village of Wellsville. Figure 2: Treatment plant discharge data from January 2005 to December 2015. Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015. Figure 4: Water distribution system, pipe material. Figure 5: Water distribution system, composition by pipe diameter. Figure 6: Composition of Wellsville Water Customer, March 2015-2016. Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016. Figure 8: Village billing for Argenteiri Laundry.	55 10 12 14 15 21 25
Figure 1: Population data for the Village of Wellsville	7 12 14 15 21
Figure 1: Population data for the Village of Wellsville	10 12 15 21 25
Figure 1: Population data for the Village of Wellsville	10 14 15 21 25
Figure 1: Population data for the Village of Wellsville	10 12 15 21 25
Figure 2: Treatment plant discharge data from January 2005 to December 2015. Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015. Figure 4: Water distribution system, pipe material. Figure 5: Water distribution system, composition by pipe diameter. Figure 6: Composition of Wellsville Water Customer, March 2015-2016. Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016. Figure 8: Village billing for Argenteiri Laundry.	10 14 15 21 25
Figure 3: Village of Wellsville unaccounted for water, 2005 – 2015	12 14 15 21 25
Figure 4: Water distribution system, pipe material. Figure 5: Water distribution system, composition by pipe diameter Figure 6: Composition of Wellsville Water Customer, March 2015-2016 Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016 Figure 8: Village billing for Argenteiri Laundry	14 15 21 25
Figure 5: Water distribution system, composition by pipe diameter	15 21 25 30
Figure 6: Composition of Wellsville Water Customer, March 2015-2016	21 25 30
Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016	25 30
Figure 8: Village billing for Argenteiri Laundry	30
	31
Figure 9: Village billing for Jones Hospital	
<u>TABLES</u>	
Table 1: Median Household Income	7
Table 2: Annual water production, consumption, and unaccounted for water.	
Table 3: Water distribution system material and length.	
Table 4: Water distribution system, pipe diameter.	
Table 5: Fire flows measured by Insurance Services Office, 1997.	
Table 6: Pressure fluctuations for pumps on and off	
Table 7: Characterization of Wellsville Water District Users, March 2015-2016	
Table 8: Village of Wellsville Water Rate Structure, 1999-2002.	
Table 9: Village of Wellsville Water Rate Structure, 2003-2010.	
Table 10: Village of Wellsville Water Rate Structure, 2011-2015.	
Table 11: Village of Wellsville Water Rate Structure, 2015-2016.	
Table 12: Village of Wellsville Water Fund Bond Expense, 2014-2015	
Table 13: O&M costs with proposed rate structure increase to base meter charge only	
Table 14: Proposed rate structure with revisions to entire rate structure	
Table 15: O&M costs with proposed rate structure increase to entire rate	
Table 16: Wellsville Water System, Estimated Volume of Leaks, 2013 – 2016	
Table 17: Village of Wellsville, NYLD Leak Detection Summary, 2016	
Table 18: Village of Wellsville, large meter flow testing, 2016.	
Table 19: Watermain replacement construction cost for locations with tanbark	
Table 20: Transite asbestos pipe replacement costs for the Village of Wellsville	

Appendix A	A: Project	Location I	Марр	ing
------------	------------	------------	------	-----

Appendix A: Project Location Mapping
Appendix B: Environmental Resources
Appendix C: Hydraulic Model Narrative & Input Data
Appendix D: Tanbark Location Map
Appendix E: Hydrant Fire Flows
Appendix F: Potential Problem Areas Map
Appendix G: Water Storage Tank Inspection Report
Appendix H: Static Pressure Mapping

Appendix I: Fire Flow Mapping

Appendix J: Allegany County DOH 2015 Sanitary Survey Appendix K: Asset Management

Appendix L: Village Budget and Water Rates

Appendix M: Leak Repair Reports
Appendix N: NYLD Leak Detection Survey Appendix O: Water Meter Testing Reports

Appendix P: Cost Estimates

Appendix Q: Project Improvements Map

I. Background and Introduction

The Village of Wellsville, located in the Genesee River Valley in the southern part of Allegany County, New York, currently owns and operates a public water system (herein referred to as the Water District) to serve the Village of Wellsville and several out of district users. Given the age and condition of the system as characterized by the percentage of unaccounted for water, the Village has expressed interest in completing a comprehensive evaluation of their water system, developing a water system asset management plan, and identifying and quantifying potential improvements with the intent to improve their current water system based on the findings.

The purpose of this report is to describe the existing system conditions and identify any shortcomings, evaluate various alternatives to address the shortcomings, and provide recommendations for improvements based on monetary and non-monetary factors.

II. Project Planning

A. Location

The Village of Wellsville is approximately 2.4 square miles in area and is primarily a rural community. New York State Route 417 bisects the Village in an easterly to westerly direction while New York State Route 19 bisects the Village in a northerly to southerly direction. Please refer to Appendix A for a Project Location Map.

B. Environmental Resources Present

The following section describes, in brief, various environmental resources that have been located in or around the Water District. Please refer to Appendix B for associated mapping.

1. Agricultural Districts

Review of the Allegany County Agricultural District Mapping shows that the Village of Wellsville water system is not located within an agricultural district. The Village is located within proximity of Agricultural District 2 and 4. However, any proposed improvements to the existing water system would not impact existing agricultural lands.

2. Water Bodies

The Village of Wellsville is bisected by the Genesee River, a Class C water body, from which they receive their water supply. Several Class C streams, brooks, and creeks flow into the Genesee River within the Village limits.

Impacts from any proposed improvements to surface waters would be mitigated through erosion and sediment control practices during construction so as to protect the Village's water source.

3. Floodway and Floodplains

As demonstrated on the attached National Flood Insurance Program's Flood Insurance Rate Map (FIRM) of the Village of Wellsville, portions of the Village along the Genesee River and several creeks are within the 100 year floodplain. The Village's water treatment plant is located within the 100 year floodplain.

4. Wetlands

The New York State Department of Environmental Conservation (NYSDEC) mapping was reviewed regarding potential state designated wetlands in and around the Village of

Wellsville. As demonstrated in the attached mapping, there are no wetlands designated by New York State within the existing system service area.

Review of the United States Fish and Wildlife Service National Wetlands Inventory mapping shows several federally regulated freshwater emergent wetlands and forested shrub wetlands within the Village limits. The wetlands present are within the Genesee River corridor and are not encroached upon by existing water system infrastructure.

5. Subsurface Conditions

Utilizing the National Resources Conservation Service online soil survey tool, the predominant soil types throughout the Water District area include: Chenango Series, Mardin Series, and Valois Series. Soil drainage within the Water District is predominately characterized by moderately well-drained to well-drained with areas of poor drainage. Soil mapping of the Water District is attached along with descriptions of the various soil types.

Groundwater depth within the project area varies. The presence of groundwater throughout the water system area is influenced by the Genesee River that bisects the Village.

6. Endangered or Threatened Species

Review of the NYSDEC environmental resources mapping, which incorporates the New York National Heritage Program Information, shows the Water District to be free of rare plant and wildlife species, and other environmental resources considered by the NYSDEC.

The United States Fish and Wildlife Service's Information for Planning and Conservation (IPaC) database was reviewed to determine that one species of bat, the northern long-eared bat, is an endangered species that may occur or could potentially be affected by activities in the Village of Wellsville. Habitat for the species consists of hibernation in caves in the winter and then migration to wooded areas during the summer. The northern long-eared bat will roost underneath bark, in cavities or crevices of both live and dead trees. Any projects within the Village of Wellsville will need to be evaluated for their potential to impact this endangered species. Projects shall incorporate measures, such as ensuring that the species habitat is not destroyed, in order to prevent impacts to the northern long-eared bat. Given the bat's habitat, it is unlikely that improvements to the existing water system infrastructure will impact the bat, as existing infrastructure is located within paved roads and in areas that receive routine disturbance and maintenance.

7. Archeological Sensitivity

Review of Office of Parks, Recreation, and Historic Preservation (OPRHP) and State Historical Preservation Office (SHPO) archeological mapping shows the southern portion of the Water District to be in an archeologically sensitive zone. New improvements to be constructed within these areas will likely require further investigation to determine the sensitivity of the areas.

C. Population Trends

The Village of Wellsville currently has an estimated population of 4,621 (2014 ACS 5-year population estimate). Using U.S. Census Data, the Village has experienced a notable decline in population since 1950. In 1950, the population was estimated to be 6,402 persons and has steadily declined to 4,679 persons, as recently documented in the 2010 U.S Census. This population trend can be graphically shown as follows:

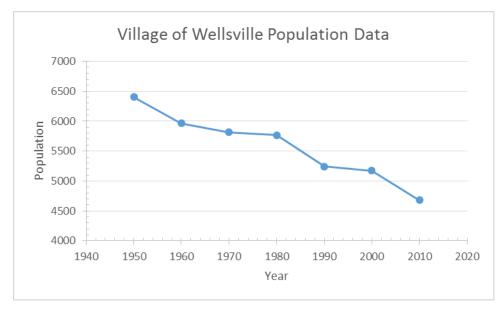


Figure 1: Population data for the Village of Wellsville.

Utilizing the downward trend, the projected population growth for the next two decades is 4,531 in 2020 and 4,296 in 2030.

The Village of Wellsville has a median household income (MHI) of \$39,792, which is below the New York State MHI of \$58,687.

Table 1: Median Household Income.

Place	Median Household Income (2010-2014 5-year ACS)
Village of Wellsville	\$39,792
Allegany County	\$42,726
New York State	\$58,687
United States	\$53,482

The Village's median income is approximately 25% lower than the United States median household income of \$53,482 and 32% lower than the New York State median household income of \$58,687.

A declining population coupled with an aging water system can result in one of the following outcomes:

- 1. Increased rates to compensate for the reduction in water consumption to provide necessary maintenance and upgrades to the water system.
- 2. Maintain relatively flat rates during the population decline resulting in a decrease in water revenue needed to complete maintenance and upgrades to the water system.

D. Community Engagement

HUNT, in conjunction with the Village of Wellsville, shall host public information meetings to openly discuss the project with the residents of the Village and out of district users to address any questions or comments the public may have. The Village of Wellsville shall notify the public of the water system evaluation progression through newsletters or bulletins.

III. Existing Water System

A hydraulic model of the existing water system was developed. Refer to Appendix C for the hydraulic model narrative.

A. History

The Village of Wellsville water system was purchased in 1915 from the Wellsville Water Company. The water system was originally created and owned by the Wellsville Water Company until Village residents became dissatisfied with the company and the Village voted to purchase the company. This water system relied on a reservoir built on Crowner Creek and nine wells near the bank of the Genesee River. In 1916, a slow sand filtration plant was constructed, but did not perform as expected as the Genesee River was too muddy for the plant. A 3 million gallon open concrete reservoir was built in 1916 on Lee Place as part of the slow sand filter project. Due to the poor performance of the slow sand filter, it was abandoned and a new plant was constructed with a rapid sand mechanical filter in 1921. In 1929, a concrete intake dam was constructed that supplied water to a power house and the Village water treatment plant. In 1948, a 750,000 gallon steel tank reservoir was constructed to increase the Village's storage capacity. In 1956, the U.S. Army Corps of engineers conducted a flood control project on the Genesee River through Wellsville, which was later redesigned to increase the flood control capacity. This required the Village to move their existing intake. Pollution of the river was discovered in 1981 that resulted in placement on the Superfund list with high priority due to the threat to the water supply. The company found to be at fault for polluting the river built a new intake, pump station, and transmission line upstream of the polluted section of the river. The new intake, pump station, and transmission line were dedicated to the Village and the existing infrastructure was kept as an emergency backup. The rapid sand filter plant was replaced in 1990 with a new treatment plant that is currently used. In 1997, the 750,000 gallon steel storage tank was removed from service and demolished due to structural damage. In 2001, two 2 million gallon concrete tanks were constructed and the open concrete tank was removed from service.

B. Service Area

The water distribution system includes the entire Village of Wellsville. According to the Village's annual water report, the system services approximately 5,700 people. The Village also serves a number of customers outside of the Village corporate limits that are within the Town of Wellsville. There are seven (7) areas within the Town that are served by the Village of Wellsville water system: the airport, along Morningside Drive, along George Street, along Riverside Drive, to Alfred State College along Tower Drive, along East State Street, and the extension along New York State Route 417. It should be noted that out of district water users cannot legally be served unless a water district is formed including the users outside of the Village, or individual agreements are established between the Village and each user. The Village currently has established agreements with the Town to serve these out of district areas.

C. Water Supply, Demands and Unaccounted-for-Water

1. Genesee River

The primary water supply source for the Village of Wellsville consists of the Genesee River that flows up from Pennsylvania and the rural areas of New York to the south. The Genesee River bisects the Village of Wellsville in a northwesterly to southeasterly direction. Water is drawn from the river just south of the water treatment plant. The Village is permitted to withdraw 2.5 million gallons per day and in 2015, the Village withdrew a daily average of 741,276 gallons.

2. Water Treatment Facility

Raw water from the Genesee River is withdrawn from the intake just south of the treatment plant and then pumped to the Water Treatment Facility. As water enters the plant, a coagulant is added in order to aid in filtering the water and chlorine is added as a disinfectant. The Water Treatment Facility contains three filter units. The water enters the plant clarifier and is pumped to the top of the system. The water then filters down through the final set of filters. From the filter units, water is discharged to the plants 500,000 gallon clearwell for final treatment. Chlorine is injected into the water as it is pumped to the clearwell for final disinfection. Also added to the clearwell water is soda ash (for pH control) and fluoride. The volume of the clearwell of 500,000 gallons provides suitable contact time for the chlorine as well as provides adequate equalization volume to buffer between the plant to where the system booster pumps discharge the treated water into the system. It takes the water approximately four hours from the time it is discharged from the filter to the time it is pumped out from the clearwell by one of two (2) vertical turbine pumps. The vertical turbine pumps are 125 HP and discharge at a rate of 950 gpm. A smaller 60 HP pump also pumps from the clearwell and is operated in conjunction with one of the larger pumps if the water level in the clearwell is high.

3. Production

The Water Treatment Plant has a meter on the intake line from the Genesee River prior to the filter units and also has a meter on the discharge side of the vertical turbine pumps that draw treated water from the clearwell. The meter after the clearwell represents the water production for the Village of Wellsville.

Water production records were obtained from the Village of Wellsville and HUNT plotted the Water Treatment Plant's discharge data to determine a maximum monthly production of approximately 25,000,000 gallons, as can be seen in the following figure. Note that in determining the maximum monthly demand, statistical outliers have been removed from consideration. For instance, there were significantly high water production levels that were likely attributed to flushing, breaks or other issues that were eliminated from the data. The Village had a maximum day of 1,543,300 gallons in 2015 and 1,784,400 gallons in 2016.

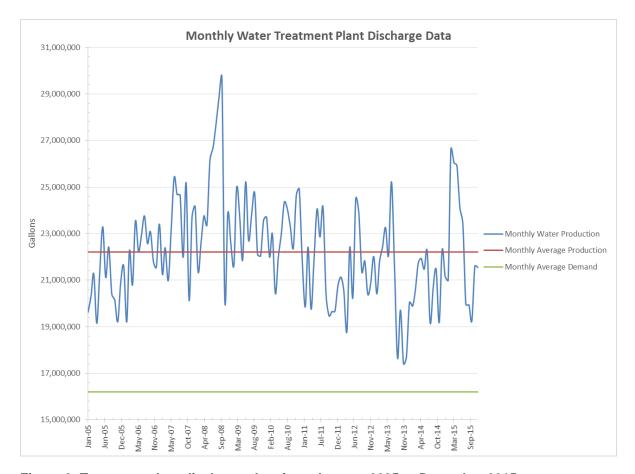


Figure 2: Treatment plant discharge data from January 2005 to December 2015.

Additionally, as can be seen from Figure 2, the system sees typical seasonal fluctuation. Water demand varies over time depending on weather conditions, and it is possible to distinguish seasonal and non-seasonal components of water use. In the residential sector, nearly all seasonal use is outdoor use. Non-seasonal use is assumed to be relatively constant throughout the days and months of the year, and in the residential sector it generally represents indoor use. Using the three winter months for calculating non-seasonal use is as follows:

Nonseasonal Use = Average Winter Season Consumption =
$$\frac{(V_{Dec.} + V_{Jan.} + V_{Feb.})}{3}$$

Where:

V = Total Volume of Water Consumed During the Month

Nonseasonal Use = Average Winter Season Consumption
$$= \frac{(15,932,400 + 14,273,336 + 12,763,124)}{3}$$
Nonseasonal Use = Average Winter Season Consumption = $14,323,000$ gallons per month

4. Unaccounted for Water

Unaccounted for water is the difference in the total water consumption and the billed water production. Sources of unaccounted for water include leaks, unmetered water use, inaccurate meters, hydrant flushing, and street cleaning. The Environmental Protection Agency (EPA) has established an industry goal of 10 percent for unaccounted for water system losses.

Annual consumption and production rates were tabularized from 2005 to 2010 and are shown in the table below. The percentage of unaccounted for water for 2015 is calculated as follows:

$$\label{eq:Unaccounted} \text{Unaccounted for water} = \frac{\text{(Total Production} - \text{Total Consumption)}}{\text{Total Production}} \times 100\%$$

$$\text{Unaccounted for water} = \frac{(270,565,600-166,595,804)}{270,565,600} \times 100\%$$

Unaccounted for water = 38%

Table 2: Annual water production, consumption, and unaccounted for water.

Year	Produced (gallons)	Consumed (gallons)	Unaccounted (gallons)	Percent Unaccounted
2005	249,196,600	195,420,984	53,775,616	22%
2006	265,361,600	188,424,232	76,937,368	29%
2007	276,882,300	217,754,768	59,127,532	21%
2008	298,178,100	203,839,724	94,338,376	32%
2009	279,011,250	196,217,305	82,793,945	30%
2010	277,701,400	212,597,308	65,104,092	23%
2011	255,952,000	214,040,959	41,911,041	16%
2012	256,734,500	194,063,012	62,671,488	24%
2013	251,286,800	177,561,561	73,725,239	29%
2014	250,642,600	171,847,016	78,795,584	31%
2015	270,565,600	166,596,804	103,968,796	38%

Over the past ten years, the water system has an average of 27% unaccounted for water of the amount of water produced, and recently as high as 38% unaccounted for water in 2015. Due to the historically high percentage of unaccounted for water, the Village has taken measures to reduce the volume of unaccounted for water within their system. The production meters for the Water Treatment Facility were replaced in 2009. Since 2011, the Village has started to replace smaller meters in the water systems for residential and commercial properties. Between 2011 and 2015, the Village replaced 1,977 meters out

of their 2,300 service connections, approximately 86% of the total meters in the system. The Village completed residential meter replacement in 2016.

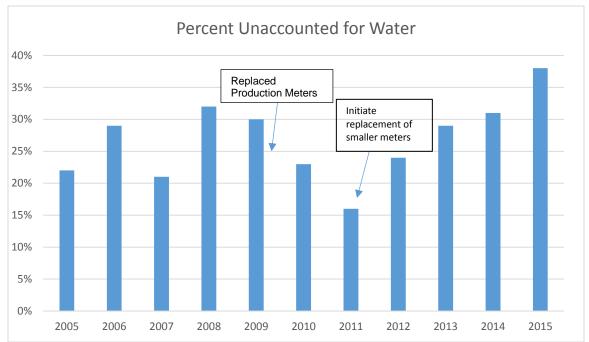


Figure 3: Village of Wellsville unaccounted for water, 2005 - 2015.

D. Water Distribution and Transmission

1. General Nature of Distribution System

The water distribution system was originally installed in 1883 and was owned and operated by the Wellsville Water Company. The Village of Wellsville purchased the water system in 1915. The current Village of Wellsville's water distribution system consists of nearly 26 miles of distribution pipe and has 2,300 service connections, serving 5,700 people. The distribution system consists of water main ranging from 2 to 14 inches in diameter, composed of over 50% cast iron, nearly 30% ductile iron, and the remaining consists of cement asbestos, plastic, galvanized, PVC, copper and miscellaneous materials. The Village's water supply is the Genesee River.

The majority of the water system contains looped water main with the most notable exception being dead end water main, located at the northern portion of Riverside Drive, serving the Town of Scio. Smaller sections of dead end water main are located at the water system limits. The Volunteer Fire Department performs annual hydrant flushing of the whole system to maintain the chlorine residual. A known location of stagnation, leading to low chlorine residual, is the dead end located at East Windover Road. To maintain proper chlorine levels, the associated hydrant is blown off at least every other month.

As investigated in the previous section, the Village sees cycles of high and low unaccounted for water. While some of the difference between the years of high and low unaccounted for water percentages is attributed to meter replacement, the cycles of high and low unaccounted for water speaks to the fragility of the water system and the frequent occurrence of watermain breaks. The high frequency of breaks within the water system is likely attributed to the types of pipe material, such as cast iron and transite asbestos, and tanbark subbase. Cast iron and transite asbestos pipe becomes brittle as

it ages and thus more susceptible to leaks. In addition, the Village of Wellsville has various roads that have subsoils consisting of tanbark, a waste material from tanning processes where the bark from trees is used. Several tanneries were located within the Village limits and tanbark was placed onto the roads so that in some locations the tanbark is greater than eight (8) feet deep. At this depth, the tanbark is located below the watermains. Refer to Appendix D for a map of the known locations of roads with subsurface tanbark. Historically, the Village has had issues with settlement in their roadways due to tanbark underneath that has resulted in pavement up to three (3) or four (4) feet thick in order to deal with settling of the road. Settlement of tanbark also affects the watermains as it causes the pipe to bend and flex. When coupled with older watermains consisting of brittle pipe materials such as cast iron and transite asbestos, the possibility of leaks is high. In addition, tanbark has the potential to be corrosive, as is typical of decaying organics, and can cause deterioration of the watermains.

Fire hydrant testing was not completed as part of this water system analysis. However, fire flows conducted by the Insurance Services Office (ISO) provide insight as to the condition of the interior of the water main. ISO completed an evaluation in 1997. Results are provided in Appendix E.

2. Size and Types of Water Main

As noted below in the following table and pie chart, the distribution system consists of approximately 44% cast iron water main and approximately 33% ductile iron water main. Cast iron water main is generally less resistant to scaling or tuberculation, while ductile iron water main is lined and retains a smooth interior for extended periods of time.

Use of ductile iron water main began in the early 1970's. Therefore, only those improvements after such time utilized ductile iron pipe. Even though the system has relatively large diameter water main, given the size and demands, tuberculation within cast iron pipe will consistently decrease observed flow rates within said cast iron pipes.

Table 3: Water distribution system material and length.

		Length	Length	Percent
Pipe Material	Abbreviation	(ft)	(mi)	(%)
Cast Iron	CI	91324	17.30	44%
Ductile Iron	DI	68866	13.04	33%
Transite Asbestos	TA	18441	3.49	9%
Plastic/HDPE	PL	10758	2.04	5%
Galvanized	GAL	5451	1.03	3%
Copper	CU	861	0.16	0.4%
Miscellaneous	MISC.	13266	2.51	6%
Total		208967	39.58	100%

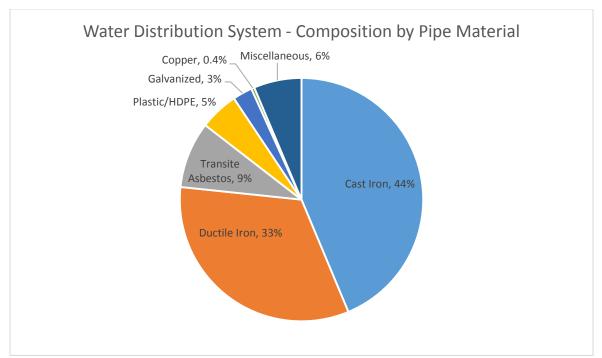


Figure 4: Water distribution system, pipe material.

As demonstrated below in Table 4 and Figure 4, the greatest percentage of system piping is 6-inch pipe, at 41%. The next greatest percentage is 37% of 8-inch pipe. The remaining network consists of 1 & 2-inch, 4-inch, 10-inch, 12-inch, and 14-inch pipe.

Table 4: Water distribution system, pipe diameter.

Pipe Type	Length (ft)	Length (mi)	Percent (%)
1 & 2 inch	9307	1.76	4%
4 inch	2347	0.44	1%
6 inch	85716	16.23	41%
8 inch	76749	14.54	37%
10 inch	20866	3.95	10%
12 inch	10170	1.93	5%
14 inch	3606	0.68	2%
Unknown	206	0.04	0%
Total			
Length	208967	39.58	100%

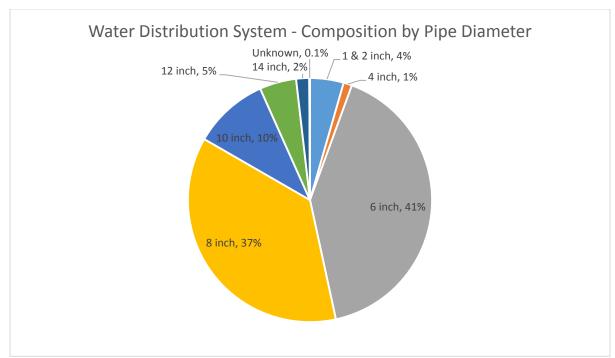


Figure 5: Water distribution system, composition by pipe diameter.

3. Water Services

The Wellsville Water System has approximately 2,300 service connections. Service piping and taps in the system are primarily copper with a variety of fittings based on installation date. The Village currently replaces service connections with $\frac{3}{4}$ inch copper and composite fittings as needed, to respond to leaks or perform preventative maintenance.

4. Valves

In-line distribution piping isolation valves within the system were last exercised 6 years ago and are reported to be working properly by the Water Department, with the exception of the valve on Fair St. on the east line of the District, which has failed in the closed position.

5. Hydrants

The Village provided records of the flow at 84 hydrants in the district in the recent past. The maximum flow measured was 1,550 gpm at Embser Sons Funeral Home parking lot, 34 West State Street. Flow at or below 500 gpm was measured at six (6) of the hydrants: 290 Farnum, 25 Hamilton, Pleasant, 45 North Broad, 329 Scott, and 54 Stevens. These hydrants are spread throughout the distribution system, see map in Appendix F.

Fire flows conducted by the Insurance Services Office (ISO) in 1997 provide insight as to the condition of the interior of the water main. Results are provided in Appendix E. ISO completed fifteen (15) fire hydrant tests and identified a needed fire flow at each hydrant. Eight (8) of the fire hydrants flushed by ISO yielded the required fire flow requirements. The following table summarizes the fire hydrant test location, flow required and flow achieved (Q_{20} Flow):

Table 5: Fire flows measured by Insurance Services Office, 1997.

Fire Flow Test Location	Needed Flow	Flow Provided	Percent of Needed Flow Provided
W. State St. 1st hydrant east of South Brooklyn St.	3,500 gpm	5,800 gpm	166%
South Brooklyn St. opposite Lunn Court	3,000 gpm	3,700 gpm	123%
Chenault Ave. at Water Treatment Plant	2,250 gpm	1,500 gpm	67%
South Brooklyn St. at Otis Eastern Pipe Yard	1,500 gpm	1,000 gpm	67%
Stevens St. 2nd hydrant east of Highland Ave	750 gpm	1,100 gpm	147%
Riverwalk Plaza 2nd hydrant south of Bolivar Rd.	2,250 gpm	1,300 gpm	58%
Route 417 @ Nursing Home	1,500 gpm	1,100 gpm	73%
Route 19 2nd hydrant north of Vassler Rd.	500 gpm	900 gpm	180%
North Mains St. and E. Pearl St.	3,500 gpm	3,900 gpm	100%
Central Ave west of Loder St.	3,000 gpm	4,300 gpm	143%
Oconnor St. west of Scott St.	2,250 gpm	900 gpm	40%
North Main St. 1st hydrant north of East State St.	3,000 gpm	3,500 gpm	117%
South Main St. north of Dyke Creek	2,500 gpm	2,500 gpm	100%
School St. opposite Fair St.	4,500 gpm	2,200 gpm	49%
South Main St. opposite Orchard Place	500 gpm	450 gpm	90%

The following locations achieved measured flows less than the needed hydrant flow, based on property insurance premium calculations only. The South Brooklyn Street at Otis Eastern Pipe Yard flow measurement is 67% of the required flow, and the ISO report states that three hydrants on the property are unusable. The area located on the northern part of the distribution system and west of the Genesee River had two hydrant flow measurements taken at terminating points that showed results below the needed flow. This area of the system is composed of ductile iron. Use of ductile iron water main began in the early 1970's. Therefore, only those improvements after such time utilized ductile iron pipe. Due to the relative youth of these pipes and the standard cement-mortar linings of ductile iron pipe, tuberculation is unlikely the cause of reduced flow. It is more likely the

lack of achieving the required flow is due to the terminus location and the high required fire flow demand, based on the nature of these commercial institutions. These included Riverwalk Plaza, measured to be 58% of the required flow, and the nearby Route 417 at the Nursing Home achieved 73% of the required flow. Another terminating point measured was Oconnor Street west of Scott Avenue located at the northeastern edge of the water district. This area has ductile iron piping. The School Street hydrant opposite Fair Street had the lowest percentage of needed flow at only 49%. This is in part due to the high need of 4,500 gpm, based on the nearby facilities. The volume of flow of 2,200 gpm is consistent to flow rates throughout the distribution system. The lowest flow measured in the system is the South Main Street hydrant opposite Orchard Place, measuring only 450 gpm. This low rate is concerning as this location is part of a looped system of ductile iron pipe at the southern side of the distribution system and east of the Genesee River. The distribution loop bounded by Main Street, South Franklin Street, and Rauber Street should be investigated further to identify if the low measurement is representative of this area, or simply a poor measurement or hydrant.

E. Finished Water Storage

The finished water storage currently has a capacity of 4,500,000 gallons from the two concrete tanks and the water treatment plant clearwell. The two (2) 2 million gallon AWWA D110-95 wire-wound concrete water storage tanks were constructed at the location of the previous water storage tank and reservoir on property owned by the Village. These tanks have an inside diameter of 130.5 feet with a total water depth of 20 feet. The water level within the tank is monitored to within one-tenth of an inch, with a pressure sensor located in a concrete vault and radio telemetry. The most recent water tank inspection was performed on August 27, 2014 by Robotic Observation Ventures, Appendix G. The inspection was performed using a robotically operated vehicle with topside real time monitoring and recording. The inspection found the roof to be in good condition, some biofilm cover on all below water surfaces, and wall surfaces to be in good condition. The floor has 90% heavy sediment on bottom of tank, and the inflow and outflow floor penetrator pipes are located within catch basins set into the floor, and the inside of pipes appear in relatively good condition. The concluding opinion is that no immediate attention is required at this time.

Based upon the following sections from the Recommended Standards for Water Works, 2012 Edition, (RSWW) a conservative approach to estimate the minimum required volume of a water storage tank involves the addition of the recommended emergency storage volume (comprised of the fire storage and the average daily consumption volume) and the equalization storage volume. The flow contribution from water supply sources with standby power can reduce the associated storage requirement. The required storage volume is calculated as follows:

Required Storage = Fire Protection Volume + Average Daily Demand + Pump Equalization – Supply from Sources with Emergency Generator

Using the criteria above, the current minimum storage required for the Village's water distribution system is estimated as follows:

Fire Protection Volume:

As per the 1997 Hydrant Flow Data Summary from the Insurance Services Office, Appendix D, the Village will need to be capable of providing fire flows of 500 to 4,500 gpm. The highest fire flows (4,500 gpm) required are at a hydrant located on School Street, opposite Fair Street. As per Section 604 of the Fire Suppression Rating Schedule, a fire flow duration of two (2) hours is recommended.

Therefore, the storage volume required based upon fire protection is:

```
Fire Flow Storage = 4,500 gpm x 2 hr. x 60 min/hr.
Fire Flow Storage = 540,000 gallons
```

Average Daily Demand

From consumption data provided by the Town, the total consumption was 163,788,812 gallons for 2015. Thus, the daily average demand for the water system is 448,736 gallons.

Pump Equalization:

The amount of equalization volume depends upon demand rates and available production rates. When supplying a constant supply rate into a system over an extended period, the required amount of equalization storage will typically range from 15 to 30 percent (%) of the maximum daily use. Therefore, the associated equalization storage volume is:

```
Equalization = 20% x Average Daily Demand Equalization = 0.20 x 448,736 gpd Equalization = 89,747 gallons
```

Water Supply from Sources with Emergency Generators:

Required storage volumes can be reduced by the volume that a water supply having a backup power supply can provide during the required 2-hour fire flow period. Emergency power supply is available for the existing pumps, therefore, reduction of the required fire flow can be realized. The volume reduction that would be supplied by the largest pump is calculated as follows:

```
Volume Reduction = Pump Discharge x (2 hours x 60 minutes/hour)
Volume Reduction = 250 gpm x (2hours x 60 minutes/hour)
Volume Reduction = 30,000 gallons
```

Given the estimated storage and supply volumes calculated above, the required storage volume as determined by the RSWW is estimated as:

```
Water Storage Capacity = 540,000 gallon + 448,736 gallons + 89,747 gallons
-30,000 gallons
Water Storage Capacity = 1,048,483 gallons
```

The Village has a combined water storage capacity of 4,500,000 gallons; therefore, the Village has a surplus of approximately 3,400,000 gallons. The existing system storage capacity exceeds the minimum required storage capacity of the system to meet average daily demand and required fire protection volume. The tanks are suitably sized for continued use, with ample capacity for growth, and are in good condition.

F. System Operations

1. System Pressures

Water systems are characterized by flows and pressure recorded during static conditions and under various operating conditions (i.e. fire flows). Topography and water levels within the water storage tank dictate system pressures throughout the Village's water distribution while water piping configuration/condition impact working or residual pressures and flows.

The ground elevations within the lower service area range from 1,486 feet to 1,675 feet and observe a static pressure, created by the water storage tank (overflow elevation of 1,740) ranging from 28 psi to 110 psi \pm . During working conditions the normal residual pressures range from 23 psi to 105 psi \pm (without the pumps operational). Over 90 percent of the system contains static pressures ranging from 70 psi to 100 psi. Refer to Static Pressure Mapping found in Appendix H.

The Recommended Standards for Water Works requires the working system pressure to be greater than 35 psi and above 20 psi at all times. There are two primary locations where the system pressure falls below 35 psi including at the northeastern terminus of Scott Avenue and at the end of Windover West. Windover West is located close to the tank and therefore the pressures remain buffered and do not fall below 20 psi at any time. However, during a fire flow event (fire flows in the in the downtown area or in the northeastern portion of the system will result in residual pressures at the northern end of Scott Road to fall below 20 psi, often to near 0 psi.

Pressure fluctuation also occurs during pump operations. However, operation of the finished water pumps, pressures increase especially around the location of the water treatment plant and areas where the pump discharge piping interconnects with the distribution system. The table below describes the pressure fluctuations:

Table 6: Pressure fluctuations for pumps on and off.

Location	Pumps Off	Pumps On*
W. State Street at High School	104 psi	116 psi
Pump Discharge on E. State Street	97 psi	119 psi

^{*} Worst case pump operational scenario includes simultaneous operation of 125 Hp and 60 Hp pumps.

Rapid pressure fluctuation in the system associated with pump station operation can lead to water hammer as well as damage to the system over time.

2. Available Fire Flows

The water model was utilized to identify available fire flows within the water system. As demonstrated in the Fire Flow Mapping located in Appendix I (for fire flows with the finished water pumps off), the available fire flows within the distribution system range from approximately 500 gpm to over 3,500 gpm. These elevated fire flows are directly attributed to the extensive system looping, larger diameter watermain utilized in many areas of the system and elevated system pressures. The available flows are largely meet the requirements of the ISO standards as outlined in the ISO test report found in Appendix E. There are several locations where adequate fire flow is not achieved including Scott Avenue (Northeastern most portion of the system), School Street and Rte 417/E. Dyke Street (Southeastern Portion of the System, and South Brooklyn Street (Southeast portion of the system). As demonstrated areas of inadequate flow are located in the extremities of the system, the furthest away from the water source and tankage.

G. Out of District Users

The Village of Wellsville water system serves the Town of Scio, located outside of the water district. The Town of Scio uses very little water on a routine basis. The Town of Scio is located to the northwest of Wellsville, along Riverside Drive, NY 19. The cement asbestos water main originates at a valve on Coates Street, just west of the intersection with Oconnor Street. The water main follows the southwest border of Dresser Rand where it meets Vossler Road, heads west and follows Riverside Drive to Scio. The extension includes seven (7) valves and is approximately 0.85 miles long. Out of district users within the Town of Wellsville include Sinclair Water District, Riverside Water District, Bolivar Road, George Street, Morningside Street, Alfred State College, and East State Street.

H. Equivalent Dwelling Units

HUNT took an inventory of existing facilities and structures within the Wellsville service area from available information, including tax records and water billings. Provided water billings records were analyzed for monthly periods ranging from March 2015 to March 2016 to determine total consumption as well as identify the average daily single family residential water usage. The average single family residential water usage shall characterize an Equivalent Dwelling Unit (EDU).

The Equivalent Dwelling Unit (EDU) concept relates all water system usage proportionately to the equivalent of a typical single-family residence. The use of EDUs standardizes the way the total estimated service charge is calculated to provide comparable results and consistency in review of the project by funding agencies.

As previously described, one EDU is an average value for the water usage expected from one single-family house in the Wellsville community. An analysis of the residential usage for the existing water service area provided an average daily demand of nearly 119 gallons per day per single family residence. The total water consumed throughout the system was characterized as follows:

Table 7: Characterization of Wellsville Water District Users, March 2015-2016
Wellsville Water Billed March 2015 through March 2016

Equivalent Dwelling Unit where 1 EDU = 119 gallons/day					
User Class	Number of Customers	Units Billed (Units/month)	Volume Water Consumed (Gallons/day)	EDU	
Commercial	455	8,102	202,019	1,712	
Industrial	4	1,901	47,391	402	
Other Sales	8	53	1,331	11	
Public St Light	2	3	77	1	
Residential	1,587	7,567	188,659	1,587	
Unclassified	146	623	15,545	132	
Total	2,202	18,250	455,022	3,845	

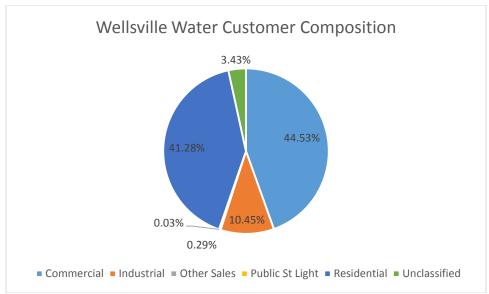


Figure 6: Composition of Wellsville Water Customer, March 2015-2016

From analysis of the aforementioned billing records, there were approximately 1,587 residential users (EDUs) along with 1,712 commercial EDUs, and 402 industrial EDUs. There were 11 other sales to Dyke Hose Co, Wellsville Fire Training Ground, Duke Hose Co., DPW, Water & Light Garage, Firehall, Police, and Municipal Building. The Unclassified sales were 132 EDUs and include clients with insufficient information to classify. Therefore, the total number of EDUs in the water service area is 3,845.

IV. Regulatory Compliance

HUNT assessed regulatory compliance of each of the components within the Village of Wellsville water system.

A. Water Source and Treatment

The Recommended Standards for Water Works, 2012 edition, (RSWW) requires that the water source be adequate to meet the maximum projected water demand of the service and provide reasonable surplus for anticipated growth. The Village of Wellsville water treatment plant is rated for a withdrawal rate of 2.5 million gallons a day and in 2016 pumped an, average of 713,967 gallons per day with a maximum day of 1,784,400 gallons. With a maximum day of 1,784,400 gallons, the plant has an excess treatment capacity of 715,600 gallons which provides for a sufficient surplus for growth as the excess capacity is equivalent to the Village's average day consumption volume.

Additionally, the RSWW indicates treatment of surface water shall include filtration preceded by appropriate pretreatment as determined by the reviewing authority, the Allegany County Department of Health. The water treatment plant currently treats water by adding a coagulant and disinfects with chlorine gas prior to treatment by the filter units. Water is passed through the clarifier portion of the filter where the majority of particles are removed and then through the mixed-media filter. After the water is filtered, the water is discharged to the clearwell where the water is again disinfected with chlorine gas and soda ash (for pH control) and fluoride are added. According to the Allegany County Department of Health 2015 Sanitary Survey, refer to Appendix J, the Village's treatment process does not have any deficiencies or violations.

B. Pumps

Recommended Standards for Water Works requires that a booster pump station shall contain not less than two pumps with capacity such that the maximum day demand can be met with the largest pump out of service. The District's booster pump station currently has three (3) pumps that are operated manually: two (2) 125 HP pumps and one (1) 60 HP pump. The 125 HP pumps are operated such that they alternate every day while the 60 HP pump is operated if the level in the clearwell is high. The treatment plant provides an adequate number of pumps to discharge treated water into the distribution system and with one pump out of service, the remaining pump has a capacity of approximately 875 gpm or 682,500 gallons per day which exceeds the maximum daily demand of 448,736 gallons. In addition, the Allegany County Department of Health 2015 Sanitary Survey indicated that no deficiencies were for the pumps and pump controls.

C. Water Storage Tanks

The concrete water storage tanks are compliant with the Recommended Standards for Water Works (RSWW) for finished water storage. The tanks are concrete structures located above ground and are protected from contamination with suitable watertight roof and sidewalls, overflow drains provided and brought down to an elevation of 12 to 24 inches above ground surface, and appropriate venting. Additionally the tanks have sufficient capacity to meet the average daily consumption within the system and the required fire flow volume.

The RSWW requires protection from trespassers with fencing and locks on access manholes to prevent trespassing, vandalism, and sabotage. While the tanks are provided with locks on access manholes and ladders, the tanks are not protected by fencing. In order to be compliant with regulatory standards, fencing should be provided around both of the water storage tanks in order to deter trespassing.

V. Asset Management

Asset management is the key to maintaining an even rate structure over time as well as ensuring financial self-sufficiency to the greatest extent possible. The basis for developing an asset management plan is to develop a fiscal plan that provides for a sustainable system. There needs to be a budget in place to operate the system on the day-to-day basis. In addition, the budget needs to detail the resources necessary to anticipate the repair, rehabilitation, and replacement of the elements in the system to minimize or avoid disruption in delivery. The revenue necessary to perform this needs to be planned in order to provide a balance for the community. This provides an ongoing operation, repair, rehabilitation, and replacement plan for optimum delivery of water service, and moves the community away from operating and maintaining the water system in a reactive nature. Asset management includes both a maintenance and replacement plan for short term assets (ie. pumps, chlorination equipment, etc.) and a capital plan with loans and grants utilized for capital projects and long term assets (ie. water storage tanks, wells, etc.).

In addition, funding agencies are beginning to look at whether a municipality utilizes an asset management program when evaluating a municipality's need for funding. It is anticipated that there will be less funding available for maintenance and repairs that could have been completed under an asset management plan. This means funding agencies may not recognize projects involving the maintenance or replacement of short term assets as a funding priority since the municipality should be planning for the maintenance and replacement of these short term assets within its budget.

A. Physical Inventory of Assets

The asset management plan looks at both short term assets and long term assets. Short term assets are assets having an expected useful life of less than 20 years. Short term assets are assets such as pumps, disinfection equipment, meters, valves, etc., while long term assets are wells, tanks, piping, etc. The physical inventory of the water system consists of two (2) surface water supply pumps, three (3) mixed media filter units, one (1) 500,000 gallon clearwell, three (3) vertical turbine pumps, two (2) booster pump in the distribution system, two (2) concrete water storage tanks, nearly 40 miles of watermain, 283 hydrants, 369 isolation valves, meters and associated fixtures, hardware and plumbing. The inventory of the water system assets takes into account the expected useful life for an asset and the remaining life left of the asset before replacement. Refer to Appendix K for the asset management worksheets. The replacement value for the water system is approximately \$51,361,265.

B. Asset Management Plan

The asset management plan looks at both short term and long term assets. As previously discussed, short term assets are assets such as pumps, disinfection equipment, meters, valves, etc. that have an expected life of 20 years or less while long term assets are storage tanks, distribution piping, etc. that have a longer expected useful life. An asset management plan addresses the amount of money that should be set aside each year to plan for the maintenance of water system assets. The annual asset management budget was established through identifying the short lived assets within the Village water system, identifying the replacement costs associated with the respective asset and amortizing the cost over its expected life. For example, the two (2) 125 HP vertical turbine pumps at the water treatment plant that pump out of the treated water clearwell were installed in 1990 and have been in operation for 27 years. The expected useful life of a pump is 15 to 20 years, but preventative maintenance measures and alternating between pumps can extend the life of the pump, such is the case for the vertical turbine pumps at the water treatment plant. Assuming the pumps have an adjusted useful life of 35 years, the pumps may require replacement in 8 years. Replacement of the 125 HP booster pump plus installation and testing costs is estimated to be approximately \$62,000. Therefore, assuming the pump's anticipated remaining useful life is 8 years, nearly \$7,750 per year should be budgeted to allow for replacement of this pump in year 2025. In the amortized annual cost for replacement of short term assets, the replacement cost of long term assets were not included. Typically, long term assets are replaced through a capital project and as such have been excluded from the amortized asset management annual replacement cost. The current inventory value of the Village of Wellsville water system is approximately \$51,361,265. If all elements of the water system were to be replaced at the end of its design life, the necessary annual investment by the Village for replacement of short term assets would need to be approximately \$98,456.

VI. Financial Status of Existing Facilities

A. Existing Water Rate

1. Historic Water Rates

HUNT reviewed the Village of Wellsville water billing rate structure through review of Annual Water Quality Reports. In 2003, the Village converted from a charge based on gallons metered to a unit billing based on one unit equals 748 gallons. The Village has used the following rate structures since 1999:

Table 8: Village of Wellsville Water Rate Structure, 1999-2002.

	Village
Customer Meter Charge per month	\$13.00
1,000-2,200 gallons	\$0.68
Greater than 2,200 gallons	\$4.68

Table 9: Village of Wellsville Water Rate Structure, 2003-2010.

	Village
Customer Meter Charge per month	\$14.00
1 to 3 units (per unit)	\$0.54
4 to 50 units (per unit)	\$3.68
51 to 100 units (per unit)	\$2.54
101 to 150 units (per unit)	\$2.27
over 150 units (per unit)	\$1.32

Table 10: Village of Wellsville Water Rate Structure, 2011-2015.

	Village
Customer Meter Charge per month	\$15.25
1 to 3 units (per unit)	\$0.54
4 to 50 units (per unit)	\$3.68
51 to 100 units (per unit)	\$2.54
101 to 150 units (per unit)	\$2.27
over 150 units (per unit)	\$1.32

2. Current Water Rate

The current rate for water consumption is comprised of a monthly billing structure based on number of units and location of property. The total Water Fund revenues were estimated to be \$1,140,000 in the 2014-2015 Annual Budget. This is about \$30,000 higher than estimated in 2013-2014. The budget and rate structure are located in Appendix E, and is summarized as follows where 1 unit equals 748 gallons:

Table 11: Village of Wellsville Water Rate Structure, 2015-2016.

	Village	Town
Customer Meter Charge per month	\$16.00	\$24.00
1 to 3 units (per unit)	\$0.57	\$0.86
4 to 50 units (per unit)	\$3.83	\$5.75
51 to 100 units (per unit)	\$2.64	\$3.96
101 to 150 units (per unit)	\$2.37	\$3.56
over 150 units (per unit)	\$1.38	\$2.07
Meter usage fee	\$2.00	\$3.00

Additional water charges are applied for account change over, water turn on/off, meter replacement, flow testing if meter is in working order, freeze-ups on the customer side, new service connection fees, and street opening.

B. Water Fund Revenues

HUNT reviewed the Village of Wellsville Annual Budget Reports from 2012 through 2016 and identified a consistent budget for the water fund. The water fund consists of revenues generated from water sales, unmetered water sales, service charges, sale of used equipment, and penalty on water sales. The following figure presents the actual total departmental income for the water fund, with the exception of 2015-2016 which presents the approved budget:

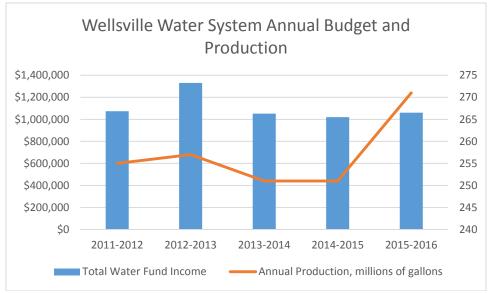


Figure 7: Village of Wellsville Total Water Fund Income, 2011-2016.

The data shows an increase in the water fund income in 2012-2013, this was due to the unanticipated \$275,916 in service charges associated with replacement of meters which began in 2011. The increase in annual production in 2016 is likely a result of leaks in the system.

C. Current Water Debt

The 2014-2015 Village of Wellsville Budget currently holds water debt, including:

Table 12: Village of Wellsville Water Fund Bond Expense, 2014-2015.

Water Fund Serial Bond	Expense
Principal WTPC PIB	\$150,000
Reservoir (New Bond)	\$40,000
Meters (New Bond)	\$50,000
Reservoir Interest (New Bond)	\$47,325
Interest WTPC PIB	\$25,500
Meters (New Bond)	\$2,998
Total Bond Expense	\$315,823

D. Proposed Water Rate with Asset Management

As previously indicated, the 2016 Village water budget anticipated \$1,030,000 in revenues and \$1,100,000 in appropriations. Historically, the Village has seen revenues ranging from \$1,019,152 to \$1,330,406 in the past five years. Refer to Appendix L for a copy of the Village's water budget.

The robustness of the water system is largely required due to the larger water users within the system, including: Wellsville Central School – High School and Elementary School, Jones Memorial Hospital, Manor Hills, Argentieri Brothers Laundry, LC Whitford, Tops Markets and K-Mart. Therefore, development of a rate should be conducted with the mindset that the larger users should pay higher costs than the typical residential users. This notion is accommodated through setting of minimum rates and corresponding water volumes. For instance, the average residential water user utilizes on average 119 gallons per day and thus 43,435 gallons per year. By increasing the base rate, the lower usage users, such as residential users, will be impacted whereas increasing the incremental cost of water beyond the minimum volumes will minimize the impact to the small user and place a higher degree of burden on the larger users.

A spreadsheet was developed to allow for incorporation of various minimum demand charges and associated volumes as well as unit costs for water utilized beyond the minimum volume. The rate structure can be adjusted dependent on which users the Village would like to place a higher degree of burden on. Adjustments to the base meter fee and the first three (3) units charge will have the greatest impact on residential users as it will result in a significant increase in their annual operation and maintenance water bill. The average single family home has an annual water use of 43,435 gallons which results in an annual operation and maintenance water bill of \$321.04. Smaller commercial users within the system that utilize approximately 349,000 gallons per year have an annual operation and maintenance water bill of \$1,887.25. Large commercial users within the system that utilize approximately 5.4 million gallons annually have an annual operation and maintenance water bill of \$13,036.80.

Incorporating the amortized replacement cost for short lived assets will add \$98,456 to the annual budget and bring the total expenditures to \$1,198,456. However, this value is reduced in that the current budget includes a reserve for equipment of \$25,000. As such, the anticipated total expenditures are \$1,173,456. Depending on where the burden of the water use is to be placed, a new rate structure can be developed by two methods: increasing the minimum monthly water use charge for 1 to 3 units, or adjusting the total rate structure to lessen the impact on residential users. For instance if the base per unit charge for 1 to 3 units is increased to \$2.89, this results in a revenue of approximately \$1,180,000 and the following charges to the users:

Table 13: O&M costs with proposed rate structure increase to base meter charge only.

User	O&M Cost	Cost	Percent Difference
Residential (43,435 gal annual use)	\$321.04	\$404.56	26.0%
Commercial (349,316 gal annual use)	\$1,887.25	\$1,970.77	4.4%
Commercial (5,484,336 gal annual use)	\$13,036.80	\$13,120.32	0.6%

As can be seen in the table above, a change in the water rate for 1 to 3 units has a significant impact on the residential users in the system and a very minor impact to large users. The other method to consider is adjusting the entire rate structure so as to place a larger burden

on the larger water users within the system and minimize the increase to residential users. Refer to the following table for the proposed water rate.

Table 14: Proposed rate structure with revisions to entire rate structure.

Meter Charge per month	\$16.00
1 to 3 units (per unit)	\$1.00
4 to 50 units (per unit)	\$4.39
51 to 100 units (per unit)	\$3.57
101 to 150 units (per unit)	\$3.23
over 150 units (per unit)	\$2.49
Meter usage fee	\$2.00

Utilizing the rate structure above results in the following annual charges to the water users:

Table 15: O&M costs with proposed rate structure increase to entire rate.

User	Existing O&M Cost	Proposed O&M Cost	Percent Difference
Residential (43,435 gal annual use)	\$321.04	\$348.88	8.7%
Commercial (349,316 gal annual use)	\$1,887.25	\$2,144.09	13.6%
Commercial (5,484,336 gal annual use)	\$13,036.80	\$20,582.64	57.9%

The water rate was adjusted so as to minimize the impact to the residential users. The monthly meter charge and meter usage fee were not changed as the Village indicated that they anticipated to maintain these monthly meter rates.

E. Rate Implementation

The significant increase associated with management of short lived assets should begin immediately understanding that several system components require replacement within the next couple of years. Implementation of these rate changes over a period of time will further place the Village behind schedule with maintaining assets and ensuring financial solvency for years to come. Therefore, it is recommended that the rate be established and implemented in its entirety. Subsequent to the implementation of the new rate, the funds gathered for asset management should be placed into accounts specifically marked for such improvements.

F. Rate Maintenance

The replacement costs associated with short lived assets were estimated using 2017 prices. In 15-20 years, these prices will be outdated; therefore, a mechanism needs to be developed to allow the rates to stay current.

The Village's cost increases each year including salaried positions, cost of fuel, vehicles, chemicals, and other materials. Therefore, the rates should be increased to reflect these cost increases. Rates should also automatically increase by the cost of living each year (Consumer Price Index). At a minimum, the rate should increase by 1.5% to 2% each year. This subtle increase will minimize impacts to large increases over time.

VII. Leak Detection and Meter Testing

A. Review of System Leak Repairs

The Village provided Leak Repair Reports between December 2012 and March 2016, Appendix M. During this time period, there were a total of 21 repairs made to the water system. There were 14 water main leaks, of which 13 were to cast iron watermain and the remaining leak from transite asbestos pipe. The water main leaks were visually estimated to produce between 20 and 80 gallons per minute (gpm) of unaccounted for water.

Leaks to water service laterals comprised 29% of the total repairs during this time period. The six service lateral leaks were estimated to produce 7-20 gpm of unaccounted for water. The service laterals were replaced with 3/4" copper line and composite fittings. One water meter leak was repaired at 100 Chamberlain, this leak was estimated to produce 40 gpm.

Based on review of repair reports, leaks were repaired within 12 hours of discovery. The following table shows unaccounted for water from leaks based on the provided estimates, and where an estimate was not made, the assumption that water main line leaks produce between 20-30 gpm, water service laterals produce 10-20 gpm, and water meter leaks produce 40 gpm was made. All leaks were assumed to be ongoing for 12 hours.

Table 16: Wellsville Water System, Estimated Volume of Leaks, 2013 – 2016.

Year	Water Main	Service Lateral	Water Meter	Total Leak
	gallons	gallons	gallons	gallons
2013	115,200		28,800	144,000
2014	43,200	43,200		86,400
2015	252,000	12,240		264,240
2016	43,200	50,000		93,200

The average volume of water estimated to be leaked during a year is calculated to be 147,000 gallons per year or 403 gallons per day. The following calculation identifies the remaining unaccounted for water in 2015:

$$\label{eq:Remaining Unaccounted for water} \begin{split} &= \frac{(\text{Total Production} - (\text{Total Consumption} + \text{System Leak Repairs}))}{\text{Total Production}} \\ &\times 100\% \end{split}$$

Remaining Unaccounted for water =
$$\frac{(270,565,600 - (166,595,804 + 264,240)}{270,565,600} \times 100\%$$

Remaining Unaccounted for water = 38%

B. Leak Detection Survey

New York Leak Detection, Inc. (NYLD) performed a leak detection survey of the Village of Wellsville in April of 2016. NYLD performed testing on all hydrants throughout the entire system, as well as testing valves and curb boxes as needed. The report generated from this testing is found in Appendix N. Surface acoustic testing was utilized for leak locations. Continuity was performed on areas containing transite asbestos pipe, accessible services were scanned in areas with plastic mains. Based on the above testing, Leakage Control Reports were generated for the following areas:

Street Address	Leak Appears	Estimation of Leak	
	to Be On	GPD	
25 Hamilton Street	Service	15,000	
252 Farnum Street	Service	7,000	
South Main Street	Water main	40,000	
540 North Main Street	Service	12,000	
47 Chestnut Street	Service	8,000	
60 Maple Avenue	Service	20,000	
47 Elm Street	Service	6,000	
313 N. Main Street	Service	10,000	
38 Rauber Street	Service	20,000	
131 Miller Street	Service	8,000	
Total		146,000	

Table 17: Village of Wellsville, NYLD Leak Detection Summary, 2016.

The total volume estimated to be leaking during this survey was 146,000 gallons per day, or 53,290,000 gallons per year. The Village of Wellsville addressed these leaks upon notification. Including the NYLD findings, the following calculation identifies the remaining unaccounted for water in 2015:

Remaining Unaccounted for water
$$= \frac{(\text{Total Production} - (\text{Total Consumption} + \text{System Leak Repairs} + \text{Leak Detection Survey}))}{\text{Total Production}} \times 100\%$$
Remaining Unaccounted for water
$$= \frac{(270,565,600 - (166,595,804 + 264,240 + 53,290,000))}{270,565,600} \times 100\%$$

Remaining Unaccounted for water = 19 %

C. Meter Testing

The Village of Wellsville obtained services from Cold Spring Environmental to perform water plant flow meter testing in May 2016, Appendix O. The following meters and chart recorders at the water treatment plant were evaluated: Treated Water Flow Meter, Clearwell Inlet Flow Meter, Treated Water Chart Recorder, and Clearwell Inlet Chart Recorder. The measured flow rate was less than or equal to 1% in all cases. No adjustments were recommended due to the accuracy of the instrumentation.

The Village requested testing and calibration for flow and accuracy of larger size meters within the water system. The planned meter calibration testing took place in June 2016 at the following locations: Wellsville Central School - High School and Elementary School, Jones Memorial Hospital, Manor Hills, and Argentieri Brothers. The following locations have larger meters, but were not tested at this time: LC Whitford, Tops Markets and K-Mart.

Table 18: Village of Wellsville, large meter flow testing, 2016.				
Site Location	Model	Model Installed Portable		Percent
		Meter CF	Meter CF	Difference
High School	Sensus 4"-6"	28	28	≤1 %
Elementary School	Sensus 4"-6"	44	45	≤1 %
Argentieri Laundry	Rockwell 4"	2	45	>100%
Jones Memorial Hospital	Rockwell 4"	35	135	>100%
Manor Care Center	Spectrum 260, 2"	34	34	≤1 %
Manor Care Center (behind WWTP)	Rockwell 2"-3"	15	15	≤1 %

The above table shows that the larger meters located at Argentieri Laundry and the Hospital are functioning with little accuracy and replacement should be considered.

1. Argenteiri Laundry Meter Reports

Argenteiri is a commercial laundry facility located at 50 West Hanover Street, it is operated from 7:00 a.m. to 5:00 p.m., or 10 hours per day. The laundry has four 90 lb washers, 5 450-500 lb washers, and one 800 lb washer. The monthly water bill records for the period of March 2015 through March 2016 supplied by the Village show a typical seasonal trend, an average billed volume of 539 Units or 13,431 gallons per day, and a maximum billed volume of 855 Units or 21,318 gallons per day:

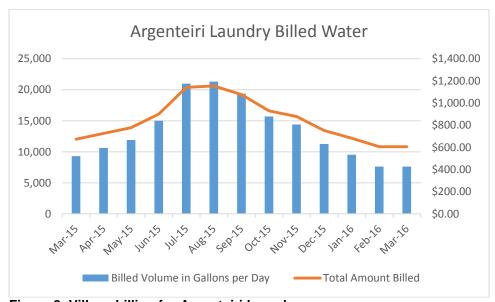


Figure 8: Village billing for Argenteiri Laundry

The Cold Spring Environmental Report measured a large discrepancy in the measured flow rate between the installed meter and the portable test meter. The installed meter measured 2 cubic feet during the 15 minute test, while the calibrated portable meter measured 45 cubic feet during the same time period. The difference between the two meters and converted to gallons per day, assuming a ten hour work day, results in a potential unmeasured flow of:

Flow in
$$gpd = \left(Flow\ in \frac{ft^3}{15\ minutes}\right) x \left(7.48 \frac{gallons}{ft^3}\right) x \left(60 \frac{min}{hour}\right) x \left(10 \frac{hour}{day}\right)$$

Potential Unmeasured Flow = Calibrated Meter Flow - Installed Meter Flow

Potential Unmeasured Flow = 13,464 gpd - 598 gpd = 12,866 gallons per day

Based on literature review of the design average daily flow rate for a laundry can be derived based on NYSDEC Design Standards for Intermediate Sized Wastewater Treatment Systems, 2014 published values of 580 gpd per machine. There are 10 industrial laundry machines at the facility, they wash an equivalent of approximately 38 traditional laundry machines, based on weight calculations. Therefore the design average daily flow rate is as follows:

 $580 \ gpd/machine \ x \ 38 \ machines = 22,040 \ gpd$

Comparing the design average flow rate of 22,040 gpd to the average billed rate of 13,431 gpd yields a discrepancy of 8,609 gpd, or a lost revenue of approximately \$476 per month. Based on the \$19,200 cost for material and installation to replace a meter of this size, it would take 40 months to recuperate the expense.

2. Jones Memorial Hospital Meter Reports

Jones Memorial Hospital is a 70 bed facility located at 191 North Main Street. The monthly water bill records for the period of March 2015 through March 2016 supplied by the Village show a step change in December 2015. The average billed volume of 509 Units or 12,695 gallons per day, and a maximum billed volume of 827 Units or 20,620 gallons per day:

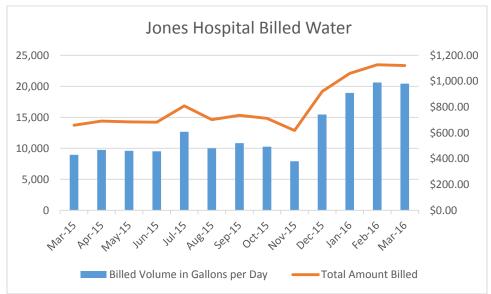


Figure 9: Village billing for Jones Hospital

The Cold Spring Environmental Report measured a large discrepancy in the measured flow rate between the installed meter and the portable test meter. The installed meter measured 35 cubic feet during the 15 minute test, while the calibrated portable meter measured 135 cubic feet during the same time period. The difference between the two meters and converted to gallons per day, assuming a 16 hour work day, results in a potential unmeasured flow of:

Flow in
$$gpd = \left(Flow \ in \frac{ft^3}{15 \ minutes}\right) x \left(7.48 \frac{gallons}{ft^3}\right) x \left(60 \frac{min}{hour}\right) x \left(10 \frac{hour}{day}\right)$$

 $Potential\ Unmeasured\ Flow =\ Calibrated\ Meter\ Flow -\ Installed\ Meter\ Flow$

Potential Unmeasured Flow = 64,627 qpd - 16,755 qpd = 47,872 qallons per day

Based on literature review of the design average daily flow rate for a laundry can be derived based on *NYSDEC Design Standards for Intermediate Sized Wastewater Treatment Systems*, *2014* published values of 175 gpd per hospital bed. There are 70 beds, therefore the design average daily flow rate is as follows:

 $175 \ gpd/bed \ x \ 70 \ beds = 12,250 \ gpd$

This design average flow rate is based on a modern hospital, a correction factor of 20% shall be added to account for the age of the hospital. Therefore the design average flow rate is 14,700 gpd.

For reference, the design average flow rate was compared to the water usage at the newly constructed Corning Hospital. This hospital utilizes approximately 10,000 gpd for a 60 bed facility. Scaling this water consumption to a 70 bed facility such as Jones Memorial Hospital results in an anticipated flow rate of 11,667 gallons per day. Again a correction factor of 20% shall be added to account for the age of the hospital, yielding a comparison of 14,000 gpd. Which is nearly identical to the above value calculated from the NYS Design Standards.

Comparing this corrected design average flow rate of 14,700 gpd to the average billed rate of 12,695 gpd yields a discrepancy of 2,005 gpd or a lost revenue of approximately \$111 per month. Based on the cost for material and installation to replace a meter of this size, \$9,600, it would take over seven years to recuperate the expense of meter replacement.

The potential combined error from these two faulty meters could lead to 10,614 gallons per day, or 3,874,110 gallons per year of unaccounted for water. Including the Cold Spring Meter Testing findings, the following calculation identifies the remaining unaccounted for water in 2015:

Remaining Unaccounted for water

× 100%

Remaining Unaccounted for water
$$= \frac{(270,565,600 - (166,595,804 + 264,240 + 53,290,000 + 3,874,110)}{270,565,600} \times 100\%$$

Remaining Unaccounted for water = 17 %

VIII.Need for the Project

A. Health, Sanitation, and Security

When utilizing a surface water source, referenced standards recommend a source protection plan be enacted for continued protection of the watershed from potential sources of contamination. The Village of Wellsville has proactively demonstrated a strong interest in protecting the source water, Genesee River, for the treatment plant. The 1974 intake and pump station, located adjacent to the treatment plant, was delegated to an emergency backup when the old Sinclair Refining Co. landfill was identified as a superfund site. The current intake weir and intake structure, built in 1981, includes a pump station and transmission line and are located upstream of the refinery site. In 2006, The Wellsville Water Treatment Plant received the National Rural Water Association's environmental cleanup project award for improvements to the community dumpsite along the back of River Road.

The NYSDEC published the Waterbody Inventory/Priority Waterbodies List Report for the Genesee River Basin in March 2003. The rivers within the Genesee River Watershed are generally satisfactory. Much of the water quality concerns in the watershed are associated with urban and industrial sources in the northern part of the watershed. Wellsville is located in the southern part of the watershed in the Genesee/Cryder Creek Watershed. This portion of the Genesee River is assessed as having no known impacts; all evaluated uses are considered to be fully supported including: water supply, public bathing, recreation, aquatic life, and fish consumption. A Source Water Assessment by the NYSDOH, conducted in 2005, found some elevated susceptibility to contamination due to agricultural activity in the watershed. However, it is important to note that this report estimates the potential for untreated drinking water sources to be impacted by contamination and do not address the quality of treated finished potable tap water. This level of susceptibility is also typical of many water supplies that experience no impacts to water supply use, and reflects the need to protect the source.

B. Aging Infrastructure

The Wellsville Water District has infrastructure limitations and deficiencies in terms of the high percentage of unaccounted for water. Over the past ten years, the water system has an average of 26% unaccounted for water of the amount of water produced, and recently as high as 38% unaccounted for water in 2015. The Village has proactively begun meter replacement, hydrant flow assessments, and repair and replacement of aging pipes.

Nearly 10% of the distribution piping is transite asbestos pipe. Health concerns arise in the event of an asbestos-cement pipe break, as repair of the break must be implemented according to OSHA standards and protocols. Asbestos can become airborne if the pipe is not handled properly.

C. Reasonable Growth

The Village of Wellsville demonstrates a steady decrease in population, as noted previously in Section II. C. Population Trends. The Village of Wellsville has the highest population in Allegany County. Downtown Wellsville retains much of its early 20th Century charm, with most of its building in fair condition and a low vacancy rate of only 4%. The Village of Wellsville has a median household income of \$39,972; 52.74% of individuals qualify as low to moderate income. The Village of Wellsville is anticipated to experience continued, yet mild, downward trend.

The existing water system treats just under one million gallons of water daily; its capacity is more than two million gallons a day if the plant were operated around the clock. The Village

has sufficient water to not only meet the needs of current industrial, commercial and residential users, but to also allow for substantial expansion of the distribution system. The 2002 Village of Wellsville Comprehensive Plan prioritized expanding water and sewer services in the following areas: 1) Morningside, 2) Route 417 west to L.C. Whitford property, 3) Route 417 west to airport and industrial park, 4) Route 417 east to Duffy Hollow Road, and 5) Route 19 south to mobile home parks. The desire to expand water services must be balanced with the economics, and will require purposeful planning and identification of funding. The current water system is designed appropriately, based on the anticipated steady population and projected water use. Decreasing unaccounted for water, in the current water supply system, will improve the percent of billed water and decrease the production needs.

IX. Proposed Water System Improvement Alternatives

The existing issues within the water system are largely related to the high percentage of unaccounted for water. As previously discussed, the water system has an average of 27% unaccounted for water over the past 10 years and recently has had unaccounted for water percentages as high as 38% in 2015. The Village has taken measures to reduce the volume of unaccounted for water within their system and started replacing smaller meters within the water system in 2011 and completed meter replacement in 2016. While the Village initially saw a decrease in the amount of unaccounted for water in 2011, the percentage of unaccounted for water has continued to rise since 2012 and a decrease in unaccounted for water has not been realized. The Village has known problems with subsurface tanbark, transite asbestos pipe, and leaks. Refer to Appendix F for mapping of potential problem areas within the distribution system.

Several areas of improvement exist to reduce unaccounted for water and the high frequency of leaks experienced by the Village each year: metering replacement for larger users in the system, addition of pump controls at the treatment plant, and replacement of critical areas of the distribution system.

A. Metering

The Village has completed replacement of smaller meters in the system and has several large commercial meters in the system that have not been replaced. As detailed previously, the larger meters in the system are Wellsville Central School – High School and Elementary School, Jones Memorial Hospital, Manor Hills, Argentieri Brothers, LC Whitford, Tops Markets and K-Mart. Regulatory agencies recommend that commercial meters be tested annually or at a minimum be tested every 2 years for inaccuracies. Five of these large meters were tested and two of the meters were found to be inaccurate. Inaccuracies in the meters can result in a higher unaccounted for water percentage and inaccuracies represent lost revenue for the Village. While the large water users only encompass approximately 7% of the monthly water billings, the Village still has the expense of treating, pumping, and storing water and larger users cause more wear and tear on the system as they require a larger demand of water than residential users. The following alternatives are: 1) Do nothing, 2) Replace inaccurate meters, 3) Replace all commercial meters.

1. Do Nothing

The large water users encompasses approximately 22% of the monthly water billings. In addition, while two meters are not accurate they do register a portion of the monthly flows for each facility. Additionally, the Village could enter into a large user contract with each facility for a set usage amount each month.

2. Inaccurate Meter Replacement

Meters typically have a life expectancy of 20 to 25 years for residential meters that are recommended to be replaced every 1,500,000 to 2,000,000 gallons. Commercial meters

see a much larger water use than residential meters and typically require replacement earlier than residential meters. Based on billing records for these larger meters, meters such as the schools experience annual water use upwards of 1,500,000 gallons. As such, large commercial meters should be tested every two years at a minimum to ensure that meters remain accurate.

As indicated by the calibration testing, two of the larger meters are currently inaccurate and should be replaced. The Village has standardized on Badger meters and the cost to replace the two meters with a 4" Badger FS meter is \$19,200. The Village has purchased both meters and has installed a new meter at Jones Memorial Hospital. The Village is in the process of replacing the meter at Argentieri Brothers Laundry.

3. Meter Replacement

While the previous option evaluates the cost to replace the existing inaccurate meters, consideration should be given to replacing all of the commercial meters. Based on the annual water use and age of the meters, replacement of the meters should be planned as it is anticipated that the meters will likely need to be replaced in the next 5 to 7 years. Additionally, the advantage to replacing the existing meters with newer meters is that most new meters are re-buildable. It is understood that the Dresser Rand meter has been recently replaced and the remaining nine number of large meters are aging and have significant volumes of flow pass through them and therefore it would be recommended to replace all of the aging large meters. The construction cost to replace all nine large commercial meters in the system is approximately \$72,641. Refer to Appendix P for the detailed cost estimate for meter replacement.

B. Water Treatment Plant Controls and Pump Improvements

1. Pump Controls

As previously described, raw water is pumped to the plant using a combination of four submersible pumps at the intake. This raw water is filtered and discharges into a buried 500,000 gallon clearwell from which three finished water pumps withdrawal water and convey it into the distribution system. Given the manual operation of the finished water pumps maintaining water levels in the clearwell often requires turning pumps on and off. For instance, if a 125 HP pump is pumping into the system and the clearwell water level is rising, the operator will engange the 60 HP pump. Likewise if the water level is dropping, the 60 HP pump can be taken off-line. This type of operation not only requires additional attention from the operator but also impacts system pressures as discussed previously in this report. There is a noticeable number of breaks that originate in close proximity to the plant and become less frequent as the distance away from the plant increases. As the distance from the plant increases, pressures stabilize. Hence, there is a strong connection between the operation of the plant and leaks that may occur in close proximity to the facility. As such, balancing of flow rates through the use of a control system and variable speed drives will allow for reduced the frequency of multiple finished water pumps operating simultaneously.

Secondly, the filter plant is approaching 27 years old and the existing control panels and interior components are archaic. Many of the parts and PLCs are no longer available or practically serviceable. Therefore, improvement of the controls to better manage system pressures will also aid in replacement of parts that have simply exceeded their useful life.

2. Pumps

a. Raw Water

There are four submersible style raw water pumps including two (2) 15 HP pumps and two (2) 25 HP pumps with variable speed drives. These pumps are original to the facility and at 27 years old have significantly exceeded their useful life. Typically, submersible style pumps that are operated for a portion of the day will commonly last 10-15 years. While there is redundancy at the plant, these pumps should be heavily considered for replacement along with the associated variable speed drives. Given technology improvements in the past 27 years, high efficiency submersible style pumps can replace the existing Meyers submersible style pumps to reduce operational costs.

b. Finished Water

The finished water pumps and motors are vertical turbine style pumps that have been properly maintained (windings, oil, etc). These motors, while 27 years old, are in acceptable shape and have adequate remaining useful life to not warrant immediate replacement. However, as indicated above, the pumps warrant additional controls to interface with the operation of the raw water pumps. This type of interface will require variable speed drives to be added to the pumps to allow for balancing of the water level within the clear well during operation of said pumps. This improvement will require the installation of three variable speed drives on the two (2) 125 HP finished water pumps and the 60 HP finished water pump.

3. Construction Cost

The cost to complete this work shall include removal and replacement of the four (4) finished water pumps, removal of existing variable speed drives and replacement, removal of existing pump control systems, installation of a new control panel designed to interface with raw water and finished water pumps, and installation of the three finished water pump variable speed drives. The construction costs to complete this work is \$302,510. Refer to Appendix P for a detailed cost breakdown.

C. Distribution System

As indicated on the potential problem areas map (Appendix F), the Village distribution system is subject to leaks. These leaks can be attributed to several factors in this area: fluctuations in pressures, underlying tanbark, and pipe material. The following sections outline improvements for the distribution system.

1. Subsurface Tanbark

A clustering of leak locations are relatively centralized near the water treatment plant. Review of tanbark location mapping shows that this area has tanbark underlying many of the roadways and the water treatment plant itself is known to be built upon tanbark. Village reports indicate that in areas of known tanbark under the road such as West State Street, the pavement is 3 to 4 feet thick and tanbark is located upwards of 8 feet in depth. Tanbark also settles unevenly which causes shifts in the watermain and the pipe to pull apart at its joint resulting in a leak. Additionally, pressures near the water treatment plant see pressure fluctuation as pumps are turned on and off and can see pressures ranging from 104 to 119 psi. This constant pressure fluctuation puts strain on the distribution system and coupled with brittle watermains such as cast iron pipe, increases the

likelihood of leaks. Tanbark also has the potential to damage watermains as with any decaying organic, it is likely corrosive.

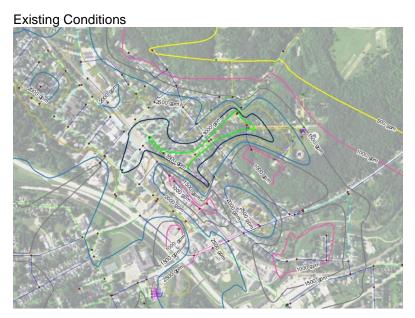
a. Watermain Replacement

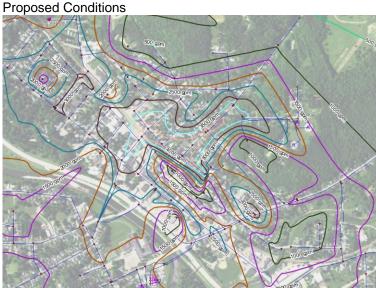
Due to the corrosive quality of tanbark as well as its tendency to unevenly settle, watermain should be replaced in the following locations where tanbark is known to occur: Madison Street, Brooklyn Avenue, Early Street, South Main Street, West State Street and Stevens Street. These watermains shall be replaced with ductile iron pipe which will further improve the system as the majority of the streets listed above are cast iron watermain that likely has tuberculation due to its age, thus decreasing the interior diameter of the pipe and restricting flows. Ductile iron pipe is also cement lined and has less friction than cast iron pipe. Newer technologies such as gripper ring gaskets shall be used at the joints to aid in not allowing pipe joints to pull apart when there is a shift in the watermain. Due to the organics in the soil from the tanbark, pipe shall be encased in polyethylene wrap in order to provide corrosion protection. Additionally, sizes of pipe will be increased during watermain replacement where appropriate so as to increase flows.

The following sections illustrate and describe the improvements at each street location.

i. Madison Street

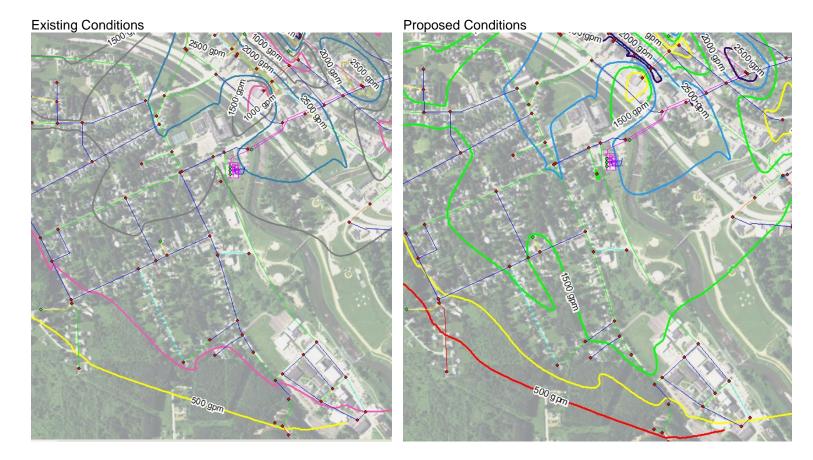
Watermain replacement at Madison Street involves replacement of cast iron pipe with ductile iron pipe and as can be seen in the images below results in a 500 gpm increase to flows in the downtown area. Additionally, Madison Street contains the distribution piping coming from the water storage tanks. While there is no history of known leaks in this waterline, it is a high priority due to the lack of multiple mains coming from the storage tanks. A break in this watermain has the potential to drain the tanks which could cause significant water damage and affect the Village's ability to provide fire protection. This watermain should be given high priority for replacement due to the threat to health, safety, and property in the event of a significant break





ii. Brooklyn Avenue

Watermain replacement at Brooklyn Avenue involves replacement of 6" cast iron pipe with 8" ductile iron pipe. As can be seen below, while the pipe replacement does not create a significant change in the central part of the Village adjacent to the Genesee River this increase in pipe size has a 500 gpm increase to the southeastern portion of the water system as higher flows are extended further into this area.



iii. Early Street

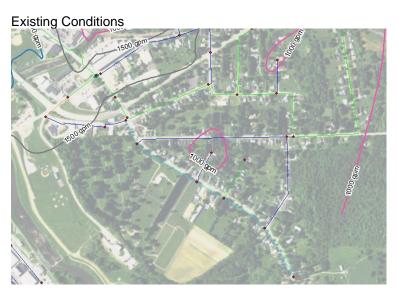
Watermain replacement at Early Street involves replacement of 6" cast iron pipe with 8" ductile iron pipe. As can be seen below, this increase in pipe size has a 500 gpm to the area and also extends higher flows to the extents of the distribution system.

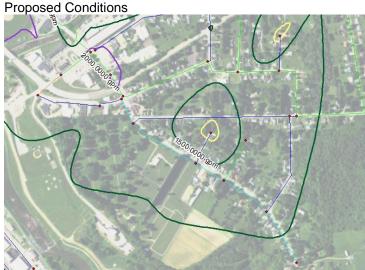




iv. South Main Street

Watermain replacement at South Main Street involves replacement of 10" cast iron pipe with 10" ductile iron pipe. As can be seen below, the change in pipe material resulted in an increase of 500 gpm to an area that has insufficient fire flows. With a 10" ductile iron pipe, sufficient fire flows are met. Due to this increase in fire flows, this watermain replacement is given priority for health and safety reasons.





v. West State Street

Watermain replacement at West State Street has two options: replace the entire length of the watermain or replace the section of West State Street to the east of Brooklyn Avenue. Full replacement of the watermain would involve replacement of 6" cast iron with 8" ductile iron pipe to the east of Brooklyn Avenue and replacement of 8" ductile iron installed in the 1970s with 8" ductile iron pipe. Partial replacement of the watermain involves replacement of just the 6" cast iron pipe to the east of Brooklyn Avenue with 8" ductile iron pipe. As can be seen below, this increase in pipe size and material for the section east of Brooklyn Avenue extends higher flows to the extents of the distribution system. This watermain replacement is given priority due to its location adjacent to the water treatment plant where water is discharged into the distribution system and as this section sees a frequency in pipe breaks.

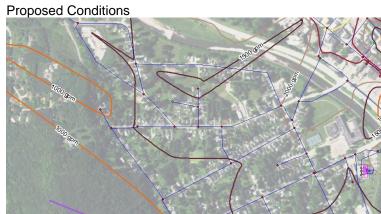




vi. Stevens Street

Watermain replacement at Stevens Street involves replacement of 6" cast iron pipe with 8" ductile iron pipe. As can be seen below, the change in pipe material and size results in an increase of 500 gpm to areas at the extents of the water distribution system.





b. Replacement Priority

Given health and safety issues, the watermains with the highest priority are Madison Street, West State Street, and South Main Street. Replacing watermains at these three locations will provide sufficient fire flows to areas with previously insufficient flows, and protect the water system's ability to serve users by replacing watermain to the storage tanks and in front of the water treatment plant.

c. Construction Cost

The table below gives the watermain replacement construction cost for each street. For a detailed cost estimate of each street, refer to Appendix P. The total project construction cost is \$3,476,853 (with replacement of West State Street only to the east of Brooklyn Avenue).

Table 19: Watermain replacement construction cost for locations with tanbark.

Street	Watermain Replacement Cost
Madison Street	\$698,281
Brooklyn Avenue	\$793,299
Early Street	\$520,580
South Main Street	\$669,736
West State Street (full)	\$672,184
West State Street (partial)	\$218,561
Stevens Street	\$576,396

2. Transite Asbestos Pipe

Approximately 9% of the water distribution system is composed of transite asbestos pipe. Based on the age of the watermains and the documented expected useful life of 70 years for transite asbestos pipe, it is anticipated that the transite pipe has a remaining useful life of 15 to 20 years. Over time, transite asbestos pipe gradual degrades in the form of corrosion by calcium leaching due to internal conveyed water and external groundwater. Leaching leads to pipe softening and loss of mechanical strength. Thus, as the water distribution system ages, the number of pipe failures increases. In addition, asbestos pipe poses health and safety risks. The water is currently tested and while no adverse impacts have been found, required testing of the water points to the potential for future regulations. As such, it is recommended that a plan be in place for replacement of transite pipe beyond its remaining useful life. The areas of the water system containing transite asbestos pipe can be separated into 6 different sections: Fairview and John Street, King Street, State Route 19 within the Town of Wellsville, Trapping Brook Road, Meadowbrook Court, and Witter Avenue.

a. Village of Wellsville - Construction Cost

The distribution system for the Village contains approximately 3,500 feet of transite asbestos. If all of the transite asbestos were to be replaced in 15 years, the Village would need to replace approximately 230 feet per year. The Village has 5 sections containing transite asbestos and should establish a goal of completing a street every 2 years. The remaining streets requiring replacement include; Fairview/John Street, King Street, Trapping Brook Street, Meadowbrook Court, Witter Ave.

The table below gives the watermain replacement construction cost for each street section. For a detailed cost estimate of each street section, refer to Appendix P. The total project construction cost for the Village is \$713,228.

Table 20: Transite asbestos pipe replacement costs for the Village of Wellsville.

Village of Wellsville Street | Watermain Replacement Cost

Tillage of Trelistine officer	Waterman Replacement Cost
Fairview and John Street	\$225,289
King Street	\$96,533
Trapping Brook Street	\$114,294
Meadowbrook Court	\$165,384
Witter Avenue	\$111,728

b. Town of Wellsville - Construction Cost

The Town of Wellsville contains distribution piping along State Route 19 that consists of approximately 5,100 linear feet of 8" transite asbestos pipe. If all of the transite asbestos were to be replaced in 15 years, the Town would need to replace approximately 340 feet per year. It is recommended that the Town plan to replace the north end of State Route 19 in 5 to 7 years. This section is a priority as it replaces a large section of transite asbestos and is a primary transmission main. The construction cost to replace the transite asbestos pipe within the Town is \$878,488.

3. Water System Looping

The existing water system has extensive looping with the exception of a few areas. Dead end watermain are primarily near obstructions such as a stream, State Road or railroad, and as such implementation of these loops would be expensive. Furthermore, these areas are not strategically placed that would likely result in significant hydraulic gains or improvements to water quality. Many of these areas are located in in close proximity to sufficient water usage to allow for adequate turnover. Three areas where looping was identified include the intersection of Bolivar Road & North Highland Ave, Madison Street & the Railroad, and South Broad Street & Dyke Creek.

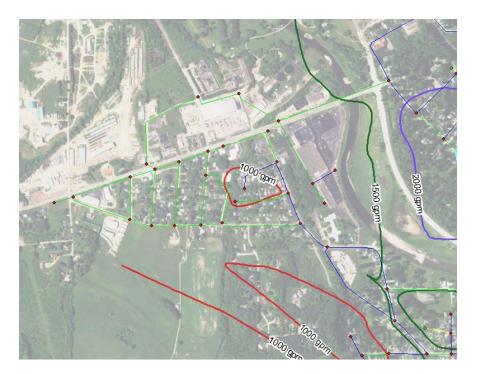
Other watermain looping was identified but implementation is limited due to excessive cost versus overall benefit. For instance, looping of watermain along East State Street with that along Route 417 would likely aid in providing a supply of water to the Southeastern portion of the distribution system, however, the watermain along East State Street is smaller distribution piping versus larger diameter transmission pipe. Looping this watermain would provide little hydraulic benefit to the Southeastern portion of the distribution system. Similarly, looping of the extreme southern portion of the system was contemplated, however, the terminus points have significant separation distance (i.e. 3,000 + feet) and a river crossing would need to be incorporated. Furthermore, the watermain in the southwest portion of the system is of poor quality and looping would result in reversal of flow direction and poor water quality for a notable period of time.

a. Bolivar Road Loop

Watermain along N. Highland is not looped with watermain along Bolivar Road. As a result low fire flows occur in the vicinity of Crowner Ave, which is near the proposed loop. The existing fire flows in this area are summarized by the graphic below. As depicted, the fire flows in the Crowner Ave location approach 500 gpm.



Completion of this loop will obviously have a positive impact on fire flows in the area as a result of maintaining pressures and providing additional supply capacity. The model predicted fire flows subsequent to the installation of 8-inch diameter watermain between N. Highland and Florida Ave, along Bolivar Road, is shown below. This improvement adds nearly 500 gpm to the fire flows experienced along Crowner while providing slightly less benefit to the other surrounding streets.



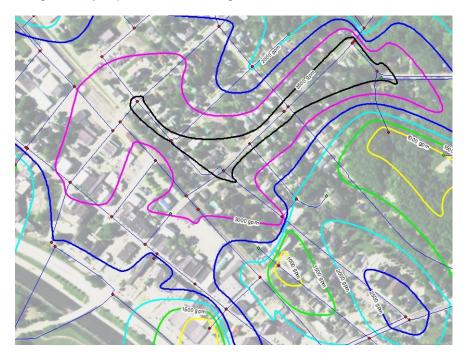
Looping the existing 8 inch diameter watermain along N. Highland with the 8 inch diameter watermain along Bolivar Road/Florida Ave would require the installation of nearly 820 feet. The cost to complete this work is complicated by the State Road and is anticipated to have a construction cost of \$109,495. Refer to Appendix P for a detailed construction cost.

b. Madison Street at Railroad

The 6-inch diameter watermain extending along the western side of the existing railroad was never interconnected with the 6 inch diameter watermain on the opposite side of the road. Fire Flows are already elevated in this area and interconnection of this watermain would require extension beneath a railroad underpass. The existing fire flows in this area are highlighted by the following graphic. Fire flows in this area range from 1000 gpm to 3000 gpm as shown below.



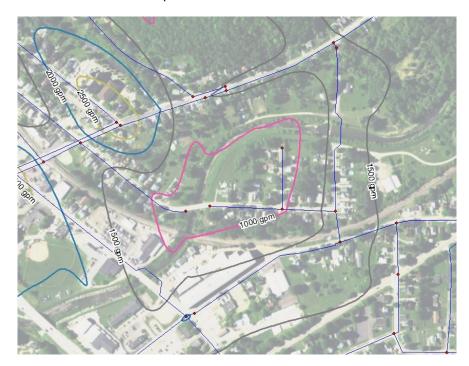
Installation of a 6-inch diameter watermain would add in the conveyance of water to the downtown area through the additional looping. This watermain would have to be installed using directional drilling methods due to the railroad underpass. Installation of the watermain would aid in the redistribution of higher fire flows such that fire flows in the area approached 3000 gpm as shown below. Improvement of the fire flows would be in areas behind the North Main street development and would not significantly impact fire flows along North Main Street.



The construction costs associated with looping this watermain is estimated to be \$137,344. Refer to Appendix P for a detailed construction cost.

c. South Broad Street at Dyke Creek

Watermain along South Broad Street was terminated at the stream bank and not installed across Dyke Creek. As such there are two dead-end 6-inch diameter watermain along South Broad Street that limit the fire flow capabilities in this area to around 900 gpm as shown below. This fire flow is sufficient to meet the current residential fire flow requirements.



Understanding that limited fire flows exist throughout the southeastern portion of the distribution system, providing looping of the watermain along South Broad Street was investigated. Installing a 6 inch diameter watermain beneath Dyke Creek through directional drilling would have a construction cost of approximately \$54,482. Installation of a 6-inch diameter watermain to loop the existing dead end piping would provide limited benefit to the surrounding area. While fire flows along South Broad Street would increase to nearly 1,300 gpm from 900 gpm, the increased benefit remains localized and does not improve supply capacity to the greater southeastern distribution system. This is demonstrated in the model predicted fire flows graphically depicted below.



The construction costs associated with looping this watermain is estimated to be \$54,482.25. Refer to Appendix P for a detailed construction cost.

4. Southeastern Distribution System Fire Flow Improvements

The southeast portion of the distribution system, extends a significant distance away from the heart of the distribution system and its inherent looping. As such this area is effectively a dead end resulting in low fire flows and poor residual pressures during fire flow events elsewhere in the system. While these poor hydraulic conditions do not impact domestic usage and day to day consumption, they unacceptable fire flows for approximately 70 homes as well as resulting in relatively low fire flows in and around the High School.

a. Improvement Options

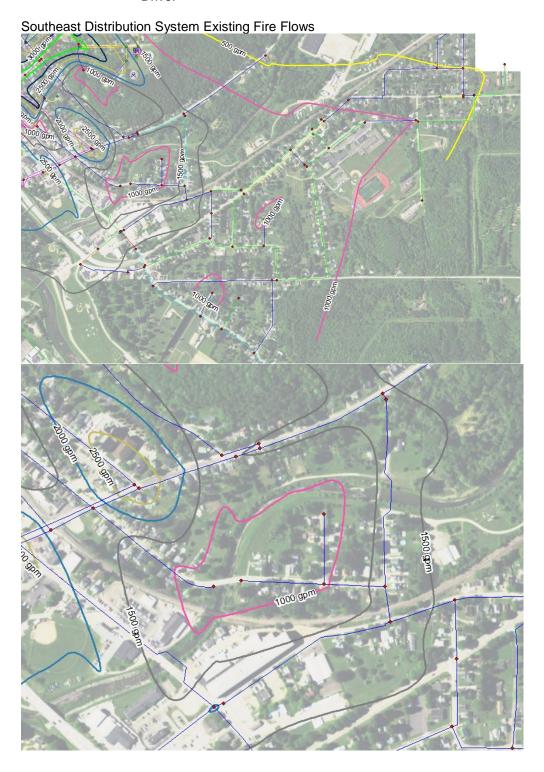
i. Looping

Looping of the watermain was investigated between the 8 inch diameter watermain located along Merriam Heights and Rauber Hill Road, it resulted in a maximum fire flow increase of less than 100 gpm. While looping in this end of the system may slightly improve water quality, the need for additional large volumes of supply is necessary to provide elevated fire flows and maintain residual pressures. Therefore, looping will not

ii. Supply Watermain Upgrades

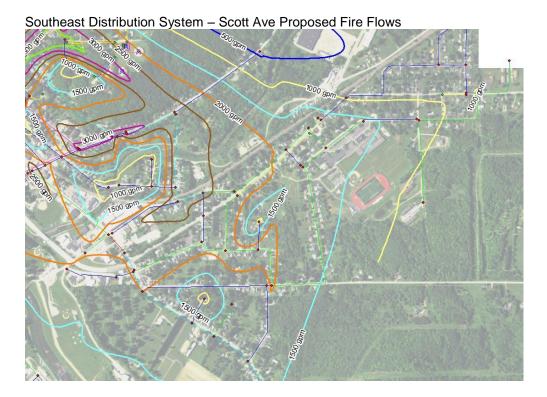
This area is supplied with water from multiple areas, however, given the pipe age and condition, significant friction losses are incurred such that low fire flows are realized. Calibration of the water model has identified appreciable attack on the watermain such that fire flows approaching 500 gpm are only available. This area is supplied water from South Main Street and watermain extending from Scott Ave. along Miller Street. Given there is a large 10 inch diameter watermain that already exists on Miller Street, upgrades to the watermain along Scott Ave

and Dyke Street would provide a large diameter conduit from the tank transmission main to this area. Another potential connection is from the existing waterline that dead ends in front of the fire training grounds that could connect to the northern end of Miller Street. As shown below, the fire flows during current conditions, with the finished water pumps off, are only approximately 1000 gpm near the High School and below 500 gpm along Sunnydale Ave and Cresent Drive.



1. Scott Ave

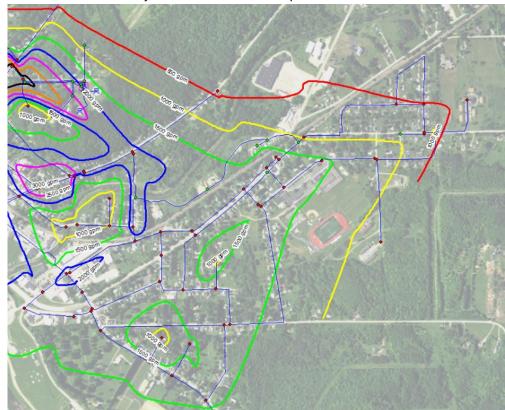
Increasing the Scott Ave and Dyke Street watermain to match the existing 10 inch diameter watermain along Miller Street will provide increased fire flows to the impacted area. Model predicted fire flows, as shown below, range from nearly 1,500 to 2000 gpm near the high school to 1000 gpm in the extreme ends of the distribution system (i.e. Sunnydale Ave & Crescent Drive).





2. Miller Street

Connecting the 8 inch ductile pipe near the fire training facility with the 10 inch watermain along Miller Street results in very little change in the model predicted fire flows as shown below. This indicates that the hydraulic shortcoming is located between Miller Street and the fire flow water supply (i.e. water storage tank). Therefore, additional improvements would have to occur such as replacement of the watermain along Scott Road from a 6 inch to a 10 inch diameter in addition to increasing the watermain size along East State Street to 10 inch to establish a large diameter transmission pipe from the near tank to Miller Street.



Southeast Distribution System - Miller Street Proposed Fire Flows

b. Construction Cost

Given that further improvements would be required for an interconnection at Miller Street to the fire training grounds in order to improve fire flows, it is recommended that the option for Scott Avenue be pursued instead. The upgrade of the watermain along Scott Ave and Dyke Street will result in the installation of nearly 3,030 feet of 10 inch diameter watermain. The construction cost of the proposed improvements is for Scott Avenue is \$431,667.74 and for East Dyke and Miller is \$151,824.07 for a total project construction cost of \$583,491.81. Refer to Appendix P for a detailed construction cost.

X. Proposed Project (Recommended Alternative)

A. Preliminary Project

The Village of Wellsville's water system is characterized by approximately 40 miles of distribution piping with associated isolation valves and hydrants, four raw water supply pumps, three booster pumps in the treatment plant, two booster pumps in the distribution system, two water storage tanks, and a treatment building and adjacent clearwell. The water system has been well maintained over the years, however, there is room for improvement as is evident through the reoccurring high volume of unaccounted for water, changing subsurface conditions resulting from buried tanbark, and future potential regulatory requirements for asbestos piping. Recognizing the need to improve the water system, the Village of Wellsville should attempt to secure funding to aid in the improvements outlined above. Hunt Engineers, Architects & Land Surveyors, PC has investigated various alternatives for improving each component of the water system to correct existing health and safety issues and aid in lowering unaccounted for water including watermain replacement, system looping, control modifications and metering.

B. Permit Requirements

Several components of the proposed project will require permits for construction. Modifications and improvements to the water system will require approval from the Allegany Department of Health. Any work done within the right-of-way of a state or county road will require a utility work permit.

C. Project Cost Estimate

The following is an estimate of the construction costs for the proposed improvements project for the Village of Wellsville water system.

Improvement	Subtotal Cost
WTP Pumps & Controls	\$ 302,510
Meter Replacement	\$ 72,641
Madison St. Transmission Replacement	\$ 698,281
West State Street Watermain Replacement	\$ 218,561
Subtotal	\$1,291,993
Engineering (8%)	\$ 103,360
Construction Observation (7%)	\$ 90,440
Administrative/Financial (3%)	\$ 38,760
Legal (2%)	\$ 25,840
Total Project Cost	\$1.550.393

D. Existing Cost to Customers

The average water use per dwelling unit is 43,435 gallons per year. Therefore, a single EDU is estimated to use 119 gallons per day. Utilizing the existing Village of Wellsville water rate structure, the annual operation and maintenance water cost is \$321.04 per EDU. Users are also billed for remaining water debt. The Village has a total outstanding water debt of \$315,823. The annual water debt charge per EDU is calculated as follows:

Annual Cost per EDU =
$$\frac{\text{Total Water Debt}}{\text{Total Number of EDUs}}$$
Annual Cost per EDU =
$$\frac{\$315,823}{3,845}$$

Annual Cost per EDU = \$82.14 per EDU Total Cost per EDU = \$321.04 + \$82.14 Total Cost per EDU = \$403.18

Thus, the existing cost of water per EDU is \$403.18.

E. Annual Operating Budget

1. Project Debt Service

Debt service is greatly dependent upon the rate of return and average interest rate utilized. Common municipal funding consists of return periods of approximately 30 years with varying interest rates. However, without attempting to speculate, a 4.5% interest rate is a safe, conservative estimate of a reasonable interest rate that could be obtained with such financing.

Therefore, the resulting debt service from the previously described project is as follows:

Annual Payment =
$$\frac{\text{Present Worth}}{\left[\frac{(1 + \text{interest rate})^{\text{# of years}} - 1}{\text{interest rate} * (1 + \text{interest rate})^{\text{# of years}}}\right]}$$
Annual Payment =
$$\frac{\$1,550,393}{\$1,550,393}$$

$$\mbox{Annual Payment} = \frac{\$1,\!550,\!393}{\left[\frac{(1+0.045)^{30}-1}{0.045*(1+0.045)^{30}}\right]}$$

Annual Payment = \$95,181.02

Annual Debt Service per EDU =
$$\frac{\text{Annual Payment}}{\text{Total EDUs}}$$

Annual Debt Service per EDU =
$$\frac{$95,181.02}{3,845}$$

Annual Debt Service per EDU = \$24.75 per year

2. Operation and Maintenance Costs

Operation and maintenance costs are based on the day-to-day requirements to keep the system operational. These items include the following:

- Pumping costs
- · Chemicals for disinfection
- Billing
- General maintenance of the system

The total operation and maintenance costs for proposed water system improvements are not expected to increase with the proposed improvements.

3. Short-Lived Asset Reserve

Refer to Appendix K for a table of short-lived assets for the water system. The table includes all of the water system's assets with expected remaining useful life and estimated replacement cost for each asset. The assets are categorized based life expectancy as either short-lived assets are or long-term assets. Based on the assets within the system, it is recommended that an annual deposit of \$98,456 be placed into the short-lived asset reserve to fund replacement and maintenance of short-lived assets. The existing Village budget (refer to Appendix L) sets aside \$25,000 for short-lived assets. Subtracting the amount the Village currently budgets for short-lived assets from the recommended annual deposits, the Village should set aside an additional \$73,456 per year. The annual cost per EDU is as follows:

Annual Reserve per EDU =
$$\frac{\text{Reserve Deposit}}{\text{Total EDUs}}$$

Annual Reserve per EDU = $\frac{\$73,456}{3,845}$

Annual Reserve per EDU = \$19.10

4. Total Financial Impact per EDU

As calculated above, the existing rate and cost of water per EDU is \$403.18. The approximate increase in debt service associated with the proposed project is \$24.75. The annual reserve for short-lived assets is \$19.10. Consequently, the approximate total annual financial impact per EDU is \$447.03.

The NYS Controller's threshold for water is \$966 per year for a single family residence in 2017. The total annual financial cost per EDU is well below the NYS Controller's threshold.

F. Review of Funding Opportunities

HUNT reviewed the current funding opportunities available to minimize the new burden to users in the water district for design and construction of the proposed project. The following grants and loans are recommended for consideration and future applications.

- 1. Office of Community Renewal, Community Development Block Grant This Public Infrastructure Grant funds up to \$600,000, of which 18% of the award may be requested for program delivery, administration, and engineering costs combined. The remaining 82% must be used for construction costs. The Village of Wellsville has a low to moderate income (LMI) of 52.74%, greater than the 51% of necessary LMI project beneficiaries. Wellsville can apply for CDBG grant assistance; the application is due end of July through the NYS CFA application process.
- 2. United States Department of Agriculture, Rural Development Water & Waste Disposal Loan & Grant Program

The USDA RD requires population of less than 10,000, and the median household income (MHI) must be below \$45,506 to qualify for RD Poverty Category Reduced Interest Rate Loan and Grant Program. The Village of Wellsville meets the population requirement with 4,679, per the 2010 Census. The Village has a MHI of \$29,177, per the 2010 Census used by USDA; well below the \$45,506 limit to qualify for the Poverty Category Reduced Interest Rate Loan and Grant Program. The Poverty Category confers eligibility for the lowest interest rate offered by RD, and possibly grant assistance. The acceptance of application for this funding is continuous.

- 3. New York State Environmental Facilities Corporation, Drinking Water State Revolving Fund Hardship Grant / Loan Program
 The NYS EFC DWSRF Grant / Loan program provides up to \$2M or 75% of project costs, whichever is less. Hardship financing requires projects have MHI less than \$46,602 to qualify. The Village of Waverly's MHI is \$38,269, per the 2013 Census used by EFC. The Village of Waverly meets the hardship requirements. Applications for DWSRF begin with listing on the Intended Use Plan; listings take place prior to August.
- 4. New York State Environmental Facilities Corporation, NYS Water Grants Drinking Water NYS Water Grants fund projects up to \$3M or 60% of total eligible project costs. The Village of Wellsville Water System Improvement Project is eligible for funding under this program. The application requires inclusion of Engineering Report, Proof of District Formation, SEQR, and SHPO. Water Grant applications are anticipated to be due in late June, 2017.

XI. Conclusions and Recommendations

The Village of Wellsville's water system is characterized by approximately 40 miles of distribution piping with associated isolation valves and hydrants, four raw water supply pumps, three booster pumps in the treatment plant, two booster pumps in the distribution system, two water storage tanks, and a treatment building and adjacent clearwell. The water system has been well maintained over the years, however, there is room for improvement as is evident through the reoccurring high volume of unaccounted for water, changing subsurface conditions resulting from buried tanbark, and future potential regulatory requirements for asbestos piping. Recognizing the need to improve the water system, the Village of Wellsville should attempt to secure funding to aid in the improvements outlined above. Hunt Engineers, Architects & Land Surveyors, PC has investigated various alternatives for improving each component of the water system to correct existing health and safety issues and aid in lowering unaccounted for water including watermain replacement, system looping, control modifications and metering.

Metering

As identified, several large meters were found to be inaccurately reading flow rates. This is due to the overall age of the meter and the large volumes of flow that pass through said meters. Including the meters that were unable to be tested as part of this report, it is known that a measureable quantity of unaccounted for water is being lost through large meter inaccuracies. Therefore, it recommended that the large diameter meters for the large users be replaced.

WTP Control & Pump Improvements

Raw water pumps and pump controls at the water treatment plant have surpassed their expected useful life and should be replaced. The treatment plant is the sole water supply of the community and limited redundancy exists, therefore, it is imperative that these improvements be completed. Furthermore, the lack of communication and control between the finished water pumps results in inefficient operation of the finished water pumps and also noticeable pressure fluctuations within the water system. These repeated pressure fluctuations are widespread and have resulted in numerous breaks in the central area of the Village surrounding the water treatment plant. The addition of controls and variable speed drives will aid in minimizing these pressure fluctuations through more efficient operation of the pumps. It is recommended that the Village immediately pursue control and pumps improvements at the water treatment plant.

Watermain Looping

While it is always desirable to eliminate dead end watermain in the system, the cost of eliminate these watermain should be weighed with the associated benefits, Currently, no significant adverse water quality issues have been encountered due to sufficient turnover, therefore, improved flows and pressures is the only additional benefit for completing such improvements.

Of the looping opportunities investigated including; Bolivar Road, Madison Street (at RR tracks) and S. Broad Street at Dyke Creek, none provided regional benefit associated with elimination of the dead end watermain. Local fire protection was enhanced but the existing fire protection provided appears adequate for the need. Therefore, none of the looping improvement projects investigated shall be considered a priority project.

Watermain Replacement in Areas of Tanbark Fill

There are multiple streets whose construction over the years included significant use of tanbark. This tanbark is organic in nature, consequently decaying over time, causing corrosion to iron piping and allowing movement of the soils surrounding the pipe. It is this movement that has contributed to countless breaks over the years on the pipes buried in this soil containing tanbark. This soil, coupled with the aforementioned pressure fluctuations, has resulted in numerous breaks along West State Street and resulted in closure of the Wellsville School on multiple occasions.

Furthermore, the only transmission main from the water supply to the Village's two (2) 2-MG water storage tanks is buried in soil containing tanbark. As a result, a break of this 14 inch diameter watermain could result in the loss of water for the community and damage to physical structures/infrastructure.

While there are a number of streets containing tanbark, there are two that should be considered priorities for improvement including the 14 inch diameter transmission main along Madison Street and the replacement of the watermain along the eastern portion of West State Street (i.e. between the River and Brooklyn Ave. The remainder of the streets should be completed as funds become available. These streets include, in this order: South Main Street, Brooklyn, Stevens, and Early.

Transite Pipe Recommendation

It is understood that the transite asbestos pipe in the distribution system has a remaining useful life of 15 to 20 years and a plan should be set in place for replacement of this pipe. The distribution system for the Village contains approximately 3,500 feet of transite asbestos. If all of the transite asbestos were to be replaced in 15 years, the Village would need to replace approximately 230 feet per year. The Village contains 5 streets containing transite asbestos. The Village should plan to replace a street with the goal of completing a street every 2 years. The streets requiring replacement; Fairview/John Street, King Street, Trapping Brook Street, Meadowbrook Court, Witter Ave. It is recommended that the Town plan to replace the north end of State Route 19 in 5 to 7 years. This section is given priority as it replaces a large section of transite asbestos and is a primary transmission main.

Therefore, the proposed immediate improvement recommendations are summarized as follows:

Improvement	Subtotal Cost
WTP Pumps & Controls	\$ 302,510
Meter Replacement	\$ 72,641
Madison St. Transmission Replacement	\$ 698,281
West State Street Watermain Replacement	\$ 218,561
Subtotal	\$1,291,993
Engineering (8%)	\$ 103,360
Construction Observation (7%)	\$ 90,440
Administrative/Financial (3%)	\$ 38,760
Legal (2%)	\$ 25,840
Total Project Cost	\$1,550,393

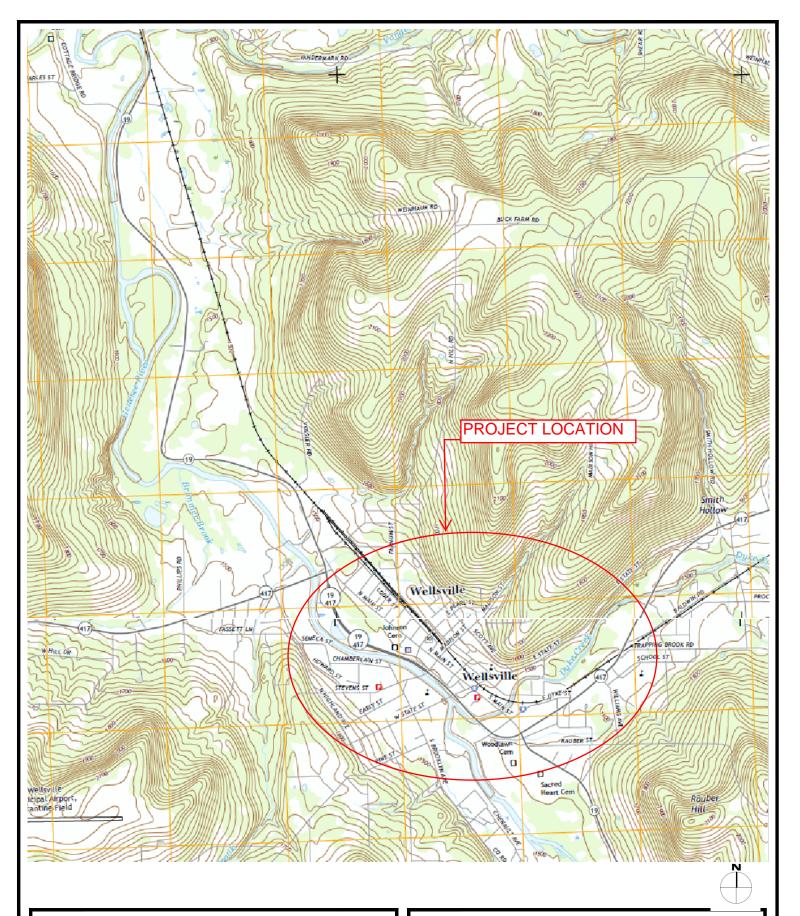
The secondary project, as monies become available and in approximately 7 years, should include the completion of:

- South Street watermain replacement,
- North Main Street watermain replacement, and
- Scott Road watermain Replacement.

The subsequent phases of the improvement project should be complete as monies become available and include;

- Brooklyn Street,
- School Street,
- · Stevens Street,
- Meadowbrook Court,
- Witter Ave
- Fairview/Johns Street
- King Street
- Early Street

APPENDIX A PROJECT LOCATION MAPPING





HORSEHEADS, NY

AIRPORT CORP. PARK 100 HUNT CENTER HORSEHEADS, NY 14845

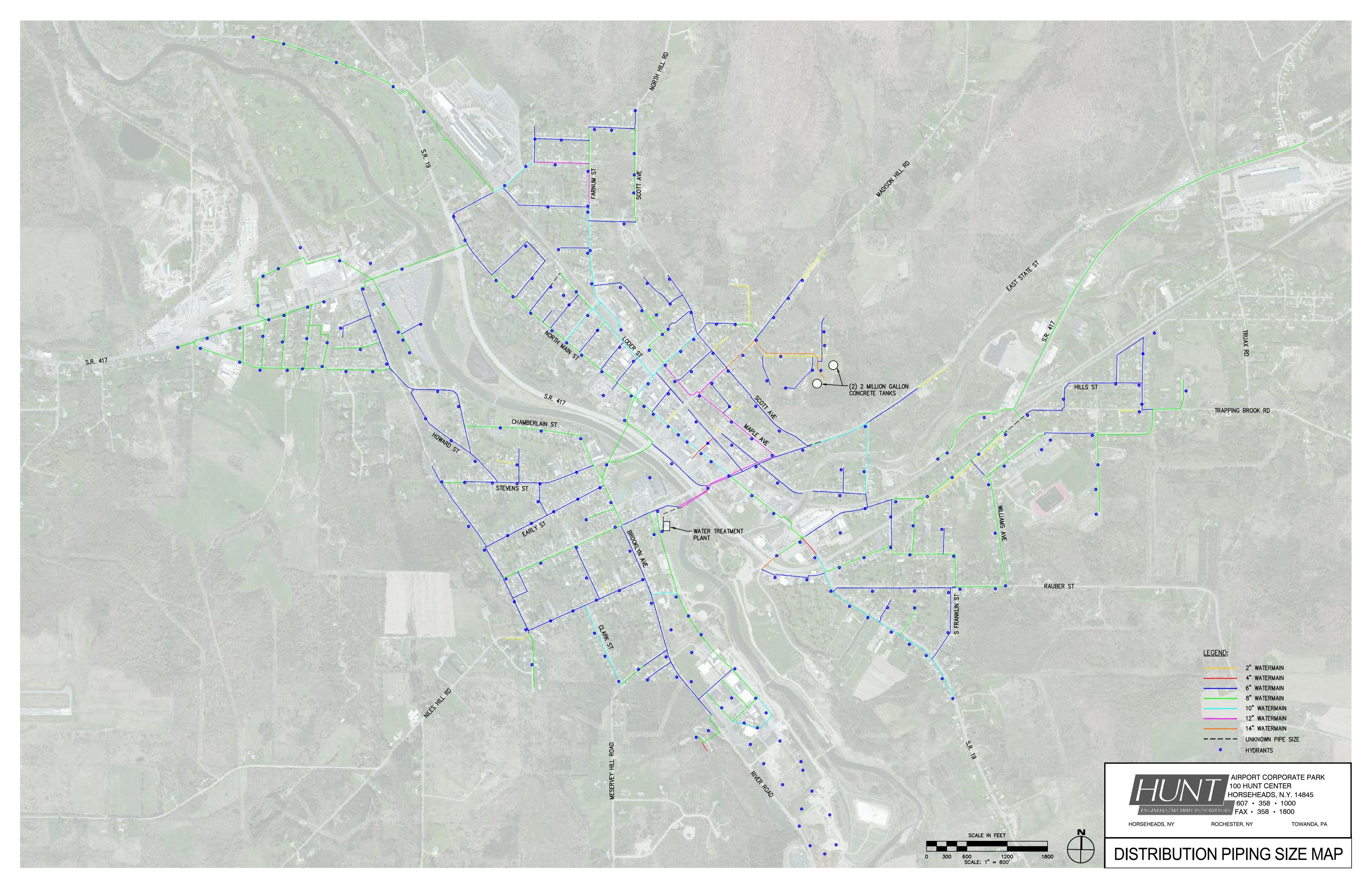
607 • 358 • 1000

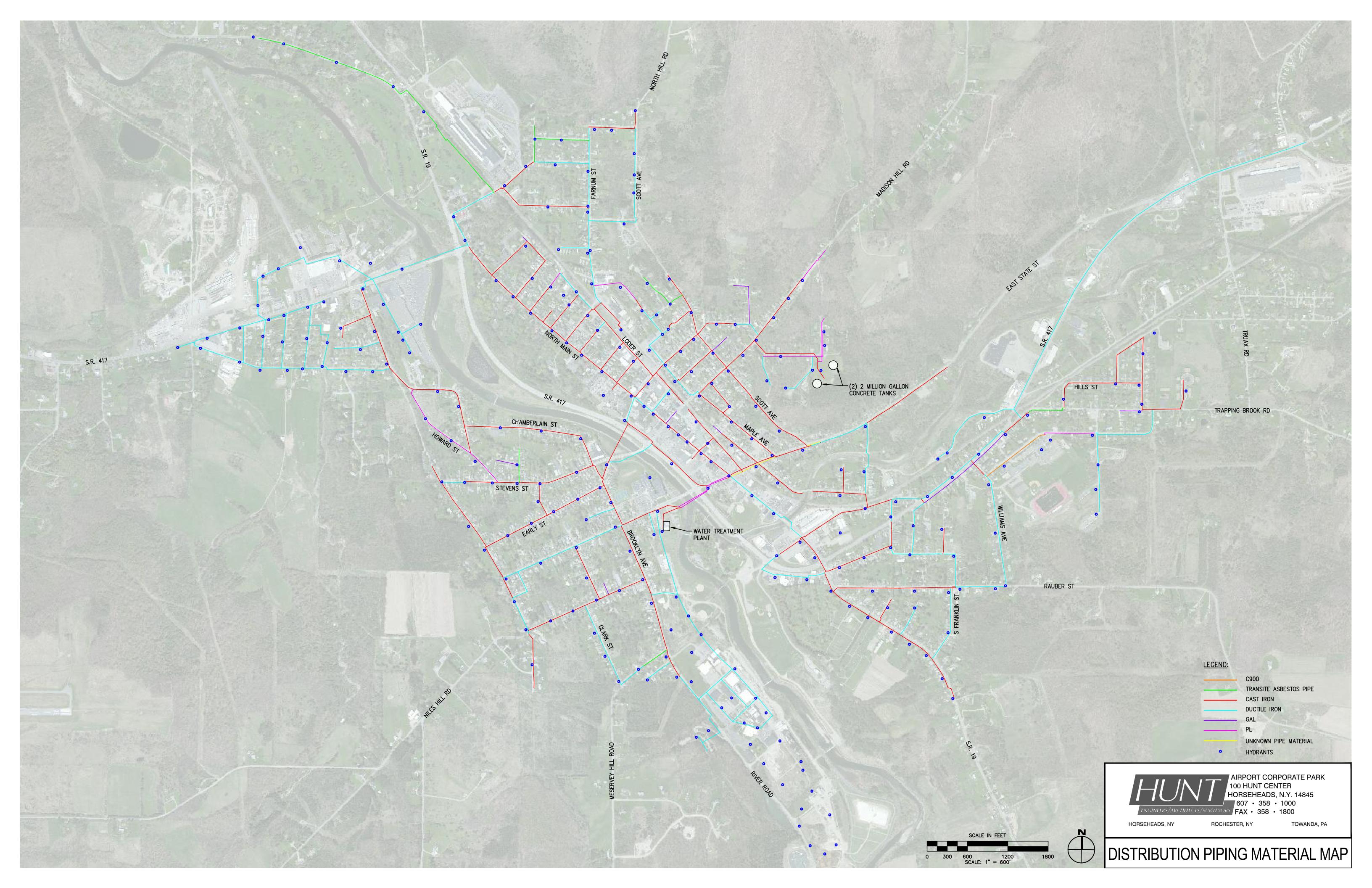
TOWANDA, PA ROCHESTER, NY

PROJECT LOCATION

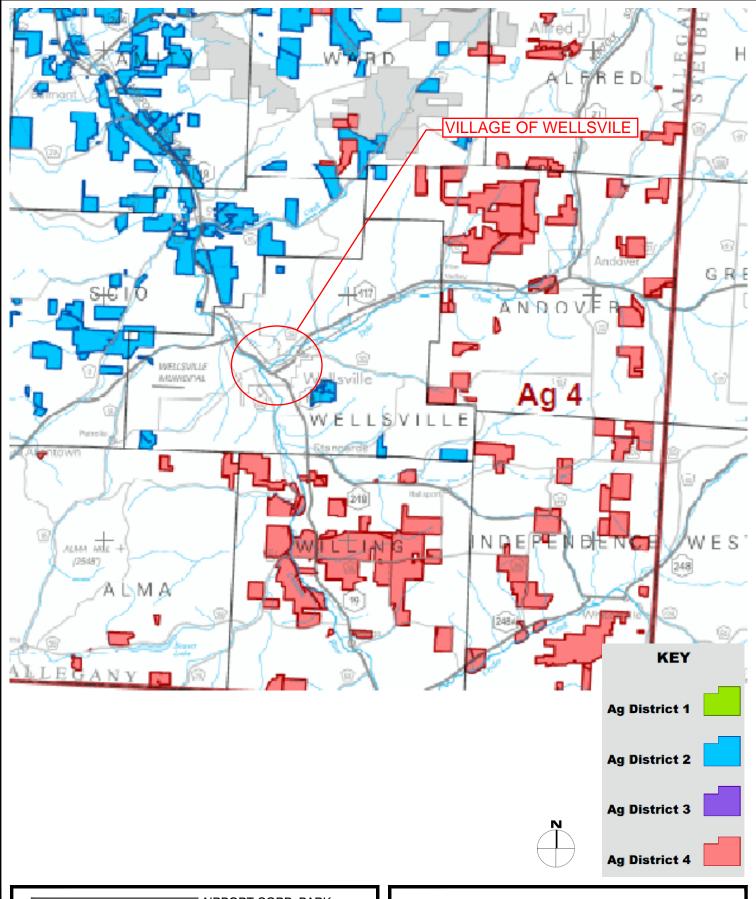
VILLAGE OF WELLSVILLE WATER SYSTEM IMPROVEMENT PROJECT VILLAGE OF WELLSVILLE, ALLEGANY COUNTY, NEW YORK







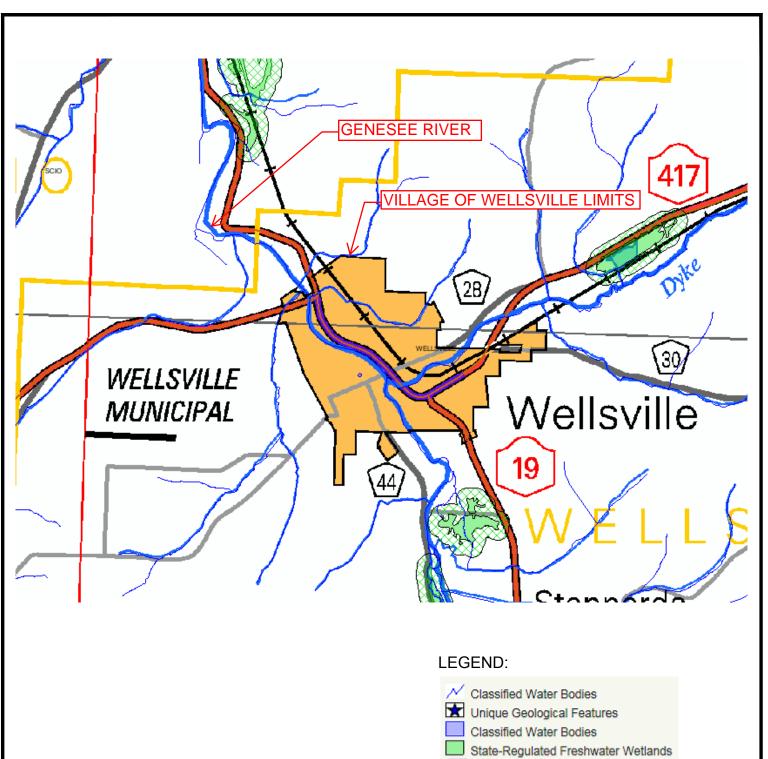
APPENDIX B ENVIRONMENTAL RESOURCES





AGRICULTURAL DISTRICTS

WATER SYSTEM IMPROVEMENTS PROJECT VILLAGE OF WELLSVILLE ALLEGANY COUNTY, NEW YORK



Classified Water Bodies
Unique Geological Features
Classified Water Bodies
State-Regulated Freshwater Wetlands
Wetland Checkzone
Rare Plants and Rare Animals
Significant Natural Communities
Natural Communities Vicinity
Background Map
Adirondack Park Boundary
Counties





HORSEHEADS, NY

AIRPORT CORP. PARK 100 HUNT CENTER HORSEHEADS, NY 14845

607 • 358 • 1000 FAX • 358 • 1800

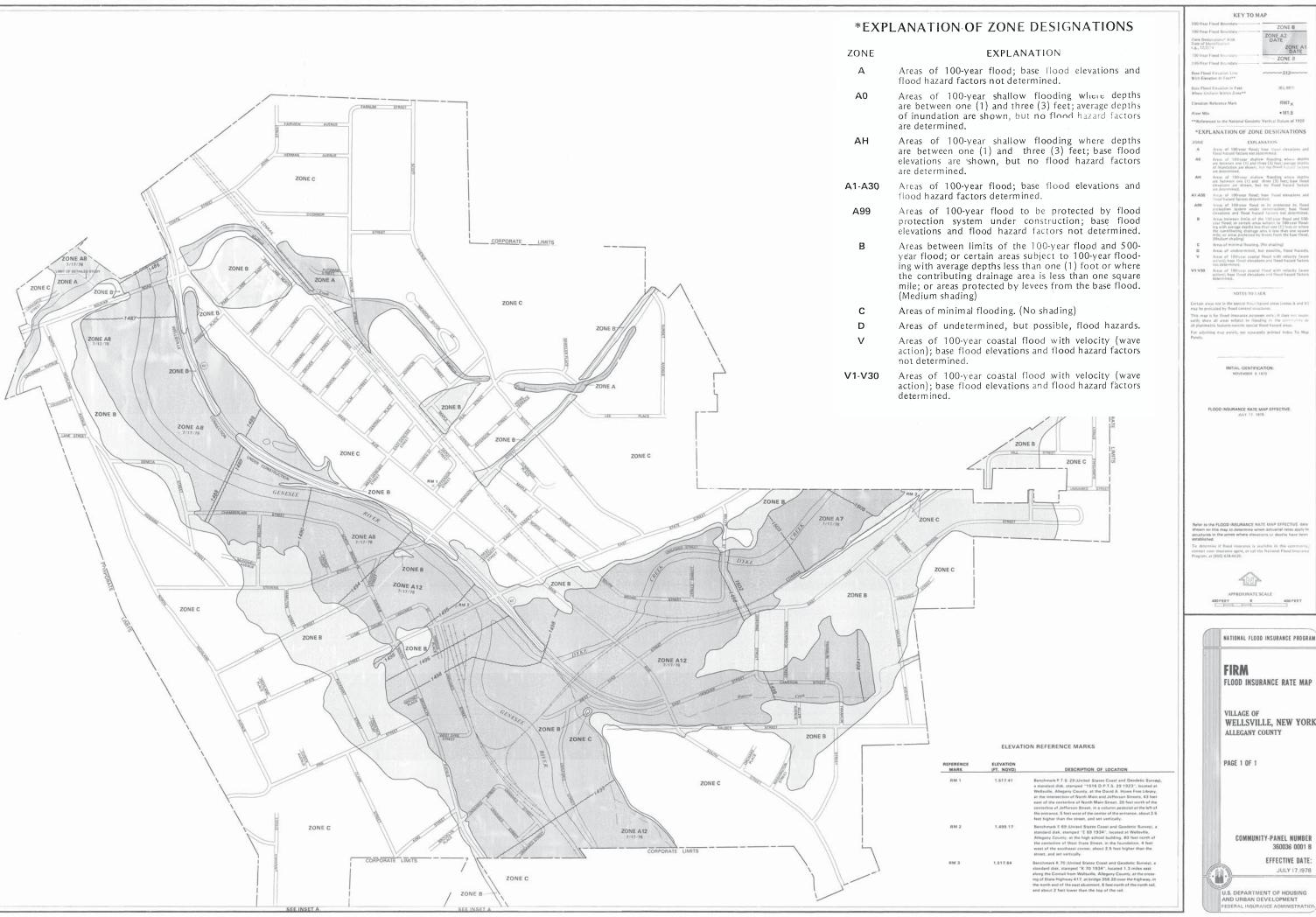
ROCHESTER, NY

TOWANDA, PA

NYSDEC RESOURCE MAP

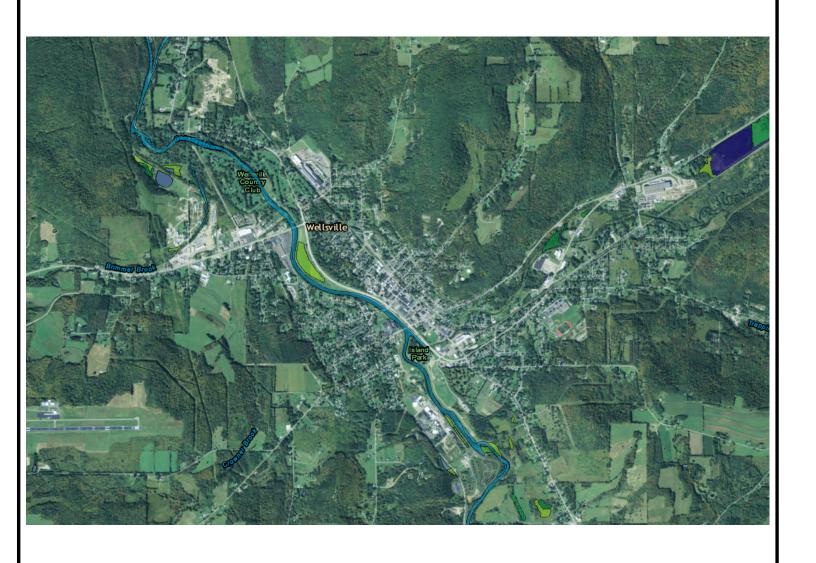
WATER SYSTEM IMPROVEMENTS PROJECT VILLAGE OF WELLSVILLE ALLEGANY COUNTY, NEW YORK

1861-03



ZONE AT ZONE B (61, 682) RM7× • M1.5 *EXPLANATION OF ZONE DESIGNATIONS Areas of 100-year flood; base flood elevations at flood hazard factors not determined. A1-A30 A/ean of 100-year flood; base flood elevations and flood hazard factors determined. Areas of months trooding, (no maining). Areas of modetermine, but possible, flood hazards.
Areas of 100-year casetal flood with velocity (wave scion); but flood floor modetermined.
Areas of 100-year casetal flood with velocity (wave action); base flood elevations and flood hazard factors determined. NOTES TO LISER

> FLOOD INSURANCE RATE MAP VILLAGE OF WELLSVILLE, NEW YORK ALLEGANY COUNTY PAGE 1 OF 1 COMMUNITY-PANEL NUMBER EFFECTIVE DATE: JULY 17 1978



LEGEND:

Estuarine and Marine Deepwater

Estuarine and Marine Wetland

Freshwater Emergent Wetland

Freshwater Forested/Shrub Wetland

Freshwater Pond

Lake

Other

Riverine





HORSEHEADS, NY

AIRPORT CORP. PARK 100 HUNT CENTER HORSEHEADS, NY 14845

607 • 358 • 1000 FAX • 358 • 1800

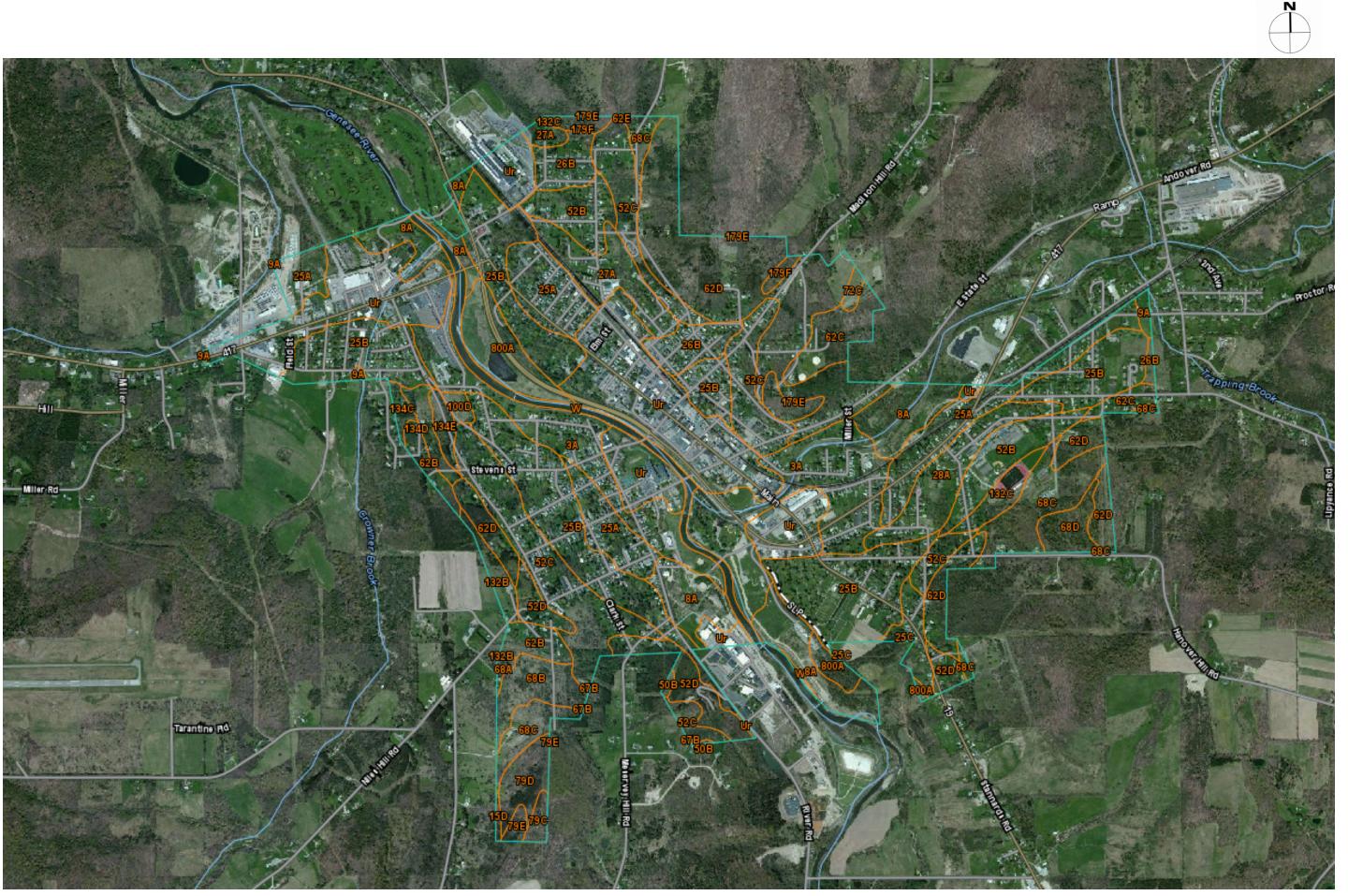
ROCHESTER, NY

TOWANDA, PA

US FISH & WILDLIFE WETLANDS MAP

WATER SYSTEM IMPROVEMENTS PROJECT VILLAGE OF WELLSVILLE ALLEGANY COUNTY, NEW YORK

1861-03



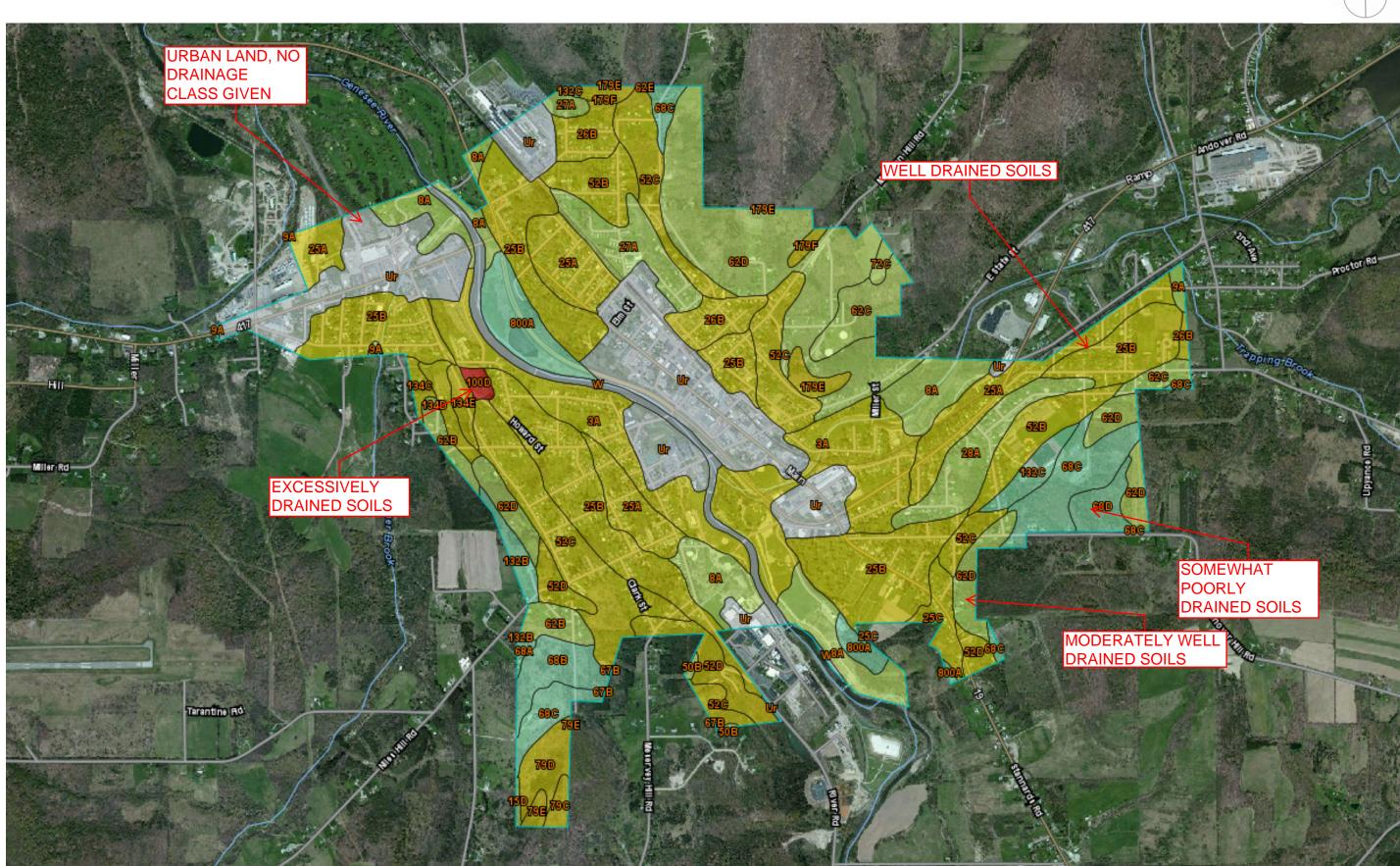
DRAWN BY:
ABW
CHECKED BY:
TKS
DATE:
MAY 2016
SCALE:
NOT TO SCALE
REVISION:

NEW YORK

SOILS MAP
WATER SYSTEM IMPROVEMENTS PROJECT
VILLAGE OF WELLSVILLE

PROJECT NO: 1861-034



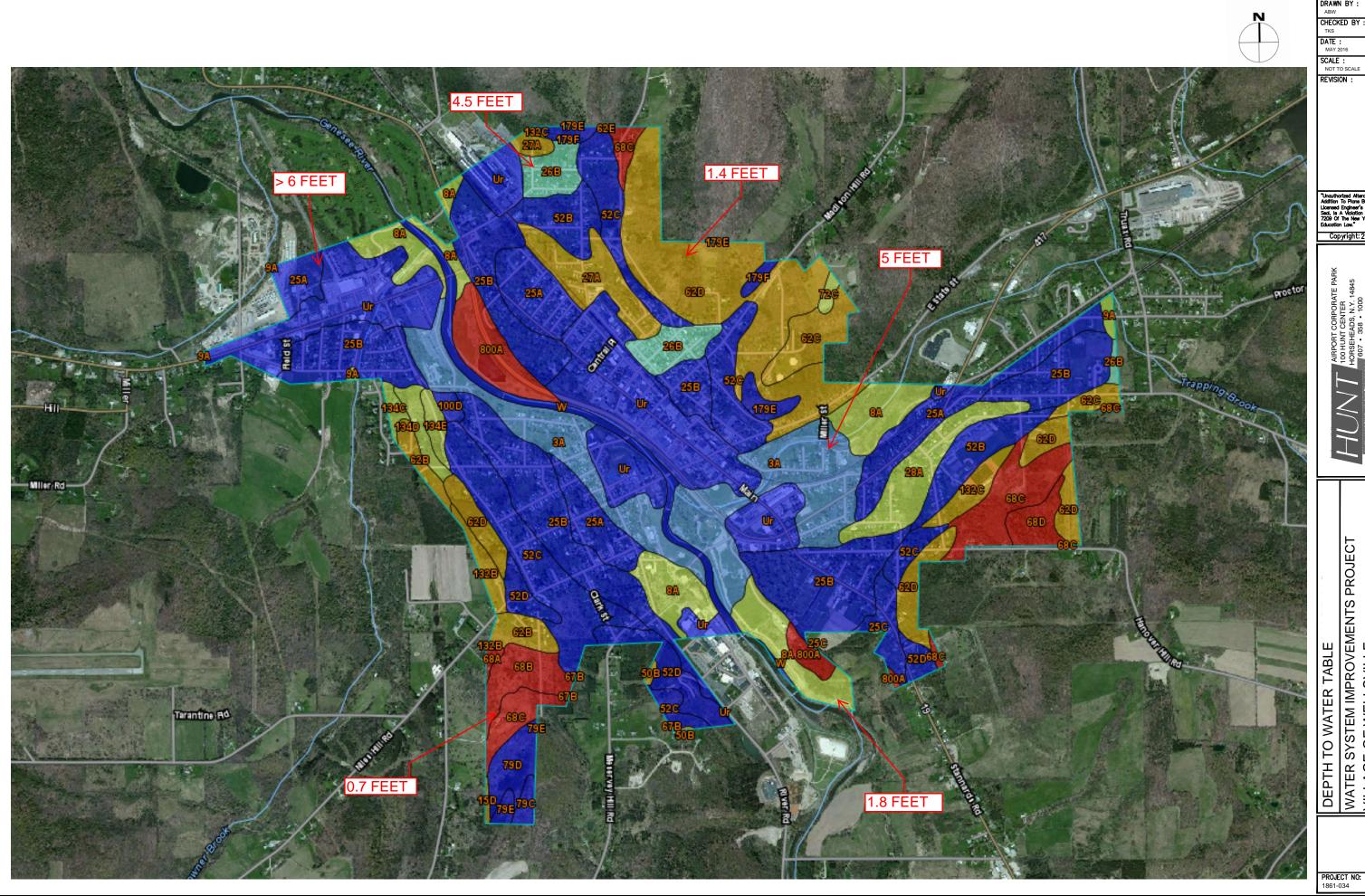


DRAWN BY :

ABW
CHECKED BY:
TKS
DATE:
MAY 2016
SCALE:
NOT TO SCALE
REVISION:

SOIL DRAINAGE CLASS MAP
WATER SYSTEM IMPROVEMENTS PROJECT
VILLAGE OF WELLSVILLE

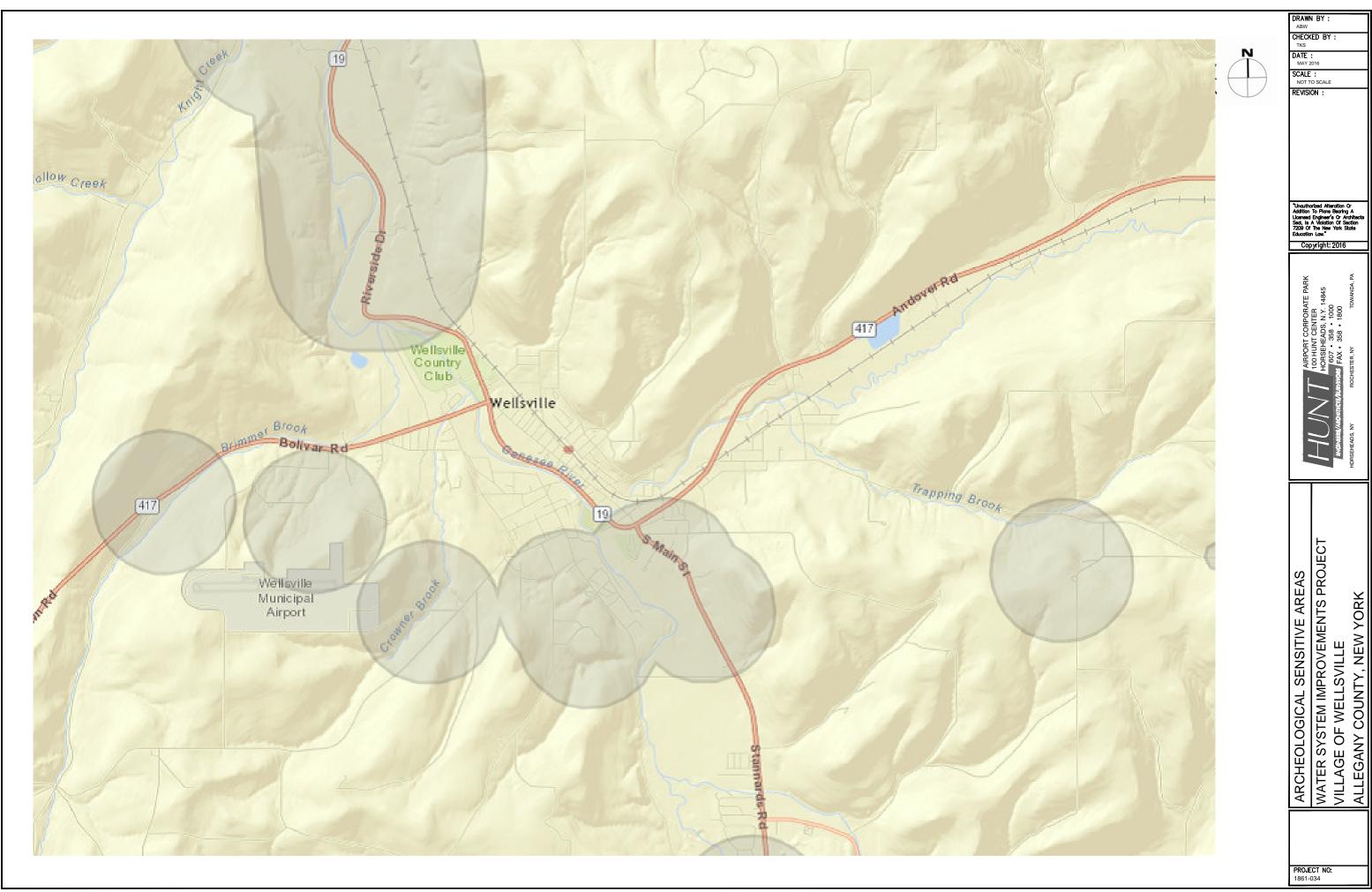
PROJECT NO: 1861-034



DRAWN BY :

DEPTH TO WATER TABLE
WATER SYSTEM IMPROVEMENTS PROJECT
VILLAGE OF WELLSVILLE , NEW YORK

PROJECT NO: 1861-034



APPENDIX C
HYDRAULIC MODEL NARRATIVE & INPUT DATA

Town of Genoa, Cayuga County, New York

Water System Hydraulic Modelling

HUNT 1861-064

January 2017

I. Hydraulic Water Model Development

In order to understand the operation of the Village of Wellsville water system along with the desire to identify system operational shortcomings and identify potential improvement scenarios, a hydraulic water model was developed. The water system was modeled with Innovyze's InfoWater Suite. InfoWater utilizes an enhanced version of the EPANET analysis engine as developed and distributed by the U.S. Environmental Protection Agency, Risk Reduction Engineering Laboratory (EPANET 2000). This software utilizes gradient algorithms and is used to simulate the distribution network with its various loops, elevations, user demands, fittings, pipes of various sizes, age and composition, water storage tanks, water sources and pumping stations. Refer to Appendix A for the hydraulic model network mapping.

Accordingly, various data is required to develop a model of the water systems. A description of the required data utilized in the development of the Village's hydraulic model is presented below. Refer to Appendix C for a complete catalog of the InfoWater input data.

A. Links

Links within the hydraulic model simulate the various watermain found throughout the distribution system. These links convey the flow as it moves from one node to another within the hydraulic model. The model simulates the pipe as a single entity including all segments of watermain and the associated fittings. A link in the model must represent a single stretch of watermain that contains uniform composition, diameter and ages. These values are manually entered by the model user during the construction of said model.

1. Configuration, Diameter, Type and Age

The configuration of the Village's water distribution system includes watermain diameter, composition, and age that were obtained from design mapping supplied by the Village's Water Department.

Refer to Appendix A for a copy of the water system configuration map. The water system map reflects all information gathered with respect to wells, valve locations, hydrant locations, tank locations, booster pump station locations, watermain configuration, and watermain size.

2. Friction Losses

As water flows through the various pipes within a water distribution system, friction losses are incurred that result in a reduction of system pressures (i.e. decrease in hydraulic grade). For purposes of this analysis, the Hazen-Williams equation was used to estimate friction head loss within the distribution system. Utilization of this formula requires the estimation of the Hazen-Williams coefficient, also known as a C-factor, which is a measure of the internal surface roughness. The Village's water system consists primarily of asbestos-cement pipe whose internal roughness remains largely unchanged throughout its age. As a result, the C factor for new asbestos cement pipe (140) was utilized for existing asbestos-cement pipe, plastic pipe and HDPE piping. Ductile Iron watermain typically has a C factor of 120 when

new, however, given the age of much of the ductile iron in the system and the potential for biofilm growth, a C factor of 110 was utilized.

Modelling un-lined cast iron watermain is more complicated as the interior of the watermain can become tuberculated. The level of tuburculation is associated with age and its location within the water system. While independent testing of the watermain is preferred, extensive research has been conducted on piping within systems producing the table below. This table was utilized to assess the internal roughness of un-lined cast iron piping in the Village's system based upon approximate age and location within the system. Piping within looped areas of the system were estimated to have experienced appreciable attack (tuburculation), while severe (attack) tuburculation was used for dead end watermain.

C VALUE RANGES FOR CAST IRON WATERMAIN												
30 years old 4-inch 6-inch 8-inch 10-inch 12-inc												
Trend 1 - Slight Attack	100	106	108.3	110.1	112							
Trend 2 - Moderate Attack	83	90	92.5	94.5	97							
Trend 3 - Appreciable Attack 59 70 71.8 74.4 7.												
Trend 4 - Severe Attack	41	50	53.0	55.7	58							

40 years old	4-inch	6-inch	8-inch	10-inch	12-inch
Trend 1 - Slight Attack	96.5	102.9	104.9	106.8	108.6
Trend 2 - Moderate Attack	78.2	86.2	88.4	90.6	93.0
Trend 3 - Appreciable Attack	55.4	65.8	67.9	70.4	73.8
Trend 4 - Severe Attack	36.9	46.0	49.3	52.0	54.4

50 years old	4-inch	6-inch	8-inch	10-inch	12-inch
Trend 1 - Slight Attack	93.7	100.4	102.3	104.2	106.1
Trend 2 - Moderate Attack	74.8	83.1	85.3	87.6	90.1
Trend 3 - Appreciable Attack	52.5	62.5	64.8	67.4	70.6
Trend 4 - Severe Attack	33.6	42.8	46.1	48.9	51.4

60 years old	4-inch	6-inch	8-inch	10-inch	12-inch
Trend 1 - Slight Attack	90	97	98.5	100.3	102
Trend 2 - Moderate Attack	69	79	80.6	83.0	85
Trend 3 - Appreciable Attack	49	58	60.6	63.1	66
Trend 4 - Severe Attack	30	39	42.5	45.3	48

100 years old	4-inch	6-inch	8-inch	10-inch	12-inch
Trend 1 - Slight Attack	81	89	90.7	92.7	95
Trend 2 - Moderate Attack	61	70	72.4	74.8	78
Trend 3 – Appreciable Attack	40	49	51.8	54.4	57
Trend 4 – Severe Attack	21	30	33.3	36.0	39

3. Minor Losses

Minor losses are defined as head loss incurred at fittings and other appurtenances within a water distribution system (i.e. valves, etc.). These minor losses are a direct result of turbulence within the flow of water as it moves through the various fittings and appurtenances. Typically, with older water distribution systems, these losses are negligible

compared to the head losses due to friction. Furthermore, head losses provided for a particular stretch of watermain may not be constant over time depending upon the flow pattern. Therefore, minor losses were not incorporated into the model and would have had negligible impact on the overall model operation.

B. Nodes

The water model consists of various types of nodal elements that commonly include reservoirs, tanks, valves and interconnections of pipes (junctions). Nodes interconnected together with the previously described links form a complete network. It is at the nodes where critical operating and boundary conditions are entered into the model as described briefly below.

1. Junctions

Junction nodes are points placed at the intersection of two or more pipes, at points of water consumption, and at points where pipe attributes (i.e. diameter composition) change. Subsequent to the creation of a junction node in a hydraulic model, the ground elevation at the newly created junction location must be entered. If demands occur within the hydraulic model the water demand is also entered at a junction near to the point of consumption. If the hydraulic model is to be used in simulating the water system for extended periods, a stepwise demand pattern must be applied to the entered demand describing how said demand occurs through a 24-hour period. The data entry requirements are described in greater detail below.

a. Elevations

Ground elevations are essential data for the hydraulic model as it influences system pressures at a given location. Elevations for the junctions were obtained from existing topography from the original system design along with elevational data obtained from the NYSGIS Clearinghouse.

b. Water Demand

Water demands (and their associated fluctuations over time) impact pressure, available flow, direction of flow, and water age within the Village's water distribution system. As such, the allocation of the overall water use across the distribution system is an important component of the development of the hydraulic model. The goal is to generally match actual water use across the system. While it is not feasible or practical to enter each individual water demand record throughout the system, the large water users were identified and their demand entered into the model. It is these large users that dictate the general flow of water throughout the distribution system. Twenty of the top water users were easily identified, below which, the typical demand mirrored that of a typical residential user. The top identified 20 users and their listed water average demand are:

User	User Address	Usage Type	Usage Hours	Usage (gpm)
Dresser Rand	37 Coats Street	Comm/Ind	24	31.8
Argentieri Brothers	50 West Hanover St.	Comm/Ind	8	29.0
Arvos Group	121 South Main St.	Comm/Ind	8	16.5
Wellsville Manor Care Ctr.	4100 Bolivar Road	Res	24	11.5
Wellsville Central School	School Street	Inst	8	10.9
Wellsville High School	126 West State St.	Inst	8	10.8
Jones Memorial Hospital	191 North Main St.	Comm/Ind	24	8.4
Wellsville Manor Hills	4192 Bolivar Road	Res	24	7.4

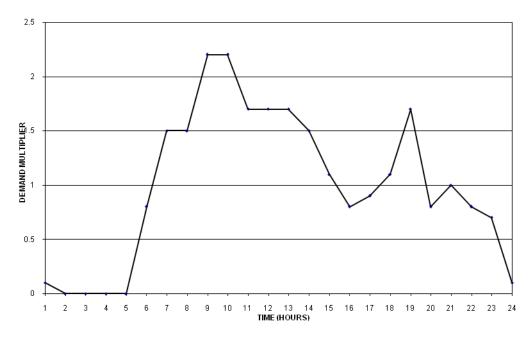
Willey, Phil	56 1/2 North Main St.	Comm/Ind	8	6.6
HRNC, LLC	160 Seneca Street	Res	24	6.3
Wellsville Lazer Wash Inc	2925 Airway Drive	Comm/Ind	16	5.8
SUNY Alfred, Food Serv.	South Brooklyn Ave.	Comm/Ind	12	5.4
Texas Hot	132 North Main Street	Comm/Ind	8	4.8
Indus Hotels Inc	30 West Dyke StreeT	Comm/Ind	16	4.1
Hanover Street Laundry	18 West Hanover St.	Comm/Ind	16	3.2
Kmart Store #7677	121 Bolivar Road	Comm/Ind	12	2.9
Rub-A-Dub Laundry	84 1/2 North Main St.	Comm/Ind	16	2.4
Scio Water Department	Riverside Drive	Res	24	2.4
TOPS Markets Inc- 249	111 Bolivar Road	Comm/Ind	12	2.2
Allegany County ARC	196 East Dyke St.	Res	24	1.1

The above listed large users consist of 159,214 gallons per day. The entire village average daily water usage is 713,967 gallons per day. The remainder of the usage is primarily residential and light commercial in nature and was distributed evenly throughout the distribution system in the areas containing residential and commercial land use.

c. Use Pattern

To model the fluctuation of water consumption of the various users over time, a number of stepwise demand patterns were developed and applied to each entered demand. The stepwise pattern mimics the times of operation identified above. For example, a typical stepwise demand pattern was utilized to simulate residential water demands over the course of a 24-hour period in absence of actual metered hourly water use data. The residential stepwise demand pattern utilized is shown graphically as follows:

RESIDENTIAL STEPWISE DEMAND PATTERN



Multiplying an average daily (i.e. baseline) demand by the dimensionless demand multiplier generates a water demand pattern at a point in time. For instance, at 8:00 AM the consumption for a single residence having an average demand of 1 gpm is calculated as follows:

Demand at 8AM = Entered Average Daily Demand x Demand Multiplier

Demand at 8AM = 1 gpm x 1.5

Demand at 8AM = 1.5 gpm

2. Storage Tanks & Reservoirs

A water storage facility can be modeled as a storage tank or a reservoir. A reservoir is used to simulate a water supply for the system whereas a water storage tank is used mainly for the purposes of temporary storage having a finite volume. Reservoirs on the other hand, have unlimited volume and are generally used to model a supply source such as a lake or groundwater supply.

a. Type

The Village of Wellsville water system utilizes two cylindrical concrete water storage tanks. Cylindrical tanks are typically ground level water storage tanks that require the input of a diameter, a base elevation, a minimum water surface level above the base elevation, and a maximum water surface level above the base elevation.

b. Diameter, Elevations, Levels & Curves

Overflow elevations and base elevations for the water storage facilities are important data in the creation of a hydraulic model of the Villages water system. Water surface elevations within storage facilities greatly influence hydraulic grades across a water system as well as water age. Each tank has the same finished floor and overflow elevation of 1720 ft. and 1740 ft. respectively. Each tank is also 131 feet in diameter.

3. Surface Water Supply & Finished Water Pumps

The Village of Wellsville withdrawals water from the Genesee River, south of the State Street Bridge. Subsequent to the treatment process, finished water enters the clearwell where it is withdrawn by some combination of three finished water booster pumps. Two 125 Hp and one 60 Hp pump finished water from the clearwell to the two 2MG reservoirs. One 125 Hp pump runs at a time and is operated manually. If the clearwell reaches a high level, the 60Hp pump becomes operational. Therefore, there are two flow/pressure regimes associated with plant operation; first one 125 Hp pump operating and second both a 125 Hp and a 60 Hp pump operating.

It is understood that the 125 Hp finished water pumps are operated manually but the water treatment plant operator with the exception of the 60 HP pump. The 60 Hp pump becomes operational when an elevated water level in the clearwell occurs. The total discharge with both pumps under various operation conditions is as follows:

Pump	Flow (gpm)
125 Hp Only	1350
60 Hp Only	700
125 HP & 60 Hp Together	1700

4. Booster Pumping Stations

The Village maintains a single booster pump station to service residential parcels on Sunset Avenue. Sunset Avenue, located near the existing water storage tanks, previously had relatively low operational pressures until a small duplex booster pump station was installed. This pump station consists of two alternating 60 gpm, 3Hp pumps and a small 45 gallon hydropneumatic tank. As such this pump station is designed to maintain pressure in a range of 30-50 psi in the watermain along Sunset Avenue.

While the program does not provide a hydropneumatic tank option, this was modelled using a ground level cylindrical tank. The tank height is dictated by the maximum pressure (pump off) of 50 psi or 115 feet (equal to 50 psi x 2.31 ft/psi). The pump is called to operate at 30 psi or 69.3 feet (30 psi x 2.31 ft/psi). The resulting diameter of a tank having a volume of 45 gallons at 115 feet in height is 0.26 feet.

Input Data - Junction Demands

input Data - Junction Der	Harius					1_						
* BASE *	4	ID (Char)	Demand 1 Pattern 1 (gpm) (Char)	Demand 2 (gpm)	Pattern 2 (Char)	Demand 3	Pattern 3 (Char)	Demand 4 Patte (gpm) (Ch		Pattern 5 (Char)	Demand 6 (gpm)	Pattern 6 (Char)
1		J10	0.00 RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00 16-H	O.00	24-HOUR	0.13	Unaccounted
2		J12	RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
3		J14	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
4		J16	2.40 RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00 16-H	O.00	24-HOUR	0.13	Unaccounted
5		J18	RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
6		J20	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
7		J22	1.00 RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00 16-H	0.00	24-HOUR	0.13	Unaccounted
8		J24	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
9		J26	0.40 RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00 16-H	O.00	24-HOUR		Unaccounted
10		J28	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR		Unaccounted
11		J30	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
12		J32	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
13		J36	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
14		J38	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR		Unaccounted
15		J42	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR		Unaccounted
16		J44	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
17		J46	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR		Unaccounted
18		J48	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
19		J50	RESIDENTIAL	3.00	8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
20		J52	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR	0.13	Unaccounted
21		J54	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
22		J56	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
23		J58	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted
24		J60	RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
25		J62	RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR		Unaccounted
26		J64	RESIDENTIAL	3.00	8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
27		J66	RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
28		J72	RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
29		J74	0.00 RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00 16-H		24-HOUR	0.13	Unaccounted
30		J76	RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR	0.13	Unaccounted
31		J78	0.00 RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00 16-H		24-HOUR		Unaccounted
32		J80	RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
33		J82	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
34		J84	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
35		J86	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H		24-HOUR		Unaccounted
36		J90	1.00 RESIDENTIAL		8-HOUR		12-HOUR	16-H	OUR	24-HOUR	0.13	Unaccounted

Input Data - Junction Demands

* BASE *	ID	Demand 1	Pattern 1	Demand 2				Demand 4					
Bride	(Char)	(gpm)	(Char)	(gpm)	(Char)	3 .	(Char)	(gpm)	(Char)	(gpm)	(Char)	(gpm)	(Char)
37	J92		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
38	J96	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
39	J98	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
40	J104	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
41	J106		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
42	J108		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	31.80	24-HOUR	0.13	Unaccounted
43	J114		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
44	J116	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
45	J118	0.40	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR		Unaccounted
46	J120		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
47	J122		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
48	J124	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
49	J126	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
50	J128	1.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR		Unaccounted
51	J130	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
52	J132	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
53	J134	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
54	J136	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
55	J138	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
56	J140	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
57	J142	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR		Unaccounted
58	J146	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
59	J148		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
60	J150	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
61	J152	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
62	J156	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
63	J158	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
64	J160	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
65	J166	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
66	J168	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
67	J176	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
68	J180	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
69	J182		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
70	J186	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
71	J188	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
72	J192	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 1		Demand 2		Demand 3				Demand 5 Pattern 5		
70		(Char)	(gpm)	(Char)	(gpm)	(Char)	, ,	(Char)	(gpm)	(Char)	(gpm) (Char)	(gpm)	(Char)
73	느늗	J194		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
74		J196		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
75		J200		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
76		J202	0.40	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
77		J208		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
78		J210		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
79		J212		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
80		J214		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
81		J218		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
82		J224		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
83		J226		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
84		J228	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
85		J230	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
86		J232	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
87		J234	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
88		J238	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
89		J240	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
90		J242	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
91		J244	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
92		J248		RESIDENTIAL	3.00	8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
93		J250	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
94		J252	2.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
95		J254	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
96		J256	0.40	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
97		J260	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
98		J262	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
99		J266	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
100		J274	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
101		J276	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
102		J278	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
103		J280	0.00	RESIDENTIAL	16.50	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
104		J288		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	0.00 24-HOUR		Unaccounted
105		J290		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	0.00 24-HOUR		Unaccounted
106		J292		RESIDENTIAL	- 30	8-HOUR	1	12-HOUR		16-HOUR	24-HOUR		Unaccounted
107		J294		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
108		J296		RESIDENTIAL		8-HOUR	1	12-HOUR		16-HOUR	24-HOUR		Unaccounted
100		0200	1.50								21110011	0.10	2

Input Data - Junction Demands

Dr	SE *		ID (Char)	Demand 1	Pattern 1	Demand 2		Demand 3				Demand 5 Pattern 5		Pattern 6
			(Char)	(gpm)	(Char)	(gpm)	(Char)	, ,	(Char)	(gpm)	(Char)	(gpm) (Char)	(gpm)	(Char)
	09	H	J298		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	10	ᆜ	J300		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	11	Щ	J304		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	12	Щ	J306		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	13	ш	J308		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	14		J312		RESIDENTIAL	10.90	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
	15		J314		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
1	16		J320		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
1	17		J322		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
1	18		J324		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	19		J326		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	20		J328	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	21		J330	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	22		J334	2.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
1	23		J336	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	24		J340	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	25		J346	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	26		J348	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	27		J350	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	28		J352	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	29		J356	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	30		J358	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	31		J360	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	32		J362	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	33		J364	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	34		J366	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
1	35		J368	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	36		J370	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	37		J372	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	38		J374	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	39		J376	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
1	40		J378	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
	41		J380		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	42		J382		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
	43		J384		RESIDENTIAL	3.00	8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
1	44		J386	0.00	RESIDENTIAL	4.80	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 1 (gpm)	Pattern 1 (Char)	Demand 2 (gpm)	Pattern 2 (Char)	Demand 3	Pattern 3 (Char)	Demand 4 (gpm)	Pattern 4 (Char)	Demand 5 Pattern 5 (Char)	Demand 6 (gpm)	Pattern 6 (Char)
145		J388		RESIDENTIAL		8-HOUR	, , ,	12-HOUR	(92111)	16-HOUR	24-HOUR		Unaccounted
146	H	J390		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
147	H	J392		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
148	H	J392	0.60	RESIDENTIAL		8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
149	-	J398	0.00	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	24-HOUR		Unaccounted
150		J400	0.60	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
151		J402	0.00	RESIDENTIAL		8-HOUR	0.00	12-HOUR	0.00	16-HOUR	24-HOUR		Unaccounted
152		J406	0.60	RESIDENTIAL		8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
153		J408	0.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	24-HOUR		Unaccounted
154	$\overline{}$	J410		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
155		J414		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
156		J416	0.00	RESIDENTIAL	10.80	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
157		J420		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
158		J422		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
159		J428		RESIDENTIAL	3.00	8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
160		J430		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
161		J432	1.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
162		J434		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
163		J436		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
164		J440		RESIDENTIAL	3.00	8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
165		J442		RESIDENTIAL	3.00	8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
166		J444	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
167		J446	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
168		J448	0.00	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
169		J450	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
170		J452	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
171		J454	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
172		J456	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
173		J458	0.00	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
174		J460	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
175		J462	0.00	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
176		J464		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	5.80	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
177		J466		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
178		J468	0.00	RESIDENTIAL	0.00	8-HOUR	2.90	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
179		J470		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
180		J472	0.00	RESIDENTIAL	0.00	8-HOUR	2.20	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted

Input Data - Junction Demands

* BASE *	ID		Pattern 1		Pattern 2			Demand 4	Pattern 4		Pattern 5		
BAGE	(Cha	, (01 /	(Char)	(gpm)	(Char)	, 3	(Char)	(gpm)	(Char)	(gpm)	(Char)	(gpm)	(Char)
181	J47	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
182	J47	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
183	J47	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
184	J48	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
185	J48	2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
186	J48	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
187	J48	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
188	J48	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
189	J49	6.30	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
190	J49	2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
191	J49	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
192	J50	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
193	J50	2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
194	J50	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
195	J50	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
196	J51	2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
197	J51	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
198	J51	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
199	J51	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
200	J52	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
201	J52	2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
202	J52	4 1.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
203	J52	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
204	J53	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
205	J53	2 0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
206	J53	4 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
207	J53	0.40	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
208	J53	3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
209		2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
210		4 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
211		3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
212		3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
213		1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
214		2 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted
215	J55	4 0.40	RESIDENTIAL	0.00	8-HOUR	5.40	12-HOUR	0.00	16-HOUR	0.00	24-HOUR	0.13	Unaccounted
216		3 1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR		24-HOUR	0.13	Unaccounted

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 1				Demand 3				Demand 5 Pattern 5		
047		(Char)	(gpm)	(Char)	(gpm)	(Char)	, , ,	(Char)	(gpm)	(Char) 16-HOUR	(gpm) (Char)	(gpm)	(Char)
217	H	J558		RESIDENTIAL		8-HOUR		12-HOUR			24-HOUR		Unaccounted
218	H	J560		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
219	\vdash	J562		RESIDENTIAL	2.22	8-HOUR	2.22	12-HOUR	2.22	16-HOUR	24-HOUR		Unaccounted
220	브	J564		RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR		Unaccounted
221	Щ	J566		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
222	Ш	J570		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
223		J572		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
224		J578		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
225		J580		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
226		J582		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
227		J584		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR		Unaccounted
228		J586	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
229		J588	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
230		J590	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
231		J592	1.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
232		J594		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
233		J600	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
234		J602	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
235		J604	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
236		J606	2.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
237		J608	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
238		J610	1.00	RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
239		J612		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
240		J614		RESIDENTIAL		8-HOUR		12-HOUR		16-HOUR	24-HOUR	0.13	Unaccounted
241		J616	11.50	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
242		J618	0.60	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	0.00	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
243		J620	0.00	RESIDENTIAL	0.00	8-HOUR	0.00	12-HOUR	3.20	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
244		J622	0.00	RESIDENTIAL	3.00	8-HOUR	0.00	12-HOUR	2.40	16-HOUR	0.00 24-HOUR	0.13	Unaccounted
245		J628	1.60	RESIDENTIAL	0.00		0.00		0.00		0.00	0.00	
246		J630	1.60	RESIDENTIAL	0.00		0.00		0.00		0.00	0.00	
247		J632	1.60	RESIDENTIAL	0.00		0.00		0.00		0.00	0.00	
248		J634		RESIDENTIAL	0.00		0.00		0.00		0.00	0.00	
249		J636		RESIDENTIAL									
250		J638		RESIDENTIAL									
251		J640		RESIDENTIAL									
252		J642		RESIDENTIAL									
202		0072											<u> </u>

* BASE *	(0	ID Char)	Demand 1 (gpm)	Pattern 1 (Char)	Demand 2 (gpm)	Pattern 2 (Char)	Demand 3	Pattern 3 (Char)	Demand 4 (gpm)	Pattern 4 (Char)	Demand 5 (gpm)	Pattern 5 (Char)	Demand 6 (gpm)	Pattern 6 (Char)
253		J644		RESIDENTIAL										
254		J646		RESIDENTIAL										
255		J648		RESIDENTIAL										
256		J650		RESIDENTIAL										
257		J652		RESIDENTIAL										
258		J654		RESIDENTIAL										
259		J656		RESIDENTIAL										
260		J658		RESIDENTIAL										
261		J660	0.80	RESIDENTIAL	0.00		0.00		0.00		0.00		0.00	
262		J662	0.40	RESIDENTIAL	0.00		0.00		0.00		0.00		0.00	
263		J664	0.60	RESIDENTIAL	0.00		0.00		0.00		0.00		0.00	
264		J666	0.60	RESIDENTIAL	0.00		0.00		0.00		0.00		0.00	
265		J668	0.00	RESIDENTIAL	0.00		0.00		0.00		0.00		0.00	
266		J670		RESIDENTIAL										

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
4				(Cital)		(Criai)	
1		J10	0.00		0.00		0.00
2	<u> </u>	J12					
3	느늗	J14	0.00		0.00		0.00
4	느늗	J16	0.00		0.00		0.00
5	느늗	J18					
6	느늗	J20	0.00		0.00		0.00
7	<u> </u>	J22	0.00		0.00		0.00
8		J24			2.00		
9		J26	0.00		0.00		0.00
10		J28					
11		J30					
12		J32					
13		J36					
14		J38					
15	ᆜ	J42					
16		J44					
17		J46					
18		J48					
19		J50					
20		J52					
21		J54					
22		J56					
23		J58					
24		J60					
25		J62					
26		J64					
27		J66					
28		J72					
29		J74	0.00		0.00		0.00
30		J76					
31		J78	0.00		0.00		0.00
32		J80					
33		J82					
34		J84					
35		J86					
36		J90					

Input Data - Junction Demands

* BASE *	ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
37	J92	0.00	(Onal)	0.00	(Onal)	0.00
38	J96	0.00		0.00		0.00
39	J98					
40	J104					
41	J106					
42	J108	0.00		0.00		0.00
43	J114					
44	J116					
45	J118	0.00		0.00		0.00
46	J120					
47	J122					
48	J124					
49	J126					
50	J128	0.00		0.00		0.00
51	J130					
52	J132					
53	J134					
54	J136	0.00		0.00		0.00
55	J138					
56	J140					
57	J142					
58	J146	0.00		0.00		0.00
59	J148					
60	J150					
61	J152					
62	J156					
63	J158					
64	J160					
65	J166					
66	J168					
67	J176					
68	J180					
69	J182					
70	J186					
71	J188					
72	J192					

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
73		J194	(дрііі)	(Criai)	(дрііі)	(Onal)	(gpiii)
74	-	J194 J196					
75		J200					
76		J200 J202	0.00		0.00		0.00
77	-	J202 J208	0.00		0.00		0.00
78	-	J210	0.00		0.00		0.00
79	-	J210 J212	0.00		0.00		0.00
80	-	J212 J214					
81	-	J218					
82	-	J224					
83	-	J224 J226					
84		J228					
85		J230					
86	H	J232					
87	H	J234	0.00		0.00		0.00
88	H	J238	0.00		0.00		0.00
89	H	J240					
90	H	J242					
91		J244					
92		J248					
93		J250					
94	Ë	J252	0.00		0.00		0.00
95	Ë	J254	0.00		5.55		0.00
96	Ë	J256	0.00		0.00		0.00
97	Ë	J260	0.00		5.55		0.00
98	Ė	J262					
99	Ė	J266					
100		J274					
101		J276					
102		J278					
103		J280	0.00		0.00		0.00
104		J288	0.00		0.00		0.00
105		J290	0.00		0.00		0.00
106	Г	J292					
107		J294	0.00		0.00		0.00
108	Г	J296					

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
100			(дріп)	(Criai)	(дрін)	(Criai)	(gpiii)
109 110		J298 J300					
		J304					
111 112		J304 J306					
113		J308	0.00		0.00		0.00
114	-	J312	0.00		0.00		0.00
115	-	J314	0.00		0.00		0.00
116	<u> </u>	J320	0.00		0.00		0.00
117	<u> </u>	J322					
118		J324					
119		J326					
120		J328					
121		J330	2.22				
122		J334	0.00		0.00		0.00
123		J336					
124		J340					
125		J346					
126		J348					
127		J350					
128		J352					
129		J356					
130		J358					
131		J360					
132		J362					
133		J364					
134		J366	0.00		0.00		0.00
135		J368					
136		J370					
137		J372					
138		J374					
139		J376					
140		J378					
141		J380					
142		J382					
143		J384					
144		J386	0.00		0.00		0.00

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
1.45			(gpiii)	(Onai)	(gpiii)	(Criai)	(дрііі)
145		J388 J390					
146 147	-	J390 J392					
147		J392 J396	0.00		0.00		0.00
			0.00		0.00		0.00
149		J398	0.00		0.00		0.00
150		J400	0.00		0.00		0.00
151		J402	0.00		0.00		0.00
152		J406	0.00		0.00		0.00
153		J408					
154		J410					
155		J414	0.00		2.22		0.00
156		J416	0.00		0.00		0.00
157		J420					
158		J422					
159		J428					
160		J430					
161		J432	0.00		0.00		0.00
162		J434					
163		J436					
164		J440					
165		J442					
166		J444					
167		J446					
168		J448	0.00		0.00		0.00
169		J450					
170		J452					
171		J454					
172		J456					
173		J458	0.00		0.00		0.00
174		J460					
175		J462	0.00		0.00		0.00
176		J464	0.00		0.00		0.00
177		J466	0.00		0.00		0.00
178		J468	0.00		0.00		0.00
179		J470					
180		J472	0.00		0.00		0.00

Input Data - Junction Demands

* BASE *		ID (Charl)	Demand 7	Pattern 7	Demand 8	Pattern 8	Demand 9
		(Char)	(gpm)	(Char)	(gpm)	(Char)	(gpm)
181		J474					
182		J476					
183		J478					
184		J480					
185		J482					
186		J484					
187		J486					
188		J488	2.22		2 22		
189		J490	0.00		0.00		0.00
190		J492					
191		J498					
192		J500					
193		J502					
194		J506	0.00		0.00		0.00
195	ᆜᆜ	J508					
196		J512					
197		J514					
198		J516					
199		J518					
200		J520					
201		J522					
202		J524	0.00		0.00		0.00
203		J526					
204		J530					
205		J532	0.00		0.00		0.00
206		J534					
207		J536	0.00		0.00		0.00
208		J538					
209		J542					
210		J544					
211		J546					
212		J548					
213		J550					
214		J552					
215		J554	0.00		0.00		0.00
216		J556					

Input Data - Junction Demands

* BASE *		ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
217		J558	(дрін)	(Onai)	(gpiii)	(Criai)	(урпі)
218	-	J560					
219	-	J562					
220	-	J564	0.00		0.00		0.00
221	-	J566	0.00		0.00		0.00
222	+	J570					
223	+	J572					
224	-	J578					
225	-	J580					
226	H	J582					
227	-	J584					
228	-	J586					
229	-	J588					
230	-	J590					
231	-	J592	0.00		0.00		0.00
232		J594	5.55		3.55		0.00
233		J600					
234	Ė	J602					
235	Ė	J604					
236		J606	0.00		0.00		0.00
237		J608					
238		J610					
239		J612					
240		J614					
241		J616	0.00		0.00		0.00
242		J618	0.00		0.00		0.00
243		J620	0.00		0.00		0.00
244		J622	0.00		0.00		0.00
245		J628	0.00		0.00		0.00
246		J630	0.00		0.00		0.00
247		J632	0.00		0.00		0.00
248		J634	0.00		0.00		0.00
249		J636					
250		J638					
251		J640					
252		J642					

* BASE *	ID (Char)	Demand 7 (gpm)	Pattern 7 (Char)	Demand 8 (gpm)	Pattern 8 (Char)	Demand 9 (gpm)
253	J644					
254	J646					
255	J648					
256	J650					
257	J652					
258	J654					
259	J656					
260	J658					
261	J660	0.00		0.00		0.00
262	J662	0.00		0.00		0.00
263	J664	0.00		0.00		0.00
264	J666	0.00		0.00		0.00
265	J668	0.00		0.00		0.00
266	J670					

Input Data - Junction Demands

* BASE *	ID	Pattern 9	Demand 10	Pattern 10
BASE	(Char)	(Char)	(gpm)	(Char)
1	☐ J10		0.00	
2	☐ J12			
3	☐ J14			
4	☐ J16		0.00	
5	☐ J18			
6	☐ J20			
7	J22		0.00	
8	☐ J24			
9	☐ J26		0.00	
10	☐ J28			
11	☐ J30			
12	☐ J32			
13	☐ J36			
14	J38			
15	☐ J42			
16	☐ J44			
17	☐ J46			
18	☐ J48			
19	☐ J50			
20	☐ J52			
21	☐ J54			
22	☐ J56			
23	J58			
24	☐ J60			
25	☐ J62			
26	☐ J64			
27	☐ J66			
28	☐ J72			
29	☐ J74		0.00	
30	☐ J76			
31	J78		0.00	
32	J80			
33	☐ J82			
34	J84			
35	J86			
36	☐ J90			

Input Data - Junction Demands

* BASE *	ID	Pattern 9	Demand 10	Pattern 10
_	(Char)	(Char)	(gpm)	(Char)
37	J92		0.00	
38	J96			
39	J98			
40	J104			
41	J106			
42	J108		0.00	
43	J114			
44	J116			
45	J118		0.00	
46	J120			
47	J122			
48	J124			
49	J126			
50	J128		0.00	
51	J130			
52	J132			
53	J134			
54	J136		0.00	
55	J138			
56	J140			
57	J142			
58	J146		0.00	
59	J148			
60	J150			
61	J152			
62	J156			
63	J158			
64	J160			
65	J166			
66	J168			
67	J176			
68	J180			
69	J182			
70	J186			
71	J188			
72	J192			

Input Data - Junction Demands

* DACE *	100	ID	Pattern 9	Demand 10	Pattern 10
* BASE *		(Char)	(Char)	(gpm)	(Char)
73		J194			
74		J196			
75		J200			
76		J202		0.00	
77		J208			
78		J210		0.00	
79		J212			
80		J214			
81		J218			
82		J224			
83		J226			
84		J228			
85		J230			
86		J232			
87		J234		0.00	
88		J238			
89		J240			
90		J242			
91		J244			
92		J248			
93		J250			
94		J252		0.00	
95		J254			
96		J256		0.00	
97		J260			
98		J262			
99		J266			
100		J274			
101		J276			
102		J278			
103		J280		0.00	
104		J288		0.00	
105		J290		0.00	
106		J292			
107		J294		0.00	
108		J296			

Input Data - Junction Demands

* BASE *	ID	Pattern 9	Demand 10	Pattern 10
_	(Char)	(Char)	(gpm)	(Char)
109	J298			
110	J300			
111	J304			
112	J306			
113	J308			
114	J312		0.00	
115	J314			
116	J320		0.00	
117	J322			
118	J324			
119	J326			
120	J328			
121	J330			
122	J334		0.00	
123	J336			
124	J340			
125	J346			
126	J348			
127	J350			
128	J352			
129	J356			
130	J358			
131	J360			
132	J362			
133	J364			
134	J366		0.00	
135	J368			
136	J370			
137	J372			
138	J374			
139	J376			
140	J378			
141	J380			
142	J382			
143	J384			
144	J386		0.00	

Input Data - Junction Demands

* BAGE *	ID	Pattern 9	Demand 10	Pattern 10
* BASE *	(Char)	(Char)	(gpm)	(Char)
145	J388			
146	J390			
147	J392			
148	J396		0.00	
149	J398			
150	J400		0.00	
151	J402			
152	J406		0.00	
153	J408			
154	J410			
155	J414			
156	J416		0.00	
157	J420			
158	J422			
159	J428			
160	J430			
161	J432		0.00	
162	J434			
163	J436			
164	J440			
165	J442			
166	J444			
167	J446			
168	J448		0.00	
169	J450			
170	J452			
171	J454			
172	J456			
173	J458		0.00	
174	J460			
175	J462		0.00	
176	J464		0.00	
177	J466		0.00	
178	J468		0.00	
179	J470			
180	J472		0.00	

Input Data - Junction Demands

* DAGE *	ID	Pattern 9	Demand 10	Pattern 10
* BASE *	(Char)	(Char)	(gpm)	(Char)
181	J474			
182	J476			
183	J478			
184	J480			
185	J482			
186	J484			
187	J486			
188	J488			
189	J490		0.00	
190	J492			
191	J498			
192	J500			
193	J502			
194	J506		0.00	
195	J508			
196	J512			
197	J514			
198	J516			
199	J518			
200	J520			
201	J522			
202	J524		0.00	
203	J526			
204	J530			
205	J532		0.00	
206	J534			
207	J536		0.00	
208	J538			
209	J542			
210	J544			
211	J546			
212	J548			
213	J550			
214	J552			
215	J554		0.00	
216	J556			

Input Data - Junction Demands

* BASE *	ID	Pattern 9	Demand 10	Pattern 10
	(Char)	(Char)	(gpm)	(Char)
217	J558			
218	J560			
219	J562			
220	J564		0.00	
221	J566			
222	J570			
223	J572			
224	J578			
225	J580			
226	J582			
227	J584			
228	J586			
229	J588			
230	J590			
231	J592		0.00	
232	J594			
233	J600			
234	J602			
235	J604			
236	J606		0.00	
237	J608			
238	J610			
239	J612			
240	J614			
241	J616		0.00	
242	J618		0.00	
243	J620		0.00	
244	J622		0.00	
245	J628		0.00	
246	J630		0.00	
247	J632		0.00	
248	J634		0.00	
249	J636			
250	J638			
251	J640			
252	J642			

* BASE *	ID (Char)	Pattern 9 (Char)	Demand 10 (gpm)	Pattern 10 (Char)
253	☐ J644			
254	☐ J646			
255	☐ J648			
256	☐ J650			
257	☐ J652			
258	☐ J654			
259	☐ J656			
260	☐ J658			
261	☐ J660		0.00	
262	☐ J662		0.00	
263	☐ J664		0.00	
264	☐ J666		0.00	
265	☐ J668		0.00	
266	☐ J670			

Input Data - Junction Information

	ID	Description	Year of	Year of	Zone	Elevation	Phase
	(Char)	(Char)	Installation	Retirement	(Char)	(ft)	(Int)
1	J10		<i>.</i>		Low Pressure	1,524.00	
2	J12				Low Pressure	1,516.00	
3	J14				Low Pressure	1,493.00	
4	J16				Low Pressure	1,506.00	
5	J18				Low Pressure	1,515.00	
6	J20				Low Pressure	1,506.00	
7	J22				Low Pressure	1,507.00	
8	J24				Low Pressure	1,505.00	
9	J26				2-inch	1,502.00	
10	J28				Low Pressure	1,509.00	
11	J30				Low Pressure	1,513.00	
12	J32				Low Pressure	1,513.00	
13	J36				Low Pressure	1,512.00	
14	J38				Low Pressure	1,512.00	
15	J42				Low Pressure	1,513.00	
16	J44				Low Pressure	1,517.00	
17	J46				Low Pressure	1,511.00	
18	J48				Low Pressure	1,513.00	
19	J50	Main Street Commercia			Low Pressure	1,520.00	
20	J52				Low Pressure	1,516.00	
21	J54				Low Pressure	1,515.00	
22	J56				Low Pressure	1,514.00	
23	J58	Main Street Commercia			Low Pressure	1,518.00	
24	J60	Main Street Commercia			Low Pressure	1,521.00	
25	J62	Main Street Commercia			Low Pressure	1,517.00	
26	J64	Main Street Commercia			Low Pressure	1,523.00	
27	J66				Low Pressure	1,517.00	
28	J72	Main Street Commercia			Low Pressure	1,518.00	
29	J74	Main Street Commercia			Low Pressure	1,522.00	
30	J76				Low Pressure	1,516.00	
31	J78				Low Pressure	1,520.00	
32	J80				Low Pressure	1,520.00	
33	J82				Low Pressure	1,540.00	
34	J84				Low Pressure	1,568.00	
35	J86				Low Pressure	1,605.00	
36	J90				Low Pressure	1,657.00	

Input Data - Junction Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Elevation	Phase
		(Char)	(Char)	iiistallatiori	Kettlement	(Char)	(ft)	(Int)
37	<u> </u>	J92				Low Pressure	1,675.00	
38	<u> </u>	J96				Low Pressure	1,572.00	
39		J98				Low Pressure	1,527.00	
40		J104				Low Pressure	1,533.00	
41		J106				Low Pressure	1,517.00	
42		J108				Low Pressure	1,517.00	
43		J114				Low Pressure	1,575.00	
44		J116				Low Pressure	1,548.00	
45		J118				Low Pressure	1,560.00	
46		J120				Low Pressure	1,558.00	
47		J122				Low Pressure	1,596.00	
48		J124				Low Pressure	1,518.00	
49		J126				Low Pressure	1,514.00	
50		J128				Low Pressure	1,519.00	
51		J130				Low Pressure	1,523.00	
52		J132				Low Pressure	1,522.00	
53		J134				Low Pressure	1,530.00	
54		J136				Low Pressure	1,536.00	
55		J138				Low Pressure	1,536.00	
56		J140				Low Pressure	1,530.00	
57		J142				Low Pressure	1,540.00	
58		J146				Low Pressure	1,610.00	
59		J148				Low Pressure	1,585.00	
60		J150				Low Pressure	1,526.00	
61		J152				Low Pressure	1,564.00	
62		J156				2-inch	1,581.00	
63		J158				Low Pressure	1,559.00	
64		J160				Low Pressure	1,605.00	
65		J166				Low Pressure	1,610.00	
66		J168				2-inch	1,696.00	
67		J176				Low Pressure	1,530.00	
68		J180				Low Pressure	1,568.00	
69		J182				Low Pressure	1,538.00	
70		J186				Low Pressure	1,569.00	
71		J188				Low Pressure	1,605.00	
72		J192				Low Pressure	1,654.00	

Input Data - Junction Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Elevation (ft)	Phase (Int)
70			(Onal)	" "	" A	1 1		(IIII)
73		J194				2-inch	1,690.00	
74		J196				Low Pressure	1,609.00	
75	<u> </u>	J200				Low Pressure	1,625.00	
76	<u> </u>	J202				High Pressure	1,718.00	
77	<u> </u>	J208				Tank	1,723.00	
78	<u> </u>	J210				High Pressure	1,768.00	
79	<u> </u>	J212				Low Pressure	1,604.00	
80	<u> </u>	J214				Tank	1,722.00	
81		J218				Tank	1,725.00	
82		J224				Low Pressure	1,570.00	
83		J226				Low Pressure	1,534.00	
84		J228				Low Pressure	1,551.00	
85		J230				Low Pressure	1,537.00	
86		J232				Low Pressure	1,548.00	
87		J234				2-inch	1,554.00	
88		J238				Low Pressure	1,533.00	
89		J240				Low Pressure	1,540.00	
90		J242				Low Pressure	1,513.00	
91		J244				Low Pressure	1,513.00	
92		J248	Main Street Commercia			Low Pressure	1,504.00	
93		J250				Low Pressure	1,504.00	
94		J252				Low Pressure	1,499.00	
95		J254				Low Pressure	1,517.00	
96		J256				2-inch	1,524.00	
97		J260				Low Pressure	1,494.00	
98		J262				Low Pressure	1,497.00	
99		J266				Low Pressure	1,532.00	
100		J274				Low Pressure	1,500.00	
101		J276				Low Pressure	1,498.00	
102		J278				Low Pressure	1,498.00	
103		J280				Low Pressure	1,496.00	
104		J288				Low Pressure	1,497.00	
105		J290				Low Pressure	1,495.00	
106		J292				Low Pressure	1,496.00	
107		J294				Low Pressure	1,494.00	
108		J296				Low Pressure	1,543.00	

Input Data - Junction Information

	4	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Elevation	Phase
400			(Criar)	ilistaliation	Kethement		(ft)	(Int)
109		J298				Low Pressure	1,523.00	
110		J300				Low Pressure	1,506.00	
111		J304				Low Pressure	1,506.00	
112	<u> </u>	J306				Low Pressure	1,507.00	
113	<u> </u>	J308				Low Pressure	1,504.00	
114	<u> </u>	J312				Low Pressure	1,508.00	
115	<u> </u>	J314				Low Pressure	1,517.00	
116	<u> </u>	J320				Low Pressure	1,590.00	
117		J322				Low Pressure	1,518.00	
118		J324				Low Pressure	1,538.00	
119	<u> </u>	J326				Low Pressure	1,534.00	
120		J328				Low Pressure	1,512.00	
121		J330				Low Pressure	1,506.00	
122		J334				Low Pressure	1,503.00	
123		J336				Low Pressure	1,503.00	
124		J340				Low Pressure	1,506.00	
125		J346				Low Pressure	1,498.00	
126		J348				Low Pressure	1,499.00	
127		J350				Low Pressure	1,501.00	
128		J352				Low Pressure	1,499.00	
129		J356				Low Pressure	1,500.00	
130		J358				Low Pressure	1,505.00	
131		J360				Low Pressure	1,508.00	
132		J362				Low Pressure	1,509.00	
133		J364				Low Pressure	1,519.00	
134		J366				Low Pressure	1,521.00	
135		J368				Low Pressure	1,538.00	
136		J370				Low Pressure	1,535.00	
137		J372				2-inch	1,527.00	
138		J374				Low Pressure	1,507.00	
139		J376				Low Pressure	1,497.00	
140		J378				Low Pressure	1,548.00	
141		J380				Low Pressure	1,529.00	
142		J382				Low Pressure	1,535.00	
143		J384	Main Street Commercia	ı		Low Pressure	1,516.00	
144	Г	J386	Main Street Commercia	ı		Low Pressure	1,517.00	

Input Data - Junction Information

	ID	Description	Year of	Year of	Zone	Elevation	Phase
	(Char)	(Char)	Installation	Retirement	(Char)	(ft)	(Int)
145	J388	Main Street Commercia			2-inch	1,525.00	
146	J390	Main Street Commercia			Low Pressure	1,516.00	
147	J392	Main Street Commercia			Low Pressure	1,516.00	
148	J396	Main Street Commercia			Low Pressure	1,533.00	
149	J398				Low Pressure	1,521.00	
150	J400	Main Street Commercia			Low Pressure	1,521.00	
151	J402	Main Street Commercia			Low Pressure	1,506.00	
152	J406	Main Street Commercia			Low Pressure	1,512.00	
153	J408				Low Pressure	1,522.00	
154	J410				Low Pressure	1,504.00	
155	J414				Low Pressure	1,496.00	
156	J416				Low Pressure	1,498.00	
157	J420				Low Pressure	1,495.00	
158	J422				WTP	1,497.00	
159	J428	Main Street Commercia			Low Pressure	1,504.00	
160	J430				Low Pressure	1,502.00	
161	J432				Low Pressure	1,492.00	
162	J434				Low Pressure	1,505.00	
163	J436				Low Pressure	1,504.00	
164	J440	Main Street Commercia			Low Pressure	1,509.00	
165	J442	Main Street Commercia			Low Pressure	1,504.00	
166	J444				Low Pressure	1,505.00	
167	J446				Low Pressure	1,525.00	
168	J448				Low Pressure	1,524.00	
169	J450				Low Pressure	1,512.00	
170	J452				Low Pressure	1,530.00	
171	J454				Low Pressure	1,508.00	
172	J456				Low Pressure	1,528.00	
173	J458				Low Pressure	1,513.00	
174	J460				Low Pressure	1,525.00	
175	J462				Low Pressure	1,514.00	
176	J464				Low Pressure	1,510.00	
177	J466				Low Pressure	1,497.00	
178	J468				Low Pressure	1,492.00	
179	J470				Low Pressure	1,492.00	
180	J472				Low Pressure	1,486.00	

Input Data - Junction Information

		ID (Ob - v)	Description	Year of	Year of	Zone	Elevation	Phase
		(Char)	(Char)	Installation	Retirement	(Char)	(ft)	(Int)
181	<u> </u>	J474				Low Pressure	1,524.00	
182	<u> </u>	J476				Low Pressure	1,513.00	
183	<u> </u>	J478				Low Pressure	1,504.00	
184		J480				Low Pressure	1,522.00	
185		J482				Low Pressure	1,517.00	
186		J484				Low Pressure	1,524.00	
187		J486				Low Pressure	1,503.00	
188		J488				Low Pressure	1,499.00	
189		J490				Low Pressure	1,498.00	
190		J492				Low Pressure	1,493.00	
191		J498				Low Pressure	1,492.00	
192		J500				Low Pressure	1,527.00	
193		J502				Low Pressure	1,499.00	
194		J506				Low Pressure	1,491.00	
195		J508				Low Pressure	1,499.00	
196		J512				Low Pressure	1,496.00	
197		J514				2-inch	1,498.00	
198		J516				Low Pressure	1,496.00	
199		J518				Low Pressure	1,500.00	
200		J520				Low Pressure	1,569.00	
201		J522				Low Pressure	1,577.00	
202		J524				Low Pressure	1,563.00	
203		J526				Low Pressure	1,492.00	
204		J530				Low Pressure	1,593.00	
205		J532				2-inch	1,636.00	
206		J534				Low Pressure	1,503.00	
207		J536				2-inch	1,503.00	
208		J538				Low Pressure	1,510.00	
209		J542				Low Pressure	1,565.00	
210		J544				Low Pressure	1,533.00	
211		J546				Low Pressure	1,505.00	
212		J548				Low Pressure	1,552.00	
213		J550				Low Pressure	1,533.00	
214		J552				Low Pressure	1,505.00	
215		J554				Low Pressure	1,502.00	
216	Г	J556				Low Pressure	1,496.00	

Input Data - Junction Information

	ID (OL)	Description	Year of	Year of	Zone	Elevation	Phase
	(Char)	(Char)	Installation	Retirement	(Char)	(ft)	(Int)
217	J558				Low Pressure	1,590.00	
218	J560				Low Pressure	1,497.00	
219	J562				Low Pressure	1,497.00	
220	J564				Low Pressure	1,495.00	
221	J566				Low Pressure	1,572.00	
222	J570				Low Pressure	1,498.00	
223	J572				Low Pressure	1,498.00	
224	J578				Low Pressure	1,496.00	
225	J580				Low Pressure	1,493.00	
226	J582				Low Pressure	1,492.00	
227	J584				Low Pressure	1,504.00	
228	J586				Low Pressure	1,501.00	
229	J588				Low Pressure	1,504.00	
230	J590				Low Pressure	1,556.00	
231	J592				Low Pressure	1,595.00	
232	J594				Low Pressure	1,497.00	
233	J600				Low Pressure	1,534.00	
234	J602				Low Pressure	1,533.00	
235	J604				Low Pressure	1,532.00	
236	J606				Low Pressure	1,559.00	
237	J608				Low Pressure	1,648.00	
238	J610				Low Pressure	1,505.00	
239	J612				WTP	1,497.00	
240	J614				WTP	1,497.00	
241	J616				Low Pressure	1,497.00	
242	J618				Low Pressure	1,493.00	
243	J620				Low Pressure	1,498.00	
244	J622	Main Street Commercia			Low Pressure	1,513.00	
245	J628				Low Pressure	1,524.00	
246	J630				Low Pressure	1,565.00	
247	J632				Low Pressure	1,560.00	
248	J634				Low Pressure	1,562.00	
249	J636				Low Pressure	1,515.00	
250	J638				Low Pressure	1,498.00	
251	J640				Low Pressure	1,498.00	
252	J642				Low Pressure	1,498.00	

	1	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Elevation (ft)	Phase (Int)
253		J644				Low Pressure	1,500.00	
254		J646				Low Pressure	0.00	
255		J648				Low Pressure	1,499.00	
256		J650				Low Pressure	1,500.00	
257		J652				Low Pressure	1,499.00	
258		J654				Low Pressure	1,505.00	
259		J656				Low Pressure	1,506.00	
260		J658				Low Pressure	1,502.00	
261		J660				High Pressure	1,691.00	
262		J662				Low Pressure	1,665.00	
263		J664				High Pressure	1,630.00	
264		J666				High Pressure	0.00	
265		J668				High Pressure	1,710.00	
266		J670				Low Pressure	1,505.00	

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
1		0	(7. 0	71 . ^	()	CI	()	(/	()
2		1					CA			
3		2					CA			
4		3					GAL			
5		4					CA			
6		5					PL			
7		6					DI			
8		7					CI			
9		8					CA			
10		9					CI			
11		10					CI			
12		11					CI			
13		12					DI			
14		13					CI			
15		14					CI			
16		15					CI			
17		16					CI			
18		18					CI			
19		19					CI			
20		20					DI			
21		21					DI			
22	<u> </u>	22					DI			
23	<u> </u>	23					CI			
24	<u> </u>	24					CI			
25		25					DI			
26	느늗	26					DI			
27	늗	28					CI			
28		29					CI			
29		30								
30		31					CI			
31		32					CA CA			
32		33					DI			
33 34		34	<u> </u>				PL			
35	늗	35 36	-				CI			
35	늗	36 37	-				DI			

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
37		38	,		4 4	(/	DI			
38		40					CI			
39		41					CA			
40		42					CI			
41		43					GAL			
42		44					CI			
43		46					GAL			
44		47					CI			
45		48					DI			
46		49					DI			
47		50					CI			
48		52					CI			
49		53					CI			
50		54					PL			
51		55					DI			
52		57					DI			
53		58					DI			
54		59					PL			
55		60					CI			
56		61					DI			
57	ᅳ	64					CI			
58	ᆜ	65					CI			
59	ᆜ	66					CU			
60		68					CI			
61	<u> </u>	70					CI			
62		71					CI			
63	<u> </u>	72					GAL			
64	느늗	73					CI			
65	느늗	74					CI			
66		77					CI			
67		78					CI			
68		79					CI			
69		80					CI			
70		84					CI			
71	L	85					CI			
72		86					DI			

Input Data - Pipe Information

	nation	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
73		87		7 3	7. 3		DI			
74		88					DI			
75		89					CI			
76		90					PL			
77		91					CA			
78		93					DI			
79		94					DI			
80		95					CI			
81		97					CI			
82		98					GAL			
83		99					DI			
84		103					DI			
85		104					CI			
86		106					DI			
87		107					DI			
88		108					CI			
89		109					DI			
90		110					CI			
91		113					CI			
92		114					GAL			
93	무	117					CI CI			
94		118					CI			
95 96		119 120					CI			
96		120					GAL			
98	-	121					CI			+
99		124					DI			
100		125					CI			+
101		126					CI			+
102	<u> </u>	127					CI			
103	<u> </u>	128					CI			
104		129					CI			
105		130					CI			
106	一一	131					PL			
107		132					DI			
108		133					PL			

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
109		135		71 - 2	4 4		CI	, ,		, ,
110		136					CI			
111		137					DI			
112		138					DI			
113		139					CI			
114		141					DI			
115		142					DI			
116		143					DI			
117		144					DI			
118		145					DI			
119		146					DI			
120		147					DI			
121		148					DI			
122		149					DI			
123		150					DI			
124		151					CI			
125		152					DI			
126		153					DI			
127		154					PL			
128		155					CI			
129		156					CI			
130		157					CI			
131		158					PL			
132		159					CA			
133		161					DI			
134	<u> </u>	162					GAL			
135	<u> </u>	163					CI			
136	<u> </u>	164					DI			
137		165					CI			
138	므	166					CI			
139		167					CI			
140		168					CU			
141		169					GAL			
142		170					DI			
143		171					DI			
144		172					DI			

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
145		173		7	4 4	. ,	CA	, ,		, ,
146		174					CI			
147		175					CI			
148		176					CI			
149		177					DI			
150		178					DI			
151		179					CI			
152		180					DI			
153		182					CI			
154		183					DI			
155		184					CI			
156		185					DI			
157		186								
158		188					DI			
159		190					CI			
160		191								
161		192								
162		193								
163		P11					DI			
164		P13					CA			
165		P15					CA			
166	<u> </u>	P17					DI			
167	<u> </u>	P19					CA			
168		P21					CA			
169	<u> </u>	P23					CA			
170	<u> </u>	P25					CA			
171	<u> </u>	P29					CI			
172		P31					CI			
173		P33					CI			
174		P37					CI			
175	무	P39					CI			
176		P41					CI			
177		P43					DI			
178	느	P45					DI			
179	느	P47					DI			
180		P49					CI			

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
18	1 [_		, ,	7		CI			
18	2	P63					CA			
18	3	P65					DI			
18	4 Γ	P67					DI			
18	5 Γ	P69					DI			
18	6	P71					CI			
18	7	P77					DI			
18	8	P79					CI			
18	9	P81					CI			
19	0 [P83					CI			
19	1 [P85					CI			
19	2 Γ	P89					CI			
19	3	P93					DI			
19	4 Γ	P95					CI			
19	5 Γ	P97					CI			
19	6	P101					DI			
19	7	P103					DI			
19	8	P105					CI			
19	9	P109					CI			
20	0 [P111					CI			
20	1 [P113					CI			
20	2 Γ	P115					CI			
20	3	P119					CI			
20	4 Γ	P121					CI			
20	5 Γ	P123					DI			
20	6	P125					CI			
20	7	P129					DI			
20	8	P133					DI			
20	9	P135					CI			
21	0	P137					DI			
21	1 Γ	P141					CI			
21	2 Γ	P145					DI			
21	3	P147					DI			
21	4 Γ	P149					DI			
21	5 Γ	P151					DI			
21	6 Г	P153					DI			

Input Data - Pipe Information

·	Tipe information	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
	217	P155		4 4	71.5		DI			
	218	P159					DI			
	219	P161					CI			
	220	P163					CI			
	221	P165					DI			
	222	P167					DI			
	223	P169					CI			
	224	P171					CI			
	225	P173					DI			
	226	P175					CI			
	227	P177					CI			
	228	P181					CI			
	229	P183					CI			
	230	P185					CI			
	231	P187					CI			
	232	P189					CI			
	233	P191					CI			
	234	P193					PL			
	235	P195					CI			
	236	P197					DI			
	237	P203					CI			
	238	P205					DI			
	239	P207					DI			
	240	P209					DI			
	241	P211					DI			
	242	P213					DI			
	243	P215					DI			
	244	P217					CI			
	245	P221					CI			
	246	P227					CI			
	247	P229					CI			
	248	P231					CA			
	249	P233					CI			
	250	P237					CI			
	251	P239				Low Pressure				
	252	P243					DI			

Input Data - Pipe Information

		ID (Char)	Description (Char)	Year of	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
253		P245	(Criai)	Installation	Retirement	(Criai)	CI	(Criai)	(Criai)	(1111)
254	÷	P247					CI			
255	÷	P249					CI			
256	-	P251					CI			
257		P253					CI			
258		P255					CI			
259		P257					DI			
260	i i	P259					DI			
261	i i	P263					DI			
262	i i	P267					CI			
263		P269					CI			
264	i i	P271					CI			
265		P275					CI			
266		P277					DI			
267		P279					DI			
268		P281					DI			
269		P283					DI			
270		P285					DI			
271		P287					HDPE			
272		P289								
273		P291					CI			
274		P293					CI			
275		P295					CI			
276		P297					DI			
277		P299					CA			
278		P301					DI			
279		P303					CI			
280		P307					CA			
281		P309					CA			
282		P311					CA			
283		P313					DI			
284		P315					DI			
285		P317					DI			
286		P319								
287		P321								
288		P323								

Input Data - Pipe Information

	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
289	P325		7 3	71 .					
290	P327								
291	P329								
292	P333					DI			
293	P335					DI			
294	P337								
295	P339					CI			
296	P341					CI			
297	P347					CI			
298	P349				Low Pressure				
299	P351				Low Pressure				
300	P353				Low Pressure				
301	P355								
302	P357				Low Pressure				
303	P359				Low Pressure:	CI			
304	P361								
305	P363								
306	P365								
307	P367								
308	P369				Low Pressure	CI			
309	P371								
310	P373								
311	P375								
312	P377								
313	P379								
314	P381				Low Pressure				
315	P383					GAL			
316	P385					PL			
317	P387				Low Pressure				
318	P389					CI			
319	P391				Low Pressure				
320	P393					DI			
321	P397					HDPE			
322	P399					HDPE			
323	P401					HDPE			
324	P403					HDPE			

	ID (Char)	Description (Char)	Year of Installation	Year of Retirement	Zone (Char)	Material (Char)	Lining (Char)	Cost ID (Char)	Phase (Int)
325	P405					PL			
326	P407					DI			
327	P409								
328	P415					DI			
329	P417								

Input Data - Pipe Modeling

* BASE *		ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
1		0	1,669.27	6.00	58.00	,	No	No
2	□ 1		5,984.20	8.00	140.00		No	No
3		2	981.50	6.00	140.00		No	No
4		3	124.91	2.00	49.00		No	No
5		4	666.34	6.00	140.00		No	No
6		5	320.27	8.00	140.00		No	No
7		6	558.28	8.00	110.00		No	No
8		7	880.92	6.00	58.00		No	No
9		8	733.23	6.00	58.00	0.00	No	No
10		9	299.13	6.00	58.00	0.00	No	No
11		10	824.44	10.00	63.00	0.00	No	No
12		11	45.57	10.00	63.00		No	No
13		12	211.56	8.00	110.00		No	No
14		13	586.10	6.00	58.00		No	No
15		14	701.70	6.00	58.00	0.00	No	No
16		15	410.89	8.00	63.00		No	No
17		16	645.35	6.00	58.00	0.00	No	No
18		18	168.34	6.00	58.00	0.00	No	No
19		19	179.78	10.00	63.00		No	No
20		20	383.11	10.00	110.00		No	No
21		21	703.13	12.00	110.00		No	No
22		22	113.89	12.00	110.00	0.00	No	No
23		23	930.56	6.00	58.00	0.00	No	No
24		24	385.22	6.00	58.00	0.00	No	No
25		25	1,918.54	8.00	110.00		No	No
26		26	948.50	6.00	110.00		No	No
27		28	1,892.42	6.00	58.00		No	No
28		29	159.47	10.00	63.00		No	No
29		30	56.60	10.00	63.00		No	No
30		31	1,099.02	12.00	63.00		No	No
31		32	472.82	6.00	140.00		No	No
32		33	1,096.47	6.00	140.00	0.00	No	No
33		34	574.52	6.00	110.00		No	No
34		35	998.51	6.00	140.00		No	No
35		36	243.50	6.00	58.00		No	No
36		37	238.46	8.00	110.00		No	No

Input Data - Pipe Modeling

* BASE *	ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
37	38	247.38	8.00	110.00		No	No
38	40	380.61	6.00	58.00		No	No
39	41	1,056.11	6.00	140.00		No	No
40	42	602.00	6.00	58.00		No	No
41	43	403.95	2.00	49.00		No	No
42	44	853.16	6.00	58.00		No	No
43	46	1,066.52	2.00	49.00		No	No
44	47	926.42	10.00	63.00		No	No
45	48	477.70	8.00	110.00		No	No
46	49	429.24	12.00	110.00		No	No
47	50	936.01	6.00	58.00		No	No
48	52	51.96	14.00	63.00		No	No
49	53	135.40	6.00	58.00		No	No
50	54	764.09	2.00	140.00		No	No
51	55	275.02	8.00	110.00		No	No
52	57	530.56	6.00	110.00		No	No
53	58	449.48	14.00	110.00	0.00	No	No
54	59	13.41	2.00	140.00	0.00	No	No
55	60	313.41	14.00	63.00	0.00	No	No
56	61	264.46	14.00	110.00	0.00	No	No
57	64	2,259.23	6.00	58.00		No	No
58	65	604.77	6.00	58.00	0.00	No	No
59	66	310.22	1.00	49.00	0.00	No	No
60	68	622.29	12.00	63.00		No	No
61	70	369.84	6.00	58.00		No	No
62	71	1,082.02	6.00	58.00	0.00		No
63	72	222.77	2.00	49.00		No	No
64	73	964.17	6.00	58.00			No
65	74	513.79	6.00	58.00		No	No
66	77	595.51	8.00	63.00		No	No
67	78	610.71	6.00	58.00	0.00	No	No
68	79	266.03	6.00	58.00		No	No
69	80	1,406.39	6.00	58.00			No
70	84	469.21	14.00	63.00	0.00		No
71	85	457.07	4.00	49.00		No	No
72	86	369.69	10.00	110.00		No	No

Input Data - Pipe Modeling

* BASE *		ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
73		87	1,545.68	6.00	110.00	(2000.0)	` ,	No
74	Ë	88	3,264.51	8.00	110.00		_	No
75		89	403.87	6.00	58.00			No
76		90	560.33	8.00	140.00			No
77		91	1,046.87	6.00	140.00			No
78		93	1,557.57	8.00	110.00			No
79		94	1,534.09	8.00	110.00			No
80		95	29.95	6.00	58.00			No
81		97	633.63	6.00	58.00			No
82	Ė	98	1,809.04	2.00	49.00			No
83	Ė	99	103.56	8.00	110.00	0.00	-	No
84	Ë	103	1,137.84	8.00	110.00	3.00		No
85	Ė	104	526.30	6.00	58.00	0.00		No
86	$\overline{}$	106	550.10	8.00	110.00	0.00		No
87	$\overline{}$	107	435.22	6.00	110.00	0.00	-	No
88	$\overline{}$	108	2,376.64	6.00	58.00	0.00		No
89	$\overline{}$	109	691.09	8.00	110.00			No
90	$\overline{}$	110	530.01	6.00	58.00	0.00		No
91	$\overline{}$	113	2.61	6.00	58.00	0.00		No
92		114	446.52	2.00	49.00			No
93		117	1,145.70	8.00	6.30	0.00		No
94		118	517.88	6.00	58.00			No
95		119	361.84	10.00	63.00			No
96		120	450.79	8.00	63.00			No
97		121	329.84	2.00	49.00			No
98		122	47.99	6.00	58.00			No
99	Ē	124	591.91	6.00	110.00			No
100		125	766.66	6.00	58.00	0.00	No	No
101		126	391.72	4.00	49.00			No
102		127	436.80	6.00	58.00		No	No
103		128	219.89	10.00	63.00		No	No
104		129	97.42	6.00	58.00		No	No
105		130	423.47	6.00	58.00			No
106		131	2,178.31	12.00	140.00			No
107		132	535.99	12.00	110.00	0.00	No	No
108		133	410.24	12.00	140.00			No

Input Data - Pipe Modeling

* BASE *	4	ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
109		135	533.67	8.00	63.00	(,	, ,	No
110	Ë	136	1,418.14	6.00	58.00		-	No
111		137	966.79	8.00	110.00			No
112		138	856.97	8.00	110.00	0.00		No
113		139	210.69	4.00	49.00	0.00		No
114		141	410.67	8.00	110.00			No
115		142	1,085.83	8.00	110.00	0.00		No
116	Ė	143	1,100.96	8.00	110.00			No
117	Ē	144	971.82	8.00	110.00		No	No
118	Ē	145	733.66	8.00	110.00		No	No
119		146	176.74	8.00	110.00		No	No
120		147	1,732.99	8.00	110.00		No	No
121		148	1,229.63	8.00	110.00		No	No
122		149	449.31	6.00	110.00		No	No
123		150	1,388.80	8.00	110.00		No	No
124		151	819.37	6.00	58.00	0.00	No	No
125		152	936.54	8.00	110.00		No	No
126		153	610.40	8.00	110.00	0.00	No	No
127		154	1,825.32	6.00	140.00		No	No
128		155	2,982.74	8.00	63.00		No	No
129		156	336.72	8.00	63.00		No	No
130		157	1,128.69	6.00	58.00		No	No
131		158	795.33	6.00	140.00		No	No
132		159	356.36	6.00	140.00		No	No
133		161	421.73	8.00	110.00		No	No
134		162	457.73	2.00	49.00		No	No
135		163	697.15	6.00	58.00		No	No
136		164	485.07	8.00	110.00		No	No
137		165	1,569.70	6.00	58.00		No	No
138		166	1,369.69	6.00	58.00		No	No
139		167	743.61	6.00	58.00			No
140		168	550.77	2.00	49.00	0.00	No	No
141		169	250.72	2.00	49.00		No	No
142		170	1,743.93	10.00	110.00			No
143		171	579.67	8.00	110.00		No	No
144		172	575.29	6.00	110.00		No	No

Input Data - Pipe Modeling

* BASE *		ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
145		173	573.32	6.00	140.00	(2000.0)	, ,	No
146	Ħ	174	262.25	6.00	58.00	0.00	_	No
147	Ë	175	1,144.86	6.00	58.00	0.00		No
148	Ë	176	1,240.44	6.00	58.00			No
149	Ë	177	533.05	10.00	110.00	0.00		No
150	Ħ	178	323.48	8.00	110.00	0.00		No
151	Ħ	179	584.26	6.00	58.00			No
152		180	440.78	8.00	110.00			No
153	Ħ	182	375.89	8.00	63.00			No
154		183	553.00	8.00	110.00			No
155		184	1,168.55	6.00	58.00			No
156		185	758.23	6.00	110.00			No
157	Ė	186	962.21	6.00	110.00	0.00		No
158	Ė	188	49.07	10.00	110.00	0.00		No
159	Ė	190	33.85	6.00	58.00	0.00		No
160		191	286.11	6.00	58.00	0.00		No
161		192	1,981.98	6.00	58.00	0.00		No
162		193	1,113.31	4.00	49.00	0.00	No	No
163		P11	338.43	8.00	110.00			No
164		P13	24.16	8.00	140.00		No	No
165		P15	1,483.45	6.00	140.00		No	No
166		P17	404.12	8.00	110.00		No	No
167		P19	194.72	8.00	140.00		No	No
168		P21	336.12	8.00	140.00		No	No
169		P23	400.71	8.00	140.00		No	No
170		P25	402.36	8.00	140.00		No	No
171		P29	334.57	10.00	63.00		No	No
172		P31	413.02	10.00	63.00		No	No
173		P33	553.71	10.00	63.00		No	No
174		P37	16.86	8.00	63.00		No	No
175		P39	453.06	8.00	63.00		No	No
176		P41	702.70	6.00	58.00	0.00	No	No
177		P43	77.78	10.00	110.00		No	No
178		P45	547.75	10.00	110.00		No	No
179		P47	775.87	10.00	110.00		No	No
180		P49	259.41	8.00	63.00		No	No

Input Data - Pipe Modeling

* BASE *		ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
181		P51	459.11	8.00	63.00	(Double)	` ,	No
182	-	P63	257.68	6.00	140.00		_	No
183	-	P65	379.92	8.00	110.00			No
	-	P67	362.84	8.00	110.00			No
184	H							No
185	H	P69	168.25	8.00	110.00	0.00		No
186	-	P71	492.39	6.00	58.00	0.00		No
187	H	P77	604.86	12.00	110.00			
188	-	P79	394.65	12.00	63.00			No
189	늗	P81	313.49	12.00	63.00			No
190		P83	24.48	14.00	63.00		_	No
191		P85	751.89	14.00	63.00			No
192		P89	1,435.82	6.00	58.00			No
193		P93	294.95	8.00	110.00			No
194		P95	434.09	6.00	58.00			No
195		P97	448.91	6.00	58.00			No
196		P101	33.84	14.00	110.00	0.00		No
197		P103	103.52	14.00	110.00	0.00		No
198		P105	1,143.48	14.00	63.00	0.00		No
199		P109	192.05	6.00	58.00	0.00		No
200		P111	751.63	6.00	58.00			No
201		P113	43.54	12.00	63.00			No
202		P115	1,325.28	12.00	63.00	0.00		No
203		P119	351.17	6.00	58.00		No	No
204		P121	257.03	6.00	58.00			No
205		P123	1,421.54	10.00	110.00	0.00	No	No
206		P125	503.59	6.00	58.00			No
207		P129	1,266.15	8.00	110.00	0.00		No
208		P133	587.53	8.00	110.00	0.00	No	No
209		P135	451.88	6.00	58.00		No	No
210		P137	1,974.65	8.00	110.00		No	No
211		P141	80.45	8.00	63.00		No	No
212		P145	914.77	10.00	110.00		No	No
213		P147	798.03	10.00	110.00		No	No
214		P149	490.78	10.00	110.00		No	No
215		P151	1,112.05	10.00	110.00		No	No
216		P153	411.27	10.00	110.00		No	No

Input Data - Pipe Modeling

* BASE *	ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
217	P155	69.73	8.00	110.00	, ,	No	No
218	P159	72.56	8.00	110.00		No	No
219	P161	533.22	6.00	58.00		No	No
220	P163	4,362.63	6.00	58.00		No	No
221	P165	918.32	8.00	110.00		No	No
222	P167	618.33	8.00	110.00		No	No
223	P169	103.25	6.00	58.00		No	No
224	P171	211.99	6.00	58.00		No	No
225	P173	64.60	8.00	110.00		No	No
226	P175	1,384.45	8.00	63.00		No	No
227	P177	812.99	10.00	63.00		No	No
228	P181	406.72	10.00	63.00		No	No
229	P183	124.46	10.00	63.00		No	No
230	P185	391.09	6.00	58.00	0.00	No	No
231	P187	893.11	6.00	58.00		No	No
232	P189	555.38	10.00	63.00		No	No
233	P191	97.99	10.00	63.00		No	No
234	P193	766.61	12.00	140.00		No	No
235	P195	43.65	6.00	58.00		No	No
236	P197	667.89	8.00	110.00	0.00	No	No
237	P203	358.92	8.00	63.00		No	No
238	P205	872.28	8.00	110.00		No	No
239	P207	371.47	8.00	110.00	0.00	No	No
240	P209	397.32	8.00	110.00	0.00	No	No
241	P211	389.56	8.00	110.00	0.00	No	No
242	P213	1,247.91	8.00	110.00	0.00	No	No
243	P215	396.80	8.00	110.00		No	No
244	P217	1,560.77	6.00	58.00		No	No
245	P221	563.76	6.00	58.00		No	No
246	P227	417.77	6.00	58.00		No	No
247	P229	663.68	6.00	58.00		No	No
248	P231	326.91	6.00	140.00		No	No
249	P233	381.89	6.00	58.00	0.00		No
250	P237	65.34	6.00	58.00		No	No
251	P239	726.64	6.00	110.00	0.00	No	No
252	P243	3.15	8.00	110.00		No	No

Input Data - Pipe Modeling

* BASE *		ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
253		P245	340.33	6.00	58.00	0.00	No	No
254	Ē	P247	1,173.11	6.00	58.00			No
255		P249	346.50	6.00	58.00		No	No
256		P251	318.91	6.00	58.00		No	No
257		P253	792.56	6.00	58.00		No	No
258		P255	205.70	6.00	58.00		No	No
259		P257	1,010.81	8.00	110.00		No	No
260		P259	1,251.05	8.00	110.00		No	No
261		P263	30.87	8.00	110.00		No	No
262		P267	191.73	6.00	58.00		No	No
263		P269	506.07	8.00	63.00		No	No
264		P271	164.09	8.00	63.00		No	No
265		P275	190.37	6.00	58.00	0.00	No	No
266		P277	1,055.29	8.00	110.00		No	No
267		P279	283.83	8.00	110.00		No	No
268		P281	531.72	8.00	110.00		No	No
269		P283	433.59	8.00	110.00		No	No
270		P285	454.56	8.00	110.00		No	No
271		P287	42.39	2.00	140.00	0.00	No	No
272		P289	1,288.79	6.00	58.00	0.00	No	No
273		P291	626.35	6.00	58.00	0.00		No
274		P293	567.27	6.00	58.00	0.00	No	No
275		P295	592.77	6.00	58.00	0.00	No	No
276		P297	457.04	8.00	110.00	0.00	No	No
277		P299	652.64	8.00	140.00	0.00		No
278		P301	254.95	10.00	110.00	0.00	No	No
279		P303	466.76	6.00	58.00	0.00	No	No
280		P307	356.07	8.00	140.00	0.00	No	No
281		P309	549.88	8.00	140.00	0.00	No	No
282		P311	488.21	10.00	63.00	0.00		No
283		P313	44.27	10.00	110.00	0.00		No
284		P315	69.04	12.00	110.00	0.00		No
285		P317	54.43	12.00	110.00	0.00		No
286		P319	104.47	12.00	110.00	0.00		No
287		P321	68.83	12.00	110.00	0.00		No
288		P323	91.48	12.00	110.00	0.00	No	No

Input Data - Pipe Modeling

* BASE *	ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
289	P325	67.48	12.00	110.00	0.00	No	No
290	P327	113.46	12.00	110.00	0.00	No	No
291	P329	61.07	12.00	110.00	0.00	No	No
292	P333	819.06	8.00	110.00		No	No
293	P335	861.93	8.00	110.00		No	No
294	P337	377.05	6.00	58.00	0.00	No	No
295	P339	334.64	10.00	63.00		No	No
296	P341	808.68	10.00	63.00	0.00	No	No
297	P347	562.20	8.00	63.00	0.00	No	No
298	P349	598.24	8.00	61.00	0.00	No	No
299	P351	499.39	8.00	61.00	0.00	No	No
300	P353	138.83	8.00	61.00	0.00	No	No
301	P355	174.18	4.00	49.00	0.00	No	No
302	P357	437.43	6.00	58.00	0.00	No	No
303	P359	162.42	8.00	61.00	0.00	No	No
304	P361	758.57	6.00	58.00	0.00	No	No
305	P363	675.41	10.00	63.00	0.00	No	No
306	P365	191.65	6.00	58.00	0.00	No	No
307	P367	679.01	6.00	58.00	0.00	No	No
308	P369	480.90	6.00	58.00	0.00	No	No
309	P371	792.57	6.00	58.00	0.00	No	No
310	P373	285.90	8.00	61.00	0.00	No	No
311	P375	291.53	8.00	61.00	0.00	No	No
312	P377	305.88	6.00	58.00	0.00	No	No
313	P379	749.94	6.00	58.00	0.00	No	No
314	P381	438.87	6.00	58.00	0.00	No	No
315	P383	338.02	2.00	49.00		No	No
316	P385	887.95	8.00	140.00		No	No
317	P387	496.06	8.00	110.00	0.00	No	No
318	P389	525.00	6.00	58.00		No	No
319	P391	2,031.11	8.00	58.00	0.00	No	No
320	P393	563.18	6.00	110.00		No	No
321	P397	675.29	2.00	140.00	0.00		No
322	P399	225.91	2.00	140.00	0.00		No
323	P401	913.11	2.00	140.00	0.00	No	No
324	P403	275.61	2.00	140.00	0.00	No	No

* BASE *	ID (Char)	Length (ft)	Diameter (in)	Roughness (Double)	Minor Loss (Double)	Totalizer (Boolean)	Check Valve (Boolean)
325	P405	85.74	2.00	140.00	0.00	No	No
326	P407	268.27	8.00	110.00	0.00	No	No
327	P409	3,609.13	8.00	63.00	0.00	No	No
328	P415	995.90	10.00	110.00	0.00	No	No
329	P417	403.61	6.00	58.00	0.00	No	No

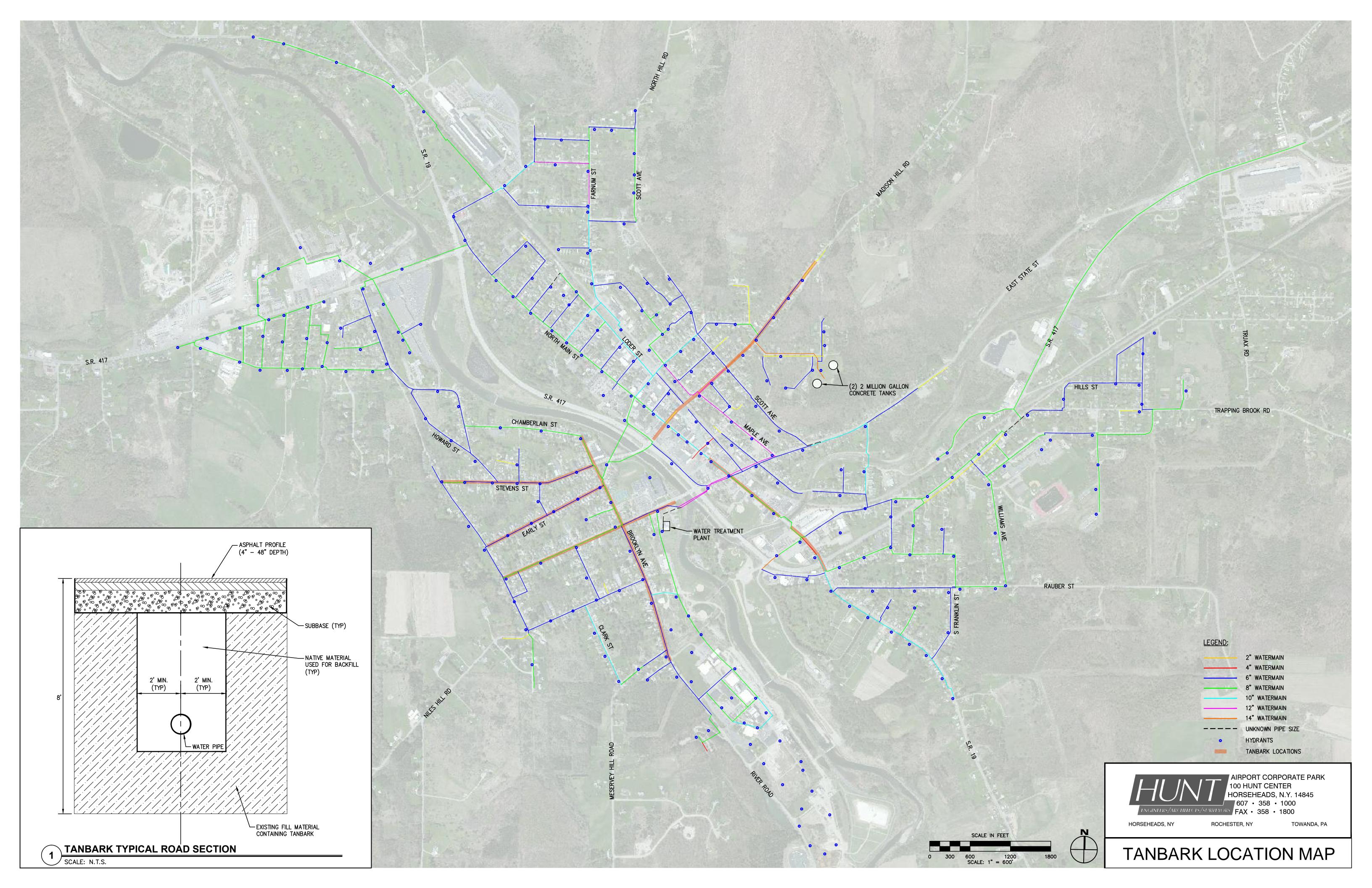
* BASE *	ID (Char)	Type (Int)	Elevation (ft)	Diameter (in)	Constant Power	Shutoff Head (ft)	Design Head (ft)	Design Flow (gpm)	High Head (ft)	High Flow (gpm)	Curve (Char)	NPSH Curve (Char)
1	SUNSET_PUMPSTATION	3: Multiple Point Curve	1,723.00	1.50		195.00	164.00	40.00	106.00	70.00	SUNSET_BPS	
2	PUMP1	3: Multiple Point Curve	1,497.00	12.00	125.00	0.00	0.00	0.00	0.00	0.00	WTP1	
3	PUMP2	3: Multiple Point Curve	1,497.00	12.00	125.00	0.00	0.00	0.00	0.00	0.00	WTP2	
4	PUMP3	3: Multiple Point Curve	1,497.00	12.00	60.00	0.00	0.00	0.00	0.00	0.00	WTP3	

Input Data - Reservoir Modeling

* BASE *	ID (Char)	Type (Int)	Head (ft)	Pattern (Char)
1	CLEARWELL	0: Fixed Head	1,487.00	

* BASE *	ID (Char)	Type (Int)	Elevation (ft)	Minimum Level (ft)	Maximum Level (ft)	Initial Level (ft)	Diameter (ft)
1	TANK_1	0: Cylindrical	1,720.00	0.00	20.00	18.00	131.00
2	TANK_2	0: Cylindrical	1,720.00	0.00	20.00	18.00	131.00
3	SUNSET_HYDRO	0: Cylindrical	1,723.00	69.30	115.00	115.00	0.26

APPENDIX D
TANBARK LOCATION MAP



APPENDIX E HYDRANT FIRE FLOWS



INSURANCE SERVICES OFFICE, INC.

4 B EVES DRIVE SUITE 200 MARLTON , NJ 08053 (609) 985-5600 FAX (609) 985-6464

October 24, 1997

Honorable Susan C. Goetschius, Mayor Village of Wellsville 156 N. Main St. Wellsville, NY. 14895

Dear Mayor Goetschius:

We wish to thank, Director of Public Works Robert Chaffee, Fire Chief Stanley Ingraham and others for the cooperation given to our representative during our recent survey. We have completed our evaluation of the fire insurance classification for your Village of Wellsville and confirm that Class 4 continues to apply.

The class 4 classification applies to properties with a needed fire flow of 3,500 gpm or less. The private and public protection at properties with larger needed fire flows are individually evaluated, and may vary from the class 4 classification

The purpose of our visit was to gather information needed to determine a fire insurance classification which may be used to develop advisory property insurance premium calculations. This survey was not conducted for property loss prevention or life safety purposes and no life safety or property loss prevention recommendations will be made.

We are attaching copies of our Grading Sheet and the results of the hydrant flow tests witnessed during our survey. Extra copies of this letter and attachments are also enclosed so that you may distribute them to other interested parties if you so desire.

If you have any questions concerning the new classification, please let us know.

Very truly yours.

Carl F. Shaner

Carl F. Shaner Technical Consultant

CSF/caf

Enclosure

CC:

Grading Sheet for Wellsville TFPD, Allegany Co., NY.

Public Protection Class: 4

Surveyed: July/97

<u>Feature</u>		Credit <u>Assigned</u>	Maximum <u>Credit</u>
Receiving and Handling Fire Alarms Fire Department Water Supply *Divergence	Total Credit	06.50 % 32.72 32.66 -03.72 68.64 %	10.00 % 50.00 % 40.00 %

The Public Protection Class is based on the total percentage credit as follows:

Class	%
1	90.00 or more
2	80.00 to 89.99
3	70.00 to 79.99
4	60.00 to 69.99
5	50.00 to 59.99
6	40.00 to 49.99
7	30.00 to 39.99
8	20.00 to 29.99
9	10.00 to 19.99
10	0 to 9.99

^{**}Divergence is a reduction in credit to reflect a difference in the relative credits for Fire Department and Water Supply.

The above classification has been develoed for use in property insurance premium calculations fire insurance rating purposes only.

HYDRANT FLOW DATA SUMMARY

CITY: Wellsville

State NY

ZIP: 14895 WITNESSED BY R. A. Herman

	REMARKS	gauge hyd. Bonnet Leaks		Flow hyd Bonnet Leaks	three hydrants on property	alinsanie alinsanie				Flow hyd. hard to operate			
FLOW AT 20 PSI	AVAIL.	+5800	3700	1500	*1000	1100	1300	1100	006	3900			
FL AT 2	NEEDED	3500	3000	2250	1500	750	2250	1500	200	3500			
PRESSURE PSI	RESID.	80	65	40	55	55	20	40	20	80			
PRES	STATIC	110	110	110	110	95	108	110	100	100			
	TOTAL	3203	2556	1320	790	750	1300	950	880	1840			
FLOW - GPM													
FLOW	INDIVIDUAL HYDRANTS			*	354								
		3203	2556	1320	790	750	1300	950	880	1840			
	SERVICE	Main	Main	Main	Main	Main	Main	Main	Main	Main			
	TEST LOCATION	W. State St. 1st.hyd e/o S. Brooklyn St.	S. Brooklyn St. opp. Lunn Court	Chenault Ave @ Water Treatment Plant	S. Brooklyn St. @ Otis Eastern Pipe Yard	Stevens St. 2nd hyd e/o Highland Ave	Riverwalk Plaza 2nd hyd s/o Bolivar Rd.	Route 417 @ Nursing Home	Route 19 2nd hyd n/o Vossler Rd.	N. Main St. E. Pearl St.			
, 2	DIST.	Сошш	Сошш	Сошш	Сотт	Res	Comm	Сотт	Res	Сотт			
Foat			2	က *	4 *	ر. د	ω	۲*	εο *	6			

THE ABOVE LISTED NEEDED FIRE FLOWS ARE FOR PROPERTY INSURANCE PREMIUM CALCULATIONS ONLY AND ARE NOT INTENDED TO PREDICT THE MAXIMUM AMOUNT OF WATER REQUIRED FOR A LARGE SCALE FIRE CONDITION. THE AVAILABLE FLOWS ONLY INDICATE THE CONDITIONS THAT EXISTED AT THE TIME AND AT THE LOCATION WHERE TESTS WERE WITNESSED.

HYDRANT FLOW DATA SUMMARY

CITY: Wellsville

ZIP: 14895 WITNESSED BY R.A. Herman

DATE: July 30, 1997

6			$\overline{}$	$\overline{}$	T			T		e.	 $\overline{}$		
	REMARKS												
FLOW	AVAIL.	4300	006	3500	2500	2200	450						
FLOW	NEEDED	3000	2250	3000	2500	4500	500						
PRESSURE PSI	RESID.	62	45	74	85	65	30						+
PRES	STATIC	95	85	100	105	100	95						
	TOTAL	2743	670	1917	1160	1436	430						
FLOW - GPM													
FLOV	INDIVIDUAL												
		2743	670	1917	1160	1436	430						
	SERVICE	Main	Main	Main	Main	Main	Main						
	TEST LOCATION	Central Ave w/o Loder St.	O'Conner St. w/o Scott St.	N. Main St. 1st hyd n/o E. State St.	S. Main St. n/o Dyke Creek	School St. opp Fair St.	S. Main St. opp Orchard Place		*Note: Water Flows Limited By Hydrant Distribution	* Flow Tests Located in Wellsville TFPD			
14	DIST.	Comm	Сотт	Сотт	Сошш	Сотт	Res		3				
TEST		2	, ,	12	13	14	15	82					

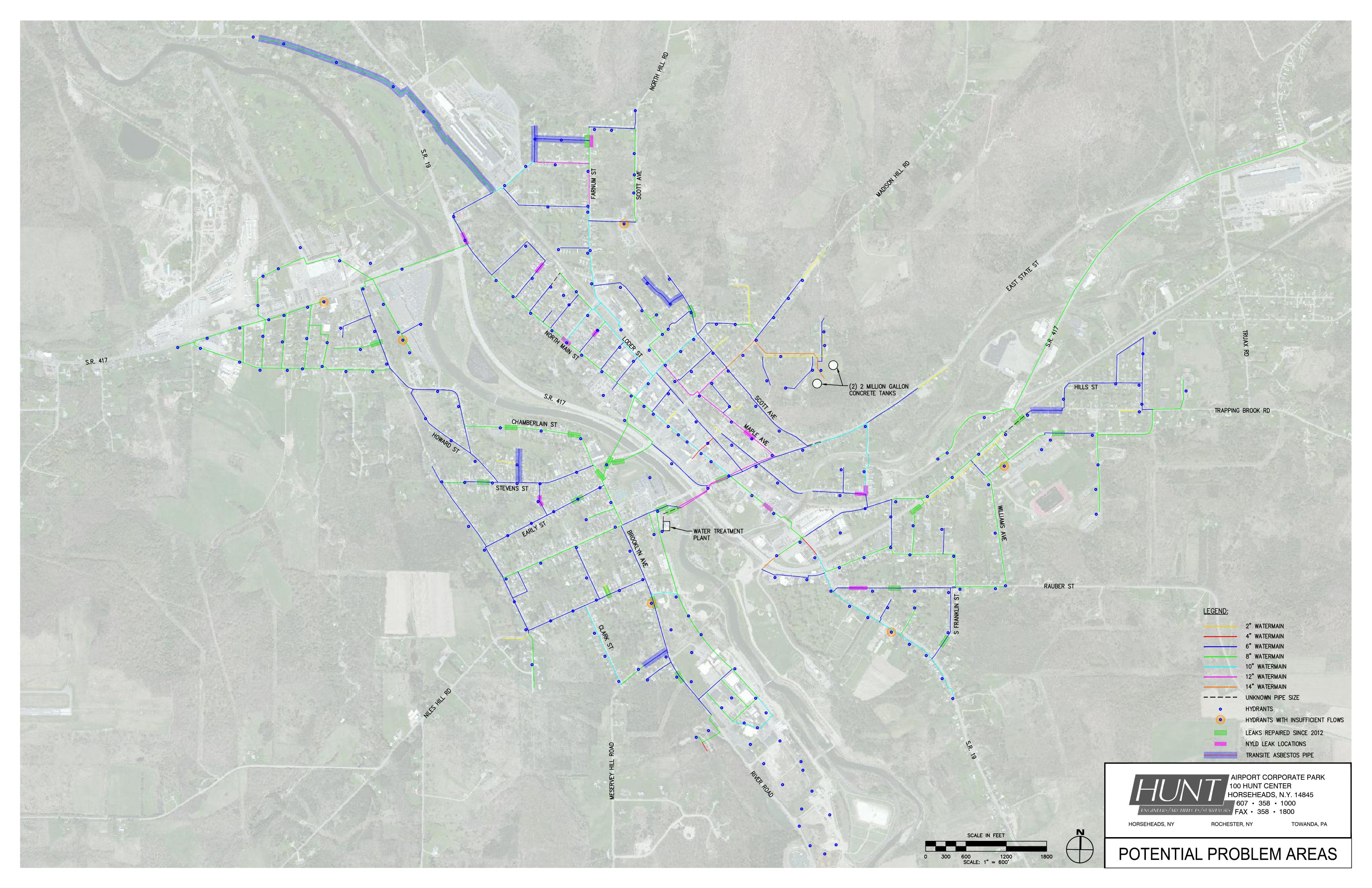
THE ABOVE LISTED NEEDED FIRE FLOWS ARE FOR PROPERTY INSURANCE PREMIUM CALCULATIONS ONLY AND ARE NOT INTENDED TO PREDICT THE MAXIMUM AMOUNT OF WATER REQUIRED FOR A LARGE SCALE FIRE CONDITION. THE AVAILABLE FLOWS ONLY INDICATE THE CONDITIONS THAT EXISTED AT THE TIME AND AT THE LOCATION WHERE TESTS WERE WITNESSED.

^{*}Comm = Commercial; Res = Residential
**Needed is the rate of flow for a specific duration for a full credit condition. Needed Fire Flows greater than 3,500 gpm are not considered in determining the classification of the city when using the Fire Suppression Raling "bedule...

LOCATION/ ADDRESS	FLOW G/MIN
EMPERS DARVING LOT	4556
EMBERS PARKING LOT	1550
LOADER NEAR LOMBARD	1455
ICE HOUSE E. DYKE	1455
FRONT OF HIGH SCHOOL	1455
E. STATE / MAPLE	1455
CENTRAL / LOADER	1455
S MAIN /RAUBER	1405
119 WILLIAMS	1400
MAIN / ELM	1375
PRESTONS CHEV	1350
HARTS JEWERERY	1300
FARNUM / OCONNER K-81	1300
159 CAMERON	1300
FARNUM / OCONNER K-11	1250
END OF S. MAIN	1250
END OF PUTZMAN	1250
375 S MAIN	1250
253 E. DYKE	1250
COATS AT DRESSER	1200
PIZZA KING	1190
MUSIC ALLEY	1190
41 HOWARD	1190
135 HOWARD	1190
130 CHAMBERLIN	1190
12 MEADOW BROOK	1190
#1 BEHIND TOPS	1190
W.STATE BROOKLYN	1150
SHERIDAN / BOLIVAR RD	. 1130
RIVERWALK PARKING LOT	1130
PUTZMAN / FARNUM	1130
POST OFFICE	1130
NATIONAL GRID	1130
N. MAIN EXT	1130
MIDDLE UPPER JEFFERSON	1130
MAIN / CHESTNUT	1130
LAZER WASH	1130
HILL CREST / FASSET LANE	1130
FLORDIA AVE / FASSETT LANE	1130
BEHIND HIGH SCHOOL	1130
91 HERMAN	1130
2895 ROBBINS COURT	1130
207 FARNUM	1130
#2_BEHIND TOPS	1130

LOCATION/ ADDRESS	FLOW G/MIN
W. DYKE W&L GARAGE	1060
HILL / CREASENT	1060
END OF LINE ACROSS FROM LC WHITFORDS	1060
CAMERON / HANOVER	1060
114 CLARK	1060
LATTER DAY SAINTS	1050
199 RAUBER	1050
TRAPPINGBROOK APTS	1000
TOP OF W. STATE	1000
STEVENS / HOWARD	1000
PINE / BROOKLYN	1000
PEARL / MADISON	1000
NELSON / S. HIGHLAND	1000
65 SCOTT AVE	1000
46 CLARK	1000
119 SENACA	1000
WHEEELER / PEARL	920
RIVERSIDE ARSNICK AND LACE	920
RIVERSIDE AUBER	900
FASSET LANE / HIGHLAND	900
BROOKLYN HTS	900
79 OCONNER	900
228 BROOKLYN	875
PEARL / SCOTT	850
PINE T. MGWS	840
16 SUNNYDALE	840
100 E. PEARL	800
STEVENS / WITTER	750
SCOTT / JEFFERSON	750
TOP OF STEVENS	650
RADIO STATIO RAILROAD AVE	650
HYLAND DALE ELDRIDGE	600
2ND HYDRANT UP ON EARLY	600
268 SCOTT	600
17 HANOVER	600
290 FARNUM	500
25 HAMILTON	500
PLEASANT	250
45 N. BROAD	250
329 SCOTT	250
54 STEVENS	100

APPENDIX F
POTENTIAL PROBLEM AREAS MAP



APPENDIX G WATER STORAGE TANK INSPECTION REPORT

ROBOTIC OBSERVATION VENTURES (R.O.V.)

POTABLE WATER STORAGE STRUCTURE INSPECTION FOR WELLSVILLE WEST WELLSVILLE, NEW YORK

INSPECTED ON AUGUST 27, 2014

GENERAL

The inspection was performed by a robotically operated vehicle (R.O.V.) with topside real time monitoring and recording.

The make and model of the remotely operated vehicle is Seabotix Gen2 150.

The inspection was performed to determine the structure's condition including all ceiling and wall surfaces, floor condition and structure penetrators.

Client's representative was present during the inspection.

The R.O.V. pilot was David Kazmierczak.

The R.O.V. handler was Brian Abrams.

Prior to insertion into the storage structure the R.O.V. and umbilical were disinfected with a solution of approximately 4% sodium hypochlorite applied by pressurized sprayer.

TANK SPECIFICATIONS

The tank is a Natgun pre-stressed, wire reinforced, concrete tank 130 feet in diameter, 20 feet deep, with a normal storage capacity of two million gallons.

No cathodic protection was present in the tank at the time of inspection.

INSPECTION

On the day of the inspection the water level in the tank was approximately 8 Ft. to 10 Ft. below overflow. In addition there was a small amount of floating particulate which reflected the R.O.V.'s. lighting. This did not, however, reduce the visibility of the top or roof of the tank.

The roof appears to be in good condition.

There is some biofilm cover on all below water surfaces.

The wall surfaces appeared to be in good condition.

The floor has 90% heavy sediment on bottom of tank.

The inflow & outflow floor penetrator pipes are located within catch basins set into the floor. The inside of the pipes appeared in relatively good condition.

OPINION

No immediate attention is required at this time.

NOTE – THE LAST PAGE OF THIS REPORT EXPLAINS THE ON SCREEN INFORMATION DISPLAY.

ALL WATER DEPTH READINGS ARE FROM THE WATER SURFACE ON THE DAY, AND AT THE TIME OF THE INSPECTION.

THE ACCURACY OF THIS DEPTH IS BASED ON THE WATER LEVEL REMAINING STATIC THROUGHOUT THE PROCEDURE.



View of overflow near water level.



View of coating patch on wall in upper third of tank. Wall appears to be in good condition.



Isolated rusting through wall coating in upper third of tank.

View of wall hatch. Hatch appears to be in good condition. Note the bio-film on wall.







Detail view of wall coating in the middle third of the tank. Note the exposed aggregate.







Isolated rusting in the middle third of the tank.

View of the tank wall intersecting with the tank floor. Note the

accumulation of flaked wall coating

on the floor.

Page 6 of 12







View of the inflow floor penetrator pipe. The pipe appears to be good condition.

Remotely Operated Vehicle Underwater Inspection





View of the outflow floor penetrator pipe. The pipe appears to be in good condition.



View of the second inflow floor penetrator pipe. The pipe appears to be in good condition.



View of the tank wall intersection with the tank roof. The roof appears to be in good condition.



View of the top of the overflow. The overflow appears to be in good condition.



View of the center of the tank roof. The roof appears to be in good condition.

View of the tank floor. Note the complete sediment coverage.

Detail view of the accumulated sediment at the tank wall intersection with the tank floor.



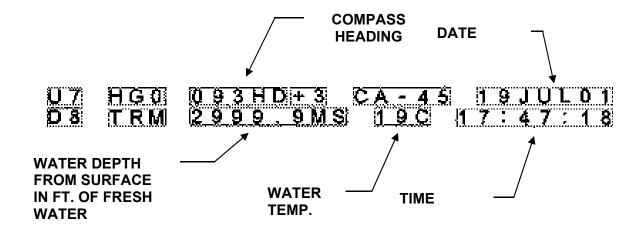




Remotely Operated Vehicle Underwater Inspection

The above report is based on our interpretation of the enclosed video in addition to our observations of the structure at the time of inspection.

VIDEO OVERLAY LEGEND



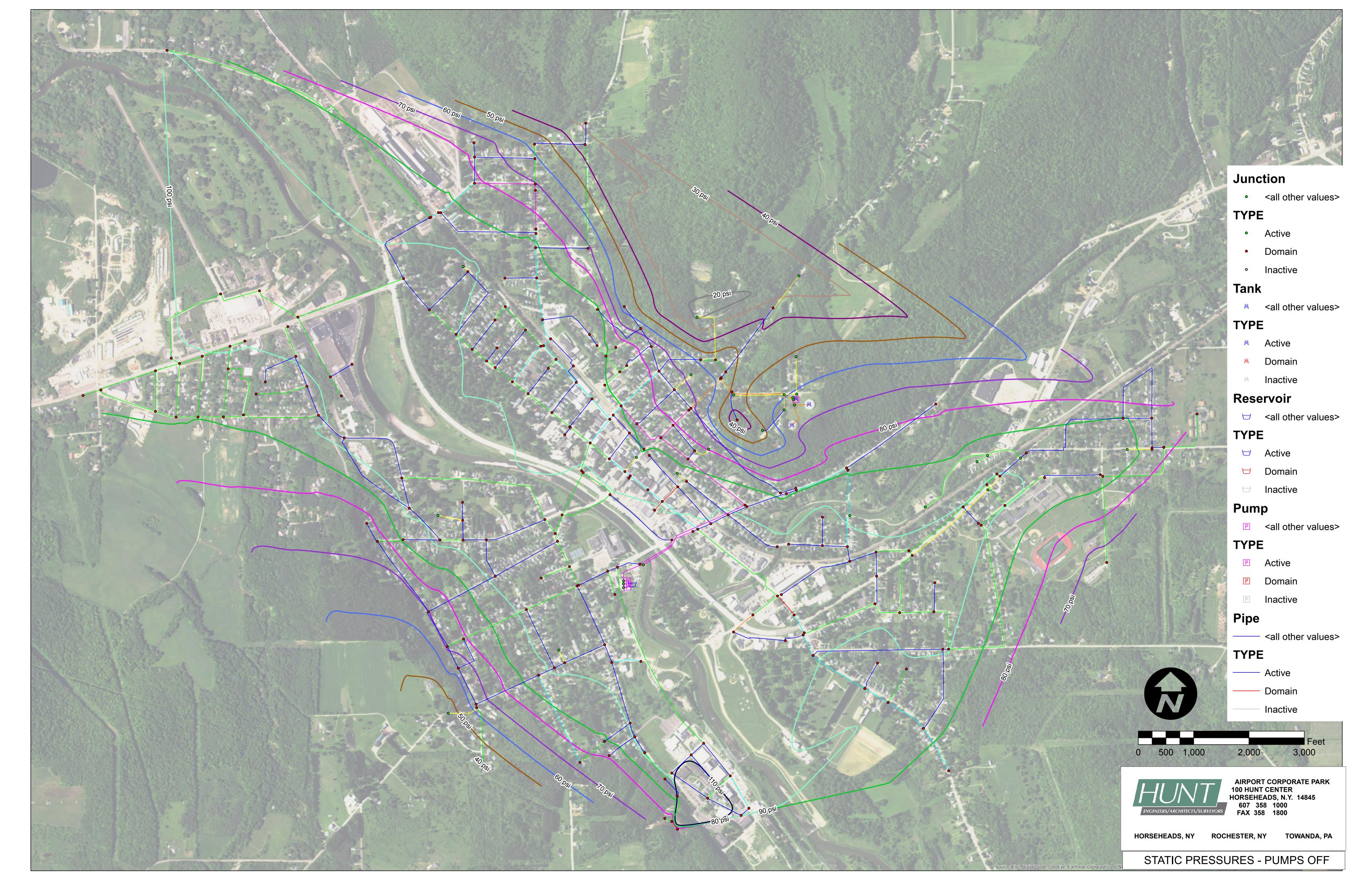
DURING THE INSPECTION THE R.O.V. IS CALIBRATED TO FEET AND TENTHS OF A FOOT AND DEGREES FAHRENHEIT.

(THE LEGEND VIEW ABOVE, PROVIDED BY THE MANUFACTURER, INDICATES METERS OF SEA WATER AND DEGREES CENTIGRADE)

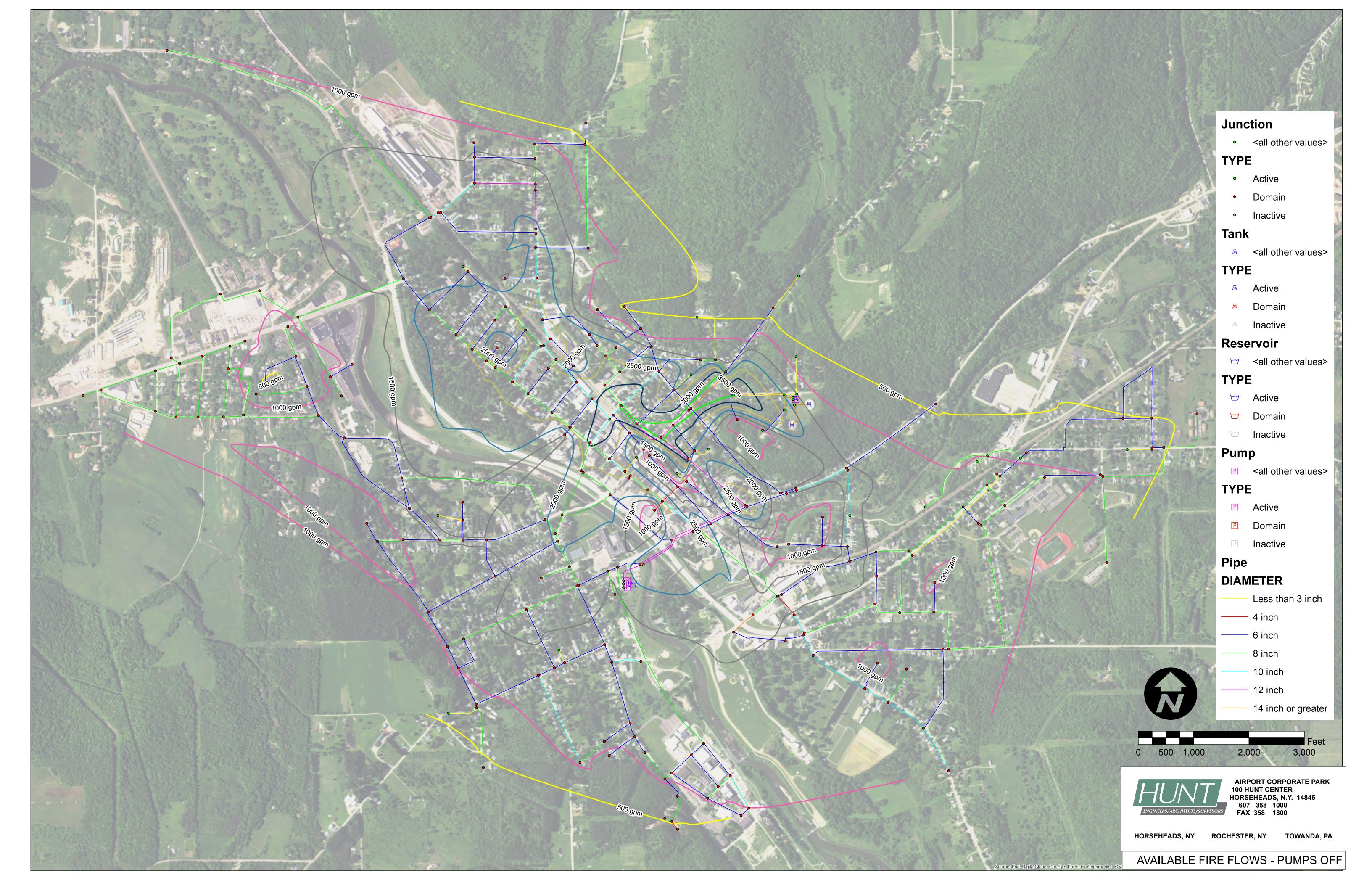
THE R.O.V. DEPTH INDICATOR IS CALIBRATED PRIOR TO DIVING TO SHOW 0.0 FT. OF DEPTH AT THE WATER COLUMN SURFACE INSIDE THE STRUCTURE AT THE TIME OF CALIBRATION.

THE REMAINDER OF THE OVERLAY CONTAINS R.O.V. OPERATIONAL INFORMATION

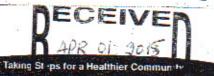
APPENDIX H STATIC PRESSURE MAPPING



APPENDIX I FIRE FLOW MAPPING



APPENDIX J ALLEGANY COUNTY DOH 2015 SANITARY SURVEY



ALLEGANY COUNTY DEPARTMENT OF HEALTH

7 Court Street, County Office Building, Room 30 Belmont, NY 14813-1076

Loreen Ballengee, MS Public Health Director E-mail: ballenl@alleganyco.com

585-268-9250 Fax: 585-268-9264 Thomas E. Hull, MS
Deputy Public Health Director
Director of Environmental Health
E-mail: hullte@alleganyco.com

March 18, 2015

Wellsville Village Board PO Box 591 Wellsville, NY 14709

RE: 2015 Sanitary Survey-Wellsville Village Water Supply

Dear Mayor Lynch and Village Board Members:

On March 16, 2015, I conducted a sanitary survey of the public water system serving the Village of Wellsville. Your Chief Water Treatment Plant Operator, Dana Harris, assisted me. The inspection evaluated eight system components in accordance with Subpart 40-2 of the New York State Sanitary Code. Deficiencies, requirements, or recommendations from the inspection are documented in the component area to which they apply. If there were no problems in a specific area, this is indicated. The inspection results are as follows:

Source-No problems or deficiencies were noted at the time of the inspection. During that time the turbidity levels of the river were higher than normal, but not to the point where the filters were unable to achieve adequate effluent turbidity levels. The operator was monitoring the influent and effluent levels and was ready to take the appropriate measures should the levels exceed the acceptable limits.

Treatment-No problems or deficiencies were noted at the time of the inspection.

Distribution System-No problem or deficiencies were reported at the time of the inspection. At the time of the inspection it was unknown if all of the backflow prevention devises throughout the distribution system had received their required annual testing, as Mr. Harris is not in charge of that aspect of the water system. Please ensure that all of the devises have been tested and that the reports are on file with the appropriate person.

Finished Water Storage-No problems or deficiencies were noted at the time of the inspection. You should consider the installation of a fence around the reservoir area to discourage unauthorized access.

Pumps and Controls-No problems or deficiencies were noted at the time of the inspection.

Monitoring and Reporting-At the time of the inspection all of your system's monitoring and reporting requirement had been met.

System Operation and Management-No problems or deficiencies were noted at the time of the inspection. The operators have taken measures to aid in the remote operating and monitoring of the plant by installing video cameras at critical components and readouts, which allow them to be viewed on their cellphones at any location where cell-service is available. This innovation is to be commended and will be recommended to other water systems throughout the county.

Operator Certification-At the time of the inspection your system had the following New York State Health Department certified operators: Dana L. Harris, Grades IIA & C; Brent W. Roberts, Grades IIA, C, & D; Daniel S. Gardner, Grades IIA & D; Joshua Fry, Grade D; and Wayne Stonemetz, Grade D. Please notify this department should this status change.

A copy of the official inspection report, DOH-4234, is enclosed for your records.

If you have any questions or concerns, please contact this office.

Sincerely,

Richard G. Haywood

Public Health Sanitarian

NEW YORK STATE DEPARTMENT OF HEALTH Bureau of Water Supply Protection

Water System Field Compliance Report: A Review of Compliance with Subpart 5-1 of the New York State Sanitary Code

d Visit Ty	Pre-operational Comptaint Incident	e Type Dish	fecti	NC NTNC [lace Ground Waiver Issued?	□ GWUDI		/ / No	3
Part 5 Subsection	Summary Description of Sanitary Code Requirement	SDWIS	Stat	Part 5 Subsection	Deficiency	Summary Description of Sanitary Code Requirement	SOWAS	Stat
5-1.12(a)	Appropriate actions are taken in response to deteriorating source water quality or diminished effectiveness of treatment with potential for MCL violation.	SA	L	5-1.23(c)	М	Conspicuous posting of Senitzry Code Section 5-1-23, "Reporting Emergencies."	SF	1
	Obtain health department approval prior to the construction	-		5-1.29	S M	Finished (treated) water used for printing pumps.		4
5-1.22(a)	or modification of a water system.	SB	14	5-1.30	SM	Redundant disinfection equipment provided.	ND	1
5-1.23(a)	Obtain health department approval prior to use of an emergency water supply or alteration of a treatment process necessary to protect public health.	50	4	5-1.30 (a) 5-1.31(a)(3)	S M	Complies with disinfection earlier provision. Cross connection control program is implemented by supplier of water, including	ND SJ	4
5-1.27	Maintain minimum distribution system pressure of 20 psi at ground level.	SH		5:1.72(c)	S M	records of all device testing. Complete deily records of operation of a water system.	09	7
5-1.30	Bypass of any stage of treatment.	52	1	5-1.72(d)	S M	Maintain records (e.g., semple results, reports, filter backwash recycle flow information).	09	1
5-1.30	Disinfection of a groundwater source, surface water source or groundwater source influenced by surface water.	ND 41	1			System is in compliance with Subpart S-4. The correct number and level of operator(s) are	50	1
5-1.30(b)	Filtration of surface source and groundwater influenced by surface water unless avoidance criteria is met.	42	1	5-I.72(b)	S	available during plant operation. System has designated operators of appropriate grade level in responsible charge.	SQ SY 12	1
	Free chlorine residual disinfection concentration in the water entering the distribution system must be at least 0.2 mg/l			5-1.73	S.M.	Provide or have available test kit.		T
5-1,30(6)(2)	and may not be less than the minimum concentration for compliance for more than four hours. Systems using other chemical disinfectants shall maintain residual disinfection levels entering the distribution system comparable to	41	1	5-1.73 App.5-A 3.2.1	S M	Developed well sources sufficient to meet maximum day demand with the largest well out of service.		4
5-1.30(q)	requirements for systems using chlorination, Maintain free chlorine residual at representative points in		-	App.5-A.6.1	S	Pumps are accessible for maintenance and 3 feet above the 100 year flood plain.		4
	the distribution system. Project the water distribution system from the continue of conce	NR .	Ц	App.5-A.703	S M	Water tanks, hatches, roofing, and access ways are watertight, vermin proof, and secure.		1
5-1.31	connections of sufficient hazard to adversely affect the health of water consumers.	53	1	App.S-A.70,7	S M	Tank overflow terminates 12*-24" above grade with proper screen on outlet.		-
5-1.71(a)	Exercise due care and diligence in the maintenance and supervision of all sources of the public water to prevent so far as possible, their pollution and depletion.	SN	1	App.5-B.2(d)	S M	Finished grade of well is mounded to divert surface water.		4
5-1.71(b)	Exercise due care and diligence in the operation and main- tenance of a water treatment plant and distribution system.	SO	1	Арр.5-8.5(g)	Š	Vented, water tight, vermin proof sanitary seal well cap.	SO	4
Have all outs	standing violations been resolved? Yes No		1	App.5-0.3(b)	S M	Well casing in good condition and more than 18" above grade.	V	4
Explain	Tie Duo			Chlorine Resi Point of Collec Chlorine Resi Point of Collec	tion	mg/l Sample Collection Timemg/l Sample Collection Time		
nents <u>S</u> 10 041		st a	11	ogckflo.	o Prev	leation devises are	rece	زب
leted by	Kuly b. Hawood					Date3/	201	5
1				5.		to account to		

Chapter 1. State Sanitary Code Part 5. Drinking Water Supplies Subpart 5-1. Public Water Systems

Statutory Authority: Public Health Law, Section 225
http://www.health.ny.gov/environmental/water/drinking/regulations

Public Health Hazards	Part 5 Requirements
1, 5-1.12(a)	13. 5-1.23(c)
2. 5-1.22(a)	14, 5-1,28
3. 5-1.23(b)	15. 5-1.30
4. 5-1.25	16. 5-1.30(e)
5. 5-1.27	17. 5-1.72(b)
6. 5-1.30	18. 5-1.72(c)
7. 5-1.30(a)(b)	19. 5-1.72(d)
8. 5-1.30(b)	20. 5-1.72(c)
). 5-1.30(g)	Appendix 5-B Requirements
0. 5-1.31	21. App. 5-B.2(d)
1. 5-1.71(a)	22. App. 5-B.5(g)
2. 5-1.71(b)	Appendix 5-D Requirements 23. App. 5-B.3(b)

Instructions for Completion of DOH-4234

- 1. Provide all information regarding name of facility, facility code, type of inspection, etc.
- 2. Within the boxes under Status, indicate the "status code explanation" number (1-5). Any items with a violation(s) (Status Code 2) must be entered into SDWIS.
- 3. Within the boxes under Deficiency, circle the corresponding deficiency code IS; M, R) which must be entered into SDWIS.
- 4. Describe violation(s) in the comments section of the form.
- 5. All violations listed on this form must be entered into both the SDWIS Site Visit module and the SDWIS Violation module. All violations listed under Public Health Hazard shall be entered as a significant deficiency in the Site Visit module as well as a violation in the Violation module.

Note to Water System Operator

- 1. Refer to Subpart 5-1 of the State Sanitary Code (SSC) to review specific requirements.
- 2. For violations noted under:
 - Standards for Water Wells, refer to Appendix 5-B
 - Special Requirements for Wells Serving Public Water Systems, refer to Appendix 5-D
 - Appendix 5-A, Recommended Standards for Water Works, 2007 Edition

APPENDIX K ASSET MANAGEMENT

					Village of Wellsville Water	er System Inventory Works	sheet (HUNT	1861-034)								
Date Worksheet Completed/	Updated: January 2017														Amortized Annual Costs	,
Asset	Description	Installation Date	Condition	Service History	Maintenance Needed	Expected Usefu Life (yrs)	I Adjusted Useful Life (yrs)	Age (yrs)	Remaining Useful Life (yrs)	Description of Cost	Replacement Cost	Installation Cost	Total Cost	Short Term Assets	Long Term Assets	Preventative Maintenance
Nater Treatment Plant																
Raw Water Supply Pump 1	15 HP	1990	Good			15-20	40	27	13	Cost to replace raw water supply pump	\$16,800.00	\$5,000.00	\$21,800.00	\$1,676.92		
Raw Water Supply Pump 2	15 HP	1990	Good			15-20	40	27	13	Cost to replace raw water supply pump	\$16,800.00	\$5,000.00	\$21,800.00	\$1,676.92		
Raw Water Supply Pump 3	25 HP	1990	Good			15-20	40	27	13	Cost to replace raw water supply pump	\$17,000.00	\$7,000.00	\$24,000.00	\$1,846.15		
Raw Water Supply Pump 4	25 HP	1990	Good			15-20	40	27	13	Cost to replace raw water supply pump	\$17,000.00	\$7,000.00	\$24,000.00	\$1,846.15		
VFD for 15 HP Supply Pumps	2 VFDs	1990	Good			10-15	35	27	8	Cost to replace VFDs for supply pumps	\$13,000.00	\$1,500.00	\$14,500.00	\$1,812.50		
VFD for 25 HP Supply Pumps	2 VFDs	1990	Good			10-15	35	27	8	Cost to replace VFDs for supply pumps	\$13,000.00	\$1,500.00	\$14,500.00	\$1,812.50		
Treatment Building		1990	Good			40-60	60	27	33	Cost to construct a new water treatment building	\$2,457,500.00	-	\$2,457,500.00		\$74,469.70	
						40-50	50	27	23	Cost to replace filter	\$275,000.00	\$200,000.00	\$475,000.00		\$20,652.17	
Filter Unit 1	Mixed-media filter	1990	Good			20	40	27	13	Cost to remove and replace filter media	\$80,000.00	=	\$80,000.00	\$6,153.85		
						40-50	50	27	23	Cost to replace filter	\$275,000.00	\$200,000.00	\$475,000.00		\$20,652.17	
Filter Unit 2	Mixed-media filter	1990	Good			20	40	27	13	Cost to remove and replace filter media	\$80,000.00	-	\$80,000.00	\$6,153.85		
						40-50	50	27	23	Cost to replace filter	\$275,000.00	\$200,000.00	\$475,000.00		\$20,652.17	
Filter Unit 3	Mixed-media filter	1990	Good			20	40	27	13	Cost to remove and replace filter media	\$80,000.00	-	\$80,000.00	\$6,153.85		
Chlorinator 1	Chlorine gas used for disinfection; Siemens	2002	Good			20-25	25	15	10	Cost to replace gas chlorinator with automatic switchover	\$2,300.00	\$1,150.00	\$3,450.00	\$345.00		
Chlorinator 2	Chlorine gas used for disinfection; Siemens	2002	Good			20-25	25	15	10	Cost to replace gas chlorinator with automatic switchover	\$2,300.00	\$1,150.00	\$3,450.00	\$345.00		
Chlorinator Booster Pump	Pump to boost flow in order to create a vacuum for chlorine gas disinfection system	2002	Good			15-20	30	15	15	Cost to replace booster pump	\$1,500.00	\$750.00	\$2,250.00	\$150.00		
Cylinder Scale 1	3.10.11.2 gas same so o y same	2002	Good			20-25	30	15	15	Cost to replace scales for chlorine gas cylinders	\$2,000.00	\$600.00	\$2,600.00	\$173.33		
Cylinder Scale 2		2002	Good			20-25	30	15	15	Cost to replace scales for chlorine gas cylinders	\$2,000.00	\$600.00	\$2,600.00	\$173.33		
Chlorination Gas Detection System		2002	Good			25	25	15	10	Cost to replace gas detection monitoring system	\$800.00	\$400.00	\$1,200.00	\$120.00		
Emergency Eye Wash		2002	Good			25	25	15	10	Cost to replace emergency eye wash	\$300.00	\$150.00	\$450.00	\$45.00		
Chlorine Analyzer	Hach	2002	Good			25	25	15	10	Cost to replace chlorine analyzer	\$950.00	\$475.00	\$1,425.00	\$142.50		
Chemical Feed Pump	Feed pump for fluoridation of water	2002	Good			10-15	20	15	5	Cost to replace chemical feed pump	\$850.00	\$425.00	\$1,275.00	\$255.00		
Treated Water Flow Meter	Turbine flow meter; Model: Water Specialities 7-TR15-S; 12 inch; S/N: 20083093	2009	Good	Calibrated on 5/13/16: 1%	Calibration showed no adjustment is necessary	25	25	8	17	Cost to replace meter	\$19,000	\$4,500.00	\$23,500.00	\$1,382.35		
Clearwell Inlet Flow Meter	Turbine flow meter; Model: Water Specialities TR15; 12	2009	Good	difference, no adjustment Calibrated on 5/13/16: 1%	Calibration showed no adjustment is necessary	25	25	8	17	Cost to replace meter	\$19,000	\$4,500.00	\$23,500.00	\$1,382.35		
Treated Water Chart I	inch: S/N: 20081718-12 Model: Honeywell DR45AT; Type: 7 day round chart; S/N:	1990	Good	difference, no adjustment Calibrated on 5/13/16: 0%	Calibration showed no adjustment is necessary	50-60	60	27	33	Cost to replace chart recorder	\$2,500	\$750.00	\$3,250.00		\$98.48	
	C4000000466114; Max flow rate: 2250 gpm Model: Honeywell DR45AT; Type: 7 day round chart; S/N:	1990	Good	difference, no adjustment Calibrated on 5/13/16: 0%	Calibration showed no adjustment is necessary	50-60	60	27	33	Cost to replace chart recorder	\$2,500	\$750.00	\$3,250.00		\$98.48	
Recorder	C4000000466113; Max flow rate: 2250 gpm			difference, no adjustment		60-80	80	27	53	Cost to demolish and rebuild	\$500,000	-	\$500,000.00		\$9,433.96	
Clearwell	500,000 gallon clearwell for treated water from plant	1990	Good		Inspection: every 3-5 years according to AWWA standards	3-5	32	27	5	clearwell Cost to inspect clearwell	\$2,000	-	\$2,000.00		***,******	\$400.00
	125 HP, Goulds with US Motors; vertical turbine pump;			Regular maintenance for pump)							_				,
Clearwell Booster Pump 1	manual operation; pumps run approx. 13 hours a day Monday thru Friday; alternated every day	1990	Good	and motor performed regularly via work orders		15-20	35	27	8	Cost to replace booster pump	\$47,000	\$15,000.00	\$62,000.00	\$7,750.00		
Pump 1 Control Valve & Controller	Operated by solenoid valve and used to reduce water hammer	1990	Good			40-50	50	27	23	Cost to replace pump control valve	\$15,000	\$5,000	\$20,000.00		\$869.57	
Pump 1 Check Valve	Backflow prevention	1990	Good			40-50	50	27	23	Cost to replace check valve	\$1,700	\$850	\$2,550.00		\$110.87	
Air Release Valve 1	Air release valve on discharge line of pump	1990	Good			15-20	35	27	8	Cost to replace air release valve	\$800	\$400	\$1,200.00	\$150.00		
Clearwell Booster Pump 2	125 HP, Goulds with US Motors; vertical turbine pump; manual operation; pumps run approx. 13 hours a day Monday thru Friday; alternated every day	1990	Good	Regular maintenance for pump and motor performed regularly via work orders		15-20	40	27	13	Cost to replace booster pump	\$47,000	\$15,000.00	\$62,000.00	\$4,769.23		
Pump 2 Control Valve & Controller	Operated by solenoid valve and used to reduce water hammer	1990	Good			40-50	50	27	23	Cost to replace pump control valve	\$15,000	\$5,000	\$20,000.00		\$869.57	
Pump 2 Check Valve	Backflow prevention	1990	Good			40-50	50	27	23	Cost to replace check valve	\$1,700	\$850	\$2,550.00		\$110.87	
Air Release Valve 2	Air release valve on discharge line of pump	1990	Good			15-20	35	27	8	Cost to replace air release valve	\$800	\$400	\$1,200.00	\$150.00		
Clearwell Booster Pump 3	60 HP, Goulds with US motors; vertical turbine pump; smaller pump is run if clearwell is full/high; manual operation	1990	Good	Regular maintenance for pump and motor performed regularly via work orders		15-20	40	27	13	Cost to replace booster pump	\$34,500	\$11,500	\$46,000.00	\$3,538.46		
Pump 3 Control Valve & Controller	Operated by solenoid valve and used to reduce water hammer	1990	Good			40-50	50	27	23	Cost to replace pump control valve	\$15,000	\$5,000	\$20,000.00		\$869.57	
Pump 3 Check Valve	Backflow prevention	1990	Good			40-50	50	27	23	Cost to replace check valve	\$1,700	\$850	\$2,550.00		\$110.87	
Air Release Valve 3	Air release valve on discharge line of pump	1990	Good			15-20	35	27	8	Cost to replace air release valve	\$800	\$400	\$1,200.00	\$150.00		

					Village of Wellsville Water System	Inventory Works	heet (HUNT	1861-034)								
Date Worksheet Completed	d/Updated: January 2017														Amortized Annual Costs	į
Asset	Description	Installation Date	Condition	Service History	Maintenance Needed	Expected Useful Life (yrs)	Adjusted Useful Life (yrs)	Age (yrs)	Remaining Useful Life (yrs)	Description of Cost	Replacement Cost	Installation Cost	Total Cost	Short Term Assets	Long Term Assets	Preventative Maintenance
Water Storage																
Water Storage Tank 1	2.0 MG volume, 130 feet in diameter, 20 feet deep;	2001	Good	Last inspected August 2014	Inspection determined that no immediate attention was required.	100	100	16	84	Replacement of tank and foundation	\$1,500,000	\$500,000	\$2,000,000		\$23,809.52	
Water eterage raint i	Natgun pre-stressed, wire reinforced concrete tank	2001	0000	Zaot mopostoa / tagaot Zo i i	Inspection: AWWA D103 recommends that all potable storage tanks be inspected every 5 years	5	5	3	2	Cost to inspect storage tank	\$2,000	-	\$2,000			\$1,000.00
Water Storage Tank 2	2.0 MG volume, 130 feet in diameter, 20 feet deep;	2001	Good	Last inspected August 2014	Inspection determined that no immediate attention was required.	100	100	16	84	Replacement of tank and foundation	\$1,500,000	\$500,000	\$2,000,000		\$23,809.52	
Water olorage rank 2	Natgun pre-stressed, wire reinforced concrete tank	2001	0000	East Inspected August 2014	Inspection: AWWA D103 recommends that all potable storage tanks be inspected every 5 years	5	5	3	2	Cost to inspect storage tank	\$2,000	-	\$2,000			\$1,000.00
Distribution System																
Valves	One valve broken in closed position during exercising program; 369+/- valves			Valves last exercised in 2010 (approx)	The Village owns a valve exerciser and used to exercise valves on a regular schedule. The Village should implement a valve exercising program. One valve broken in closed position should be replaced.					Cost to replace all isolation valves within the system	\$959,400	\$479,700	\$1,439,100			
Hydrants	283+/- hydrants									Cost to replace all the hydrants within the system	\$1,698,000	\$849,000	\$2,547,000			
Booster Pump Building		2006	Good			40-60	60	11	49	Cost to replace building and electric	\$30,000	-	\$30,000		\$612.24	
Booster Pump 1 & Controls	Booster pump located near concrete storage tanks; 3 HP	2006	Good			15-20	20	11	9	Cost to replace booster pump	\$4,000	\$2,000	\$6,000	\$666.67		
Booster Pump 2 & Controls	Booster pump located near concrete storage tanks; 3 HP	2006	Good			15-20	20	11	9	Cost to replace booster pump	\$4,000	\$2,000	\$6,000	\$666.67		
Hydropneumatic Tank		2006	Good			20-25	25	11	14	Cost to replace hydropneumatic tank	\$500	\$100	\$600	\$42.86		
Master Meter	Master meter for water used by Scio					25	25	15	10	Cost to replace meter	\$12,000	\$4,000	\$16,000	\$1,600.00		
Distribution Piping	Diameters ranges from 4in to 14 in; Pipe Material: asbestos transite, cast iron, copper, ductile iron, galvanized, plastic; 27.62 miles of distribution piping			9 leaks repaired in 2015; 9 leaks repaired in 2014; 3 leaks repaired in 2016 up until March 15						Cost to replace water system's entire distribution system	\$25,077,840	\$12,538,920	\$37,616,760			
Meters																
Meter Software	AMR handheld devices for drive by meter readings															
Service Connection Meters	Approximately 2300 service connections; Standardizing or Badger meters			Meter replacement began in 2011 and was completed in 2016.	Residential meter is \$140 each, \$115 for installation	25			25	Cost to replace all residential and small commercial meters	\$320,740	\$263,465	\$584,205	\$ 23,368.20		
	High School - Sensus 4"-6"		Good	Calibrated on 6/15/16: 1% difference, no adjustment		25			13	Cost to replace meter	\$9,375	\$5,715	\$15,090	\$ 1,160.77		
	Elementary School - Sensus 4"-6"		Good	Calibrated on 6/15/16: 1% difference, no adjustment		25			13	Cost to replace meter	\$9,375	\$5,715	\$15,090	\$ 1,160.77		I
	Argentieri Laundry - Rockwell 4'		Poor	Calibrated on 6/15/16: greater than 100% difference	No movement for a long time, kess than one revolution, for a 15 minute span	25			1	Cost to replace meter	\$6,800	\$2,800	\$9,600	\$ 9,600.00		
	Jones Memorial Hospital - Rockwell 4		Poor	Calibrated on 6/15/16: greater than 100% difference	No movement when the flow was below 2.7 CFM	25			1	Cost to replace meter	\$6,800	\$2,800	\$9,600	\$ 9,600.00		
	Manor Care Center Spectrum 260, 2"		Good	Calibrated on 6/15/16: 1% difference, no adjustment		25			13	Cost to replace meter	\$1,080	\$636	\$1,716	\$ 132.00		
	Manor Care Center (behind WWTP) - Rockwell 2"-3"		Good	Calibrated on 6/15/16: 1% difference, no adjustment		25			13	Cost to replace meter	\$2,495	\$1,459	\$3,954	\$ 304.15		
	· · · · · · · · · · · · · · · · · · ·	·										Totals	\$51,361,265	\$98,456.34	\$197,229.75	\$2,400.00

APPENDIX L VILLAGE BUDGET AND WATER RATES

Village of Wellsville

Water Rates/Rate Changes - Effective June 1st, 2015

	<u>Village</u>	<u>Town</u>
Customer Meter Charge per month:	\$16.00	\$24.00
1 to 3 units (per unit)	\$0.57	\$0.86
4 to 50 units (per unit)	\$3.83	\$5.75
51-100 units (per unit)	\$2.64	\$3.96
101 to 150 units (per unit)	\$2.37	\$3.56
over 150 units (per unit)	\$1.38	\$2.07
Meter usage fee *	\$2.00	\$3.00
Outside Village sustamore are shoused at 0/11	-0	

Outside Village customers are charged at %150

Miscellaneous Water Charges:

Account change over charge \$5.00

(applied to solid waste/electric if no water service)

Water turn on or off <u>during</u> work hours: Water turn on or off <u>after hours</u> : Water turn off/on same day:	\$20.00 \$90.00 \$20.00
Non-payment water turn on/off during work hours: Non-payment water turn on/off after hours:	\$40.00 \$80.00
Frost Bottom - meter replacement:	\$150.00
Flow test Testing if meter in working order: Testing if meter is faulty - with replacement	\$50.00 \$0.00

Freeze - ups (line thaw), customer side actual cost >> The village is responsible for service from the main to the curb box. The Customer is responsible for the line from the curb box to the house and inside the house.

New 1" Service Connection Fee \$330.00

The owner digs the ditch and backfills it. The Village taps the main, supplies and installs the line to and including the curb box, patches the street, and supplies. Connections over 1" will be at actual cost.

Street Opening \$150.00

Other calls after working hours: to be billed at actual cost

>> The Village reserves the right to change rates periodically to cover actual costs.

^{*} Usage fee will be combined with Meter charge on monthly billing

Detailed Budget Report Village of Wellsville 2014 to 2015 TENTATIVE BUDGET

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
REVENUES:							
F.2140.000							
WATER SALES	1,115,850.00	1,077,000.00	3.61%	1,056,000.00	679,747.28	1,019,620.92	1,023,310.97
F.2142.000		0.000.00	400.000/	5.000.00	0.00	2.00	0.00
WATER SALES - UNMETERED F.2144.000	0.00	6,000.00	-100.00%	5,000.00	0.00	0.00	0.00
SERVICE CHARGES	9,000.00	6,000.00	50.00%	5,000.00	13,443.46	20,165.19	275,916.50
F.2145.000	•	·		·	·	·	·
SERVICE TAPS - CONNECTION	3,000.00	6,000.00	- 50.00%	4,700.00	0.00	0.00	8,775.00
F.2148.000	40.000.00	45 000 00	00.000/	. 40.000.00	45.000.00	20.000.57	20 100 50
PENALTY ON WATER SALES	12,000.00	15,000.00	-20.00%	12,000.00	15,262.38	22,893.57	22,403.52
TOTAL DEPARTMENTAL INCOME	1,139,850.00	1,110,000.00		1,082,700.00	708,453.12	1,062,679.68	1,330,405.99
F.2401.000							
INTEREST INCOME	150.00	300.00	-50.00%	300.00	100.55	150.83	189.77
		:	_				
TOTAL USE OF MONEY AND PROPERTY	150.00	300.00		300.00	100.55	150.83	189.77
TOTAL REVENUES	1,140,000.00	1,110,300.00		1,083,000.00	708,553.67	1,062,830.51	1,330,595.76
APPROPRIATED FUND BALANCE	0.00	0.00	_	0.00	24,721.58	24,721.58	0.00
TOTAL REVENUE AND OTHER SOURCES	1,140,000.00	1,110,300.00	2.67%	1,083,000.00	733,275.25	1,087,552.09	1,330,595.76
APPROPRIATIONS:							
F.1420.101							
LEGAL - VILLAGE ATTORNEY	3,387.05	3,135.76	8.01%	3,135.76	2,111.05	3,135.76	3,179.34
TOTAL PERSONNEL SERVICES	3,387.05	3,135.76		3,135.76	2,111.05	3,135.76	3,179.34
F.1440.400							
ENGINEERING - CONSULTANTS	4,800.00	4,400.00	9.09%	2,000.00	770.00	4,400.00	4,263.75

WATER FUND TOTAL CONTRACTUAL EXPENSE	2014-2015 TENTATIVE 4,800.00	2013-2014 APPROVED 4,400.00	DIF %	2012-2013 BUDGET 2,000.00	FIRST 8 MONTHS 770.00	8 ESTIMATED 4,400.00	2012-2013 ACTUAL 4,263.75
F.1910.400 SPECIAL ITEMS - GENERAL LIBILITY	18,000.00	19,500.00	-7.69%	20,000.00	13,460.43	13,460.43	14,994.50
SPECIAL HEWS - GENERAL LIBILITY	18,000.00	19,500.00	-7.0376	20,000.00	10,400.43	15,400.45	14,554.50
TOTAL INSURANCE	18,000.00	19,500.00		20,000.00	13,460.43	13,460.43	14,994.50
TOTAL GOVERNMENT SUPPORT	26,187.05	27,035.76		25,135.76	16,341.48	20,996.19	22,437.59
F.5111.200							
VEHICLE MAINT - RESERVE FOR EQUIPMENT F.5111.202	25,000.00	20,000.00	25.00%	6,000.00	0.00	20,000.00	0.00
EQUIPMENT (from reserve fund) F.5111.204	0.00	0.00	#DIV/0!	0.00	24,721.58	24,721.58	0.00
VEHICLE MAINTENANCE - SAFETY EQUIPMENT	0.00	1,500.00	-100.00% _	1,000.00	0.00	1,500.00	0.00
TOTAL EQUIPMENT/CAPITAL OUTLAY	25,000.00	21,500.00		7,000.00	24,721.58	46,221.58	0.00
F.5111.450							
VEHICLE MAINTENANCE - CONTRACTUAL/MISC F.5111.461	1,200.00	300.00	300.00%	400.00	82.74	124.11	374.60
VEHICLE MAINTENANCE -UNLEADED FUEL F.5111.462	6,000.00	7,000.00	-14.29%	7,000.00	3,145.65	4,718.48	5,457.76
VEHICLE MAINTENANCE - UNLEADED FUEL SURC F.5111.463	600.00	600.00	0.00%	731.00	232.47	348.71	392.91
VEHICLE MAINTENANCE - DIESEL FUEL F.5111.465	1,200.00	1,800.00	-33.33%	2,154.00	493.42	740.13	1,413.82
VEHICLE MAINTENANCE - TIRES/TUBES/REPAIR F.5111.468	900.00	900.00	0.00%	1,000.00	47.00	70.50	632.82
VEHICLE MAINTENANCE - GREASE/OIL/ANTI-FR F.5111,473	300.00	300.00	0.00%	300.00	0.00	0.00	0.00
VEHICLE MAINTENANCE - VEH EQUIP F.5111,475	9,000.00	7,500.00	20.00%	7,500.00	6,374.97	9,562.46	9,034.26
VEHICLE MAINTENANCE - GENERATOR MAINT F.5111.476	2,400.00	1,800.00	33.33%	1,450.00	331.28	496.92	282.84
VEHICLE MAINTENANCE - BATTERIES	300.00	150.00	100.00%	100.00	0.00	0.00	0.00

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
F.5111.478 VEHICLE MAINTENANCE - CDL TESTING	0.00	300.00	-100.00%	200.00	0.00	0.00	258.74
TOTAL CONTRACTUAL EXPENSE	21,900.00	20,650.00		20,835.00	10,707.53	16,061.30	17,847.75
TOTAL TRANSPORTATION	46,900.00	42,150.00		27,835.00	35,429.11	62,282.88	17,847.75
F.8310.100		•					
WATER ADMIN - DPW SALARY	17,649.54	17,320.16	1.90%	16,809.12	10,748.47	17,320.16	17,932.17
F.8310.104 WATER ADMIN - BILLING SUPERVISOR F.8310.106	10,224.22	10,023.75	2.00%	10,023.75	6,772.89	10,023.75	10,064.24
VILLAGE TREASURER F.8310.108	10,482.47	9,711.90	7.93%	9,722.32	6,071.79	9,711.90	9,769.01
WATER ADMIN - VILLAGE CLERK F.8310.109	9,241.95	8,986.52	2.84%	8,551.23	5,756.21	8,986.52	8,779.97
WATER ADMIN - DEPUTY VILLAGE CLERK F.8310.111	3,695.69	4,103.01	-9.93%	4,794.07	2,729.39	4,103.01	3,809.48
WATER ADMIN - RECEPTIONIST F.8310.112	8,268.69	7,987.22	3.52%	8,007.30	4,499.78	7,987.22	7,909.41
WATER ADMIN - SECRETATY F.8310.113	7,698.53	7,489.73	2.79%	7,285.42	4,315.51	7,489.73	7,501.09
WATER ADMIN - E/W SECRETARY F.8310.117	2,524.70	2,475.20	2.00%	2,524.70	1,699.61	2,475.20	2,536.02
WATER ADMIN - DATA ENTRY CLERK SALARY F.8310.118	6,548.13	6,323.57	3.55%	6,471.69	3,601.41	6,323.57	5,878.37
WATER ADMIN - METER READING F.8310.124	745.04	4,598.10	-83.80%	5,952.96	3,668.14	4,598.10	8,227.62
DEPUTY DIRECTOR DPW	13,264.09	12,897.95	2.84%_	12,914.17	8,410.63	12,897.95	13,898.79
TOTAL PERSONNEL SERVICES	90,343.05	91,917.10		93,056.73	58,273.83	91,917.10	96,306.17
F.8310.201							
WATER ADMIN - EQUIPMENT	10,000.00	9,125.00	9.59% _	2,000.00	0.00	9,125.00	0.00
TOTAL EQUIPMENT/CAPITAL OUTLAY	10,000.00	9,125.00		2,000.00	0.00	9,125.00	0.00

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
F.8310.401							
WATER ADMIN - COPIER EXPENSE	300.00	300.00	0.00%	400.00	110.53	165.80	78.31
F.8310.402							
WATER ADMIN - COMPUTER SUPPLIES DPW	1,200.00	300.00	300.00%	400.00	1,140.46	1,710.69	434.02
F.8310.403							
WATER ADMIN - OFFICE SUPPLIES	1,200.00	1,200.00	0.00%	700.00	438,08	657.12	352.07
F.8310.405							
WATER ADMIN - CONFERENCES/SCHOOLING	3,000.00	3,000.00	0.00%	2,500.00	295.93	3,000.00	1,874.25
F.8310.406							
WATER ADMIN - ASSOCIATION DUES	600.00	600.00	0.00%	500.00	0.00	0.00	233.00
F.8310.407	'000.00	222.22	0.000/	7 00.00		. WO 40	
WATER ADMIN - COMPUTER SUPPLIES	600.00	600.00	0.00%	500.00	119.40	179.10	493.38
F.8310.408	300.00	600.00	-50.00%	4 000 00	0.00	0.00	788.63
WATER ADMIN - ADVERTISING & PRINTING F.8310.411	300.00	600.00	-50.00%	1,000.00	0.00	0.00	700.03
WATER ADMIN - VILLAGE BOARD EXPENSES	0.00	0.00	#DIV/0!	1,000.00	0.00	0.00	0.00
F.8310.412	0.00	0.00	#DIV/0.	1,000.00	0.00	0.00	0.00
WATER ADMIN - COMPUTER SOFTWARE PLAN	0.00	0.00	#DIV/0!	100.00	0.00	0.00	6.45
F.8310.413	****		,,=	100100	0.00	0.00	5. 10
WATER ADMIN - COMPUTER SOFTWARE TRAINING	600.00	600.00	0.00%	400.00	61.25	91.88	139.75
F.8310.421							
WATER ADMIN - TELEPHONE	1,200.00	1,200.00	0.00%	1,500.00	803.61	1,205.42	909.23
F.8310.422							
WATER ADMIN - CELL PHONE	1,200.00	1,500.00	-20.00%	1,750.00	123.34	185.01	161.88
F.8310.431							
WATER ADMIN - COMPUTER SOFTWARE	300.00	300.00	0.00%	200.00	0.00	0.00	0.00
F.8310.432							
WATER ADMIN - TRAINING	900.00	900.00	0.00%	900.00	0.00	0.00	27.00
F.8310.433							
WATER ADMIN - POSTAGE/POSTAL SUPPLIES	3,600.00	3,600.00	0.00%	4,500.00	2,825.40	4,238.10	3,440.35
F.8310.435							
WATER ADMIN - AUDIT EXPENSES	3,600.00	2,400.00	50.00%	2,500.00	450.00	2,400.00	3,148.03
F.8310.436	000.00	000.00					
WATER ADMIN - TELEPHONE BILLING OFFICE F.8310.437	300.00	300.00	0.00%	300.00	166.39	249.59	265.67
F.001U.40/					•		

WATER FUND WATER ADMIN - GEN OPER BILLING OFFICE	2014-2015 TENTATIVE 300.00	2013-2014 APPROVED 300.00	DIF % 0.00%	2012-2013 BUDGET 500.00	FIRST 8 MONTHS 35.15	8 ESTIMATED 52.73	2012-2013 ACTUAL 62.64
F.8310.438 WATER ADMIN -BILLING OFFICE SUPPLIES F.8310.439	300.00	300.00	0.00%	400.00	111.57	167.36	163.32
WATER ADMIN-COMPUTER MAINTENANCE F.8310.440	3,600.00	3,600.00	0.00%	3,000.00	2,521.53	3,782.30	2,916.38
WATER ADMIN - PAPER F.8310.441	300.00	300.00	0.00%	120.00	537.55	806.33	418.94
WATER ADMIN - CONSULTANT BILLING OFFICE F.8310.450	0.00	0.00	#DIV/0!	0.00	0.00	0.00	0.00
WATER ADMIN - MISCELLANEOUS EXP F.8310.460	600.00	900.00	-33.33%	725.00	848.46	1,272.69	742.14
WATER ADMIN - GIS EXPENSES F.8310.499	1,600.00	1,250.00	28.00%	1,250.00	358.53	537.80	986.22
WATER ADMIN - DPW OFFICE	300.00	300.00	0.00%	300.00	110.41	165.62	0.00
TOTAL CONTRACTUAL EXPENSE	25,900.00	24,350.00		25,445.00	11,057.59	20,867.49	17,641.66
TOTAL WATER ADMINISTRATION	126,243.05	125,392.10		120,501.73	69,331.42	121,909.59	113,947.83
F.8320.425							
SUPPLY & PUMPING - ELECTRIC TREATMENT F.8320.426	15,000.00	15,000.00	0.00%	18,000.00	9,330.51	13,995.77	12,918.03
SUPPLY & PUMPING-GENERATOR PROPANE F.8320.427	2,400.00	2,400.00	0.00%	2,000.00	2,400.00	2,400.00	2,438.98
SUPPLY & PUMPING - GENERATOR MAINT F.8320.450	600.00	600.00	0.00%	500.00	0.00	0.00	0.00
SUPPLY & PUMPING - GENERAL OPERATING F.8320.451	600.00	600.00	0.00%	500.00	8.00	12.00	429.32
SUPPLY & PUMPING - NEW INTAKE PARTS F.8320.452	600.00	600.00	0.00%	500.00	3.80	5.70	68.16
SUPPLY & PUMPING - TELEPHONE & LEASING F.8320.453	300.00	300.00	0.00%	120.00	85.20	127.80	69.32
SUPPLY & PUMPING - CONTROL BOARD F.8320.454	300.00	300.00	0.00%	200.00	0.00	0.00	0.00
SUPPLY & PUMPING - PUMPS/REPAIR	6,000.00	4,000.00	50.00%	5,000.00	304.86	4,000.00	552.11

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
TOTAL CONTRACTUAL EXPENSE	25,800.00	23,800.00		26,820.00	12,132.37	20,541.27	16,475.92
TOTAL SUPPLY AND PUMPING	25,800.00	23,800.00		26,820.00	12,132.37	20,541.27	16,475.92
F.8330.110 PURIFICATION -PLANT OPERATION F.8330.120	142,854.91	116,860.68	22.24%	146,544.26	75,921.82	116,860.68	124,875.07
PURIFICATION - PLANT OPERATION O/T	9,000.00	9,000.00	0.00%_	9,000.00	3,852.74	5,779.11	6,420.98
TOTAL PERSONNEL SERVICES	151,854.91	125,860.68		155,544.26	79,774.56	122,639.79	131,296.05
F.8330.200							
EQUIPMENT	15,000.00	15,000.00	0.00%	5,000.00	0.00	15,000.00	1,375.85
TOTAL EQUIPMENT/CAPITAL OUTLAY	15,000.00	15,000.00		5,000.00	0.00	15,000.00	1,375.85
F.8330.425 PURIFICATION - WTP ELECTRIC F.8330.426	24,000.00	24,000.00	0.00%	23,000.00	12,953.86	19,430.79	28,632.66
PURIFICATION - WTR DIESEL FUEL F.8330.427	3,600.00	3,000.00	20.00%	3,000.00	0.00	3,000.00	2,021.19
PURIFICATION - GENERATOE MAINT F.8330.450	600.00	600.00	0.00%	500.00	0.00	0.00	139.28
PURIFICATION - GEN OPER EXP F.8330.460	300.00	300.00	0.00%	250.00	100.62	150.93	-58.26
PURIFICATION - OUTSIDE LAB TESTING F.8330,461	7,200.00	8,000.00	-10.00%	6,000.00	2,164.46	8,000.00	2,744.11
PURIFICATION - LABORATORY SUPP F.8330.462	900.00	750.00	20.00%	750.00	309.42	464.13	474.48
PURIFICATION - LABORATORY EQUIP F.8330.463	900.00	900.00	0.00%	800.00	1,481.18	2,221.77	313,78
PURIFICATION - FILTER REPAIR P F.8330.464	2,400.00	2,400.00	0.00%	2,000.00	0.00	2,400.00	2,185.77
PURIFICATION - CONTROL BOARD F.8330.465	900.00	300.00	200.00%	500.00	6,693.50	6,693.50	279.92

WATER FUND PURIFICATION - PUMPS/REPAIR	2014-2015 TENTATIVE 4,800.00	2013-2014 APPROVED 4,800.00	DIF % 0.00%	2012-2013 BUDGET 5,000.00	FIRST 8 MONTHS 0.00	8 ESTIMATED 4,800.00	2012-2013 ACTUAL 4,759.57
F.8330.466 PURIFICATION - TOOLS F.8330.467	300.00	300.00	0.00%	500.00	44.97	67.46	97.16
PURIFICATION - ELECTRICAL REPAIR F.8330.468	300.00	300.00	0.00%	200.00	0.00	0.00	0.00
PURIFICATION - MISC EQUIPMENT REPAIR F.8330.480	1,500.00	1,500.00	0.00%	1,500.00	954.02	1,431.03	1,416.74
PURIFICATION - CHEMICAL FEEDER F.8330.494	1,200.00	1,000.00	20.00%	1,000.00	437.08	655.62	962.41
PURIFICATION - MISC CHEMICALS	36,000.00	35,000.00	2.86% _	35,000.00	21,095.97	31,643.96	30,049.67
TOTAL CONTRACTUAL EXPENSE	84,900.00	83,150.00		80,000.00	46,235.08	80,959.18	74,018.48
TOTAL WATER PURIFICATION	251,754.91	224,010.68		240,544.26	126,009.64	218,598.97	206,690.38
F.8340.110 TRANSMISSION & DIST - MAIN LINE REPAIR F.8340.120	81,769.81	88,780.49	-7.90%	54,834.00	64,191.05	88,780.49	80,545.04
TRANS & DIST - MAIN LINE REPAIRS OT	7,000.00	7,000.00	0.00% _	7,000.00	5,008.45	7,512.68	5,756.71
TOTAL PERSONNEL SERVICES	88,769.81	95,780.49		61,834.00	69,199.50	96,293.17	86,301.75
F.8340.200 TRANSMISSION & DIST -RESV. DIST. FUND EQ F.8340.201	0.00	1,200.00	-100.00%	1,000.00	0.00	1,200.00	0.00
TRANSMISSION & DIST - EQUIPMENT F.8340.202	0.00	1,500.00	-100.00%	1,500.00	0.00	1,500.00	0.00
TRANSMISSION & DIST - HYDRANTS	0.00	2,400.00	-100.00% _	3,000.00	2,062.71	2,400.00	874.83
TOTAL EQUIPMENT/CAPITAL OUTLAY	0.00	5,100.00		5,500.00	2,062.71	5,100.00	874.83
F.8340.450 TRANSMISSION & DIST - GEN OPER EXP F.8340.460	1,200.00	900.00	33.33%	1,000.00	624.48	936.72	828.56
TRANSMISSION & DIST - MAIN LINE REPAIR	12,000.00	12,000.00	0.00%	11,750.00	9,922.24	14,883.36	12,899.77

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
F.8340,463							
TRANSMISSION & DIST - HYDRANT REPAIRS	7,000.00	3,600.00	94.44%	3,400.00	2,502.71	3,754.07	821.50
F.8340.465							
TRANSMISSION & DIST - STREET REPAIRS	20,000.00	7,000.00	185.71%	5,000.00	5,568.28	8,352.42	4,449.89
F.8340.466							
TRANSMISSION & DIST - METER PURCHASE	6,000.00	0.00	#DIV/0!	50,400.00	3,848.00	3,848.00	289,988.76
F.8340.468			*				
TRANSMISSION & DIST - RESERVOIR	6,000.00	7,500.00	<i>-</i> 20.00%	500.00	0.00	0.00	0.00
F.8340.469							
TRANSMISSION & DIST - TOOL REPAIR	300.00	300.00	0.00%	100.00	0.00	0.00	78.95
F.8340.470							
TRANSMISSION & DIST - PUMP REPAIR	300.00	300.00	0.00%	300.00	0.00	0.00	0.00
F.8340.471							
TRANSMISSION & DIST - EQUIPMENT REPAIR	300.00	300.00	0.00%	300.00	0.00	0.00	54.97
F.8340.472							
TRANSMISSION & DIST - ELEC DIST SYS	1,200.00	1,200.00	0.00%	1,000.00	292.10	438.15	926.35
F.8340.473							
TRANSMISSION & DIST - MISC HAND TOOL	1,200.00	600.00	100.00%	1,000.00	42.23	63.35	118.25
F.8340.474							
TRANSMISSION & DIST - GRAVEL & CONCR	900.00	900.00	0.00%	1,000.00	0.00	0.00	22.26
F.8340.475	222.22						
TRANSMISSION & DIST - EQUIPMENT RENT	600.00	600.00	0.00%	500.00	0.00	0.00	0.00
F.8340.476	000.00	000.00	0.0004				
TRANSMISSION & DIST - WNYPA RAIL ROAD	600.00	600.00	0.00%	500.00	488.11	732.17	483.13
F.8340.477	4 000 00	4 000 00	0.000/	0.440.00	0.40.04		
TRANSMISSION & DIST-PPE TRAINING/EQUIP	1,800.00	1,800.00	0.00%	2,416.00	946.31	1,419.47	1,911.34
F.8340.478	4 000 00	4 000 00	00 0004				
PROPANE - RESERVOIR	1,200.00	1,800.00	-33.33% _	2,000.00	575.00	862.50	461.16
TOTAL CONTRACTUAL EXPENSE	60,600.00	39,400.00		81,166.00	24,809.46	35,290.19	313,044.89
TOTAL TRANSMISSION AND DISTRIBUTION	149,369.81	140,280.49		148,500.00	96,071.67	136,683.36	400,221.47
F.8341.401							
CONTRACTUAL - MATERIALS	42,000.00	40,000.00	5.00%	40,000.00	38,124.42	38,124.42	34,988.33
			_				. ,

Detailed Budget Report Village of Wellsville 2014 to 2015 TENTATIVE BUDGET

WATER FUND TOTAL CONTRACTUAL EXPENSE	2014-2015 TENTATIVE 42,000.00	2013-2014 APPROVED 40,000.00	DIF %	2012-2013 BUDGET 40,000.00	FIRST 8 MONTHS 38,124.42	8 ESTIMATED 38,124.42	2012-2013 ACTUAL 34,988.33
TOTAL BUILDING AND GROUNDS MAINT	42,000.00	40,000.00		40,000.00	38,124.42	38,124.42	34,988.33
F.8350.118							
BLDG & GROUNDS MAIN - JANITOR	2,483.97	1,594.85	55.75% _	1,513.20	1,548.48	2,322.72	1,602.79
TOTAL PERSONNEL SERVICES	2,483.97	1,594.85		1,513.20	1,548.48	2,322.72	1,602.79
F.8350.423	•						
BLDG & GROUNDS MAIN - NATURAL GAS F.8350.441	6,000.00	8,000.00	-25.00%	8,000.00	2,831.17	4,246.76	5,198.70
BLDG & GROUNDS MAIN - JANITOTIAL SUPPL F.8350.446	600.00	600.00	0.00%	500.00	312.62	468.93	283.23
BLDG & GROUNDS MAIN - PAINT F.8350.449	300.00	300.00	0.00%	400.00	67.66	101.49	0.00
BLDG & GROUNDS MAIN - LAWN CARE F.8350.450	300.00	300.00	0.00%	500.00	286.81	430.22	222.03
BLDG & GROUNDS MAIN - MISCELLANEOUS F.8350.451	1,500.00	900.00	66.67%	1,000.00	531.06	796,59	558.85
BLDG & GROUNDS MAIN - MAINT MATERIAL F.8350.452	1,500.00	1,500.00	0.00%	1,500.00	378.72	568.08	532.47
BLDG & GROUNDS MAIN - ELECTRICAL REPAIR F.8350.453	300.00	300.00	0.00%	300.00	25.00	37.50	152.80
BLDG & GROUNDS MAIN - PLUMBING F.8350.454	300.00	300.00	0.00%	250.00	92.57	138.86	225.35
BLDG & GROUNDS MAIN - MEDICAL SERVICE F.8350.460	0.00	0.00	#DIV/0!	100.00	0.00	0.00	0.00
BLDG & GROUNDS MAIN - GARAGE HEATING F.8350.461	3,000.00	3,600.00	-16.67%	3,300.00	1,059.80	1,589.70	2,542.16
BLDG & GROUNDS MAIN - GARAGE JANITOR F.8350.462	300.00	300.00	0.00%	700.00	76.20	114.30	2.99
BLDG & GROUNDS MAIN - GARAGE TELEPHONE F.8350.463	900.00	700.00	28.57%	1,000.00	434.70	652.05	368.19
BLDG & GROUNDS MAIN - GARAGE OPER EXPEN F.8350.465	600.00	300.00	100.00%	500.00	0.00	0.00	279.62

Detailed Budget Report

Village of Wellsville 2014 to 2015 TENTATIVE BUDGET

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
BLDG & GROUNDS MAIN - GARAGE MAINT	300.00	300.00	0.00%	100.00	0.00	0.00	0.81
F.8350.466	500.00	000.00	0.0070	100.00	0.00	0.00	0.01
BLDG & GROUNDS MAIN -COPIER	300.00	300.00	0.00%	750.00	70.29	105.44	84.05
F.8350.475	000.00	333.33	0.00.0				
BLDG & GROUNDS MAIN - MOWER REPAIRS	300.00	300.00	0.00%	500.00	2.30	3,45	299.85
F.8350.476							
BLDG & GROUNDS MAIN - OXYGEN ACETELYN	0.00	0.00	#DIV/0!	200.00	241.75	362.63	121.00
F.8350.497							
BLDG & GROUNDS MAIN - DRIVEWAY SEALER	650.00	600.00	8.33%	600.00	615.00	922.50	575.00
F.8350.498							
BLDG & GROUNDS MAIN - PHONE SYS	300.00	300.00	0.00%	300.00	203.90	305.85	163.68

TOTAL CONTRACTUAL EXPENSE	17,450.00	18,900.00		20,500.00	7,229.55	10,844.33	11,610.78
FOTAL HOME AND COMMUNITY BASED SERVICES	19,933.97	20,494.85		22,013.20	8,778.03	13,167.05	13,213.57
F.9010.800							
EMPLOYEE BENEFITS - RETIREMENT/NYSER	67,704.60	62,875.53	7.68%	54,082.94	64,732.41	64,732.41	46,531.90
F.9030.800							
EMPLOYEE BENEFITS - SOCIAL SECURITY	25,073.80	24,349.10	2.98%	24,189.16	15,827.18	23,740.77	23,932.16
F.9040.800							
EMPLOYEE BENEFITS - WORKERS COMP.	6,600.00	7,500.00	-12.00%	7,500.00	6,001.00	6,001.00	7,080.00
F.9050.800							
EMPLOYEE BENEFITS - UNEMPLOYMENT INS	0.00	0.00	#DIV/0!	0.00	0.00	0.00	0.00
F.9055.800							
EMPLOYEE BENEFITS - DISABILITY INS	600.00	600.00	0.00%	500.00	135.96	203.94	190.51
F.9060.800	04 000 04	00 074 40	0.450/	25.545.55			
EMPLOYEE BENEFITS - HOSPITALIZATION F.9070.800	34,009.81	32,874.49	3.45%	32,019.93	13,456.11	32,874.49	30,938.99
EMPLOYEE BENEFITS - PERSONAL EQUIPMENT R	0.00	0.00	#011//01	2.22	0.00		
F.9089.800	0.00	0.00	#DIV/0!	0.00	0.00	0.00	0.00
CLOTHING ALLOWANCE	2,000.00	2,000.00	0.00%	2,000.00	700.47	2.000.00	4 700 00
	۷,000.00	2,000.00	U.UU70	2,000.00	790.17	2,000.00	1,799.06
TOTAL EMPLOYEE BENEFITS	135,988.21	130,199.12	,	120,292.04	100,942.83	129,552.61	110,472.62

F.9710.600

Detailed Budget Report

Village of Wellsville 2014 to 2015 TENTATIVE BUDGET

WATER FUND	2014-2015 TENTATIVE	2013-2014 APPROVED	DIF %	2012-2013 BUDGET	FIRST 8 MONTHS	8 ESTIMATED	2012-2013 ACTUAL
SERIAL BOND - PRINCIPAL WTPC PIB F.9710.602	150,000.00	150,000.00	0.00%	150,000.00	0.00	150,000.00	150,000.00
SERIAL BOND - RESERVOIR (New Bond) F.9710.604	40,000.00	40,000.00	0.00%	27,000.00	40,000.00	40,000.00	69,000.00
SERIAL BOND - METERS (New Bond) F.9710.700	50,000.00	44,900.00	11.36%	0.00	0.00	44,900.00	0.00
SERIAL BOND - RESERVOIR INTEREST (New Bond) F.9710.701	47,325.00	48,125.00	-1.66%	73,458.00	24,262.50	48,125.00	18,991.75
SERIAL BOND - INTEREST WTPC PIB F.9710.704	25,500.00	35,700.00	-28.57%	45,900.00	15,300.00	35,700.00	40,800.00
SERIAL BOND - METERS (New Bond)	2,998.00	3,212.00	-6.66% _	0.00	1,579.00	3,212.00	0.00
TOTAL BOND EXPENSE	315,823.00	321,937.00		296,358.00	81,141.50	321,937.00	278,791.75
F.9785.501							
LEASE PAYMENT - BACKHOE	0.00	15,000.00	-100.00%	15,000.00	15,000.00	15,000.00	15,000.00
TOTAL LEASE PAYMENT	0.00	15,000.00		15,000.00	15,000.00	15,000.00	15,000.00
TOTAL DEBT SERVICE	315,823.00	336,937.00		311,358.00	96,141.50	336,937.00	293,791.75
TOTAL APPROPRIATIONS	1,140,000.00	1,110,300.00	2.67%	1,083,000.00	599,302.47	1,098,793.32	1,230,087.21
TOTAL EXCESS (DEFICIT)	(0.00)	(0.00)		0.00	133,972.78	(11,241.24)	100,508.55
Capital Purchases:	0.00	05.000.00		,			·
	0.00	25,000.00 25,000.00		0.00			·

APPENDIX M LEAK REPAIR REPORTS

170 EDYKK

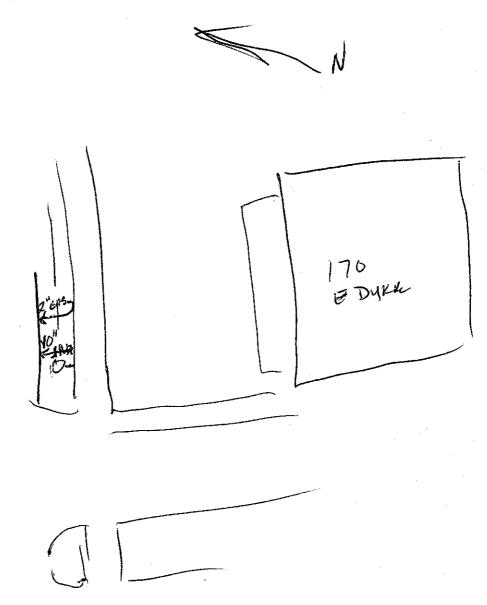
72 WATER AUDITS AND LEAK DETECTION

	C REPAIR REPORT Date 3/15/16
W.O. No.: 3	Foreman: D GANDWKU
LEAK IDENTIFICATION	Map Reference:
Refer to Leak Discovery Report	· · · · · · · · · · · · · · · · · · ·
Discovery Date: 3/14/16	Leak No.: 3
FOR MAIN AND SERVICE LATERAL LEAKS ON	· · · · · · · · · · · · · · · · · · ·
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
Street name; north arrow.	Straight down over leak or damage.
2. Meter number (if applicable).	Close-up of leak and damage.
 Mains and hydrants in shutdown area. 	 Any other photo which you feel will help.
4. All valves (give valve numbers and	nap.
show which were closed during repair). 5. Locate leak to nearest intersection	
or house with address.	
Show distances to property lines or street centerlines.	
Leak Found? (Yes/No)	er et alle en
TYPE OF LEAK	
Meter Leak Main Line Leak	Joint Leak
Meter Spud Leak Service Lateral Le	
Meter Yoke Leak Fire Hydrant Leak	
Curb Stop Leak Valve Leak	
DESCRIPTION OF REPAIR	
	iced If replaced, what material was used? Corpus + Con
If repaired, what repairs were made?	Repair Time 4 (From/To)
Leak Clamp Repacked Valve	Crew Size (persons)
Welded Recaulked Joint	Equipment Used for Repair
Other (describe)	Backhoe
	Dumptruck
landi Carlo	
Repair Costs:	Size of Leak: Measuredgpm
Materials \$	Estimated 30-50,020 DAY
abor \$	
Equipment \$	Melhod Used:
Other \$	
Total \$	

LEAR WAS A BRASS NIPPLE OUT OF COURS STOP

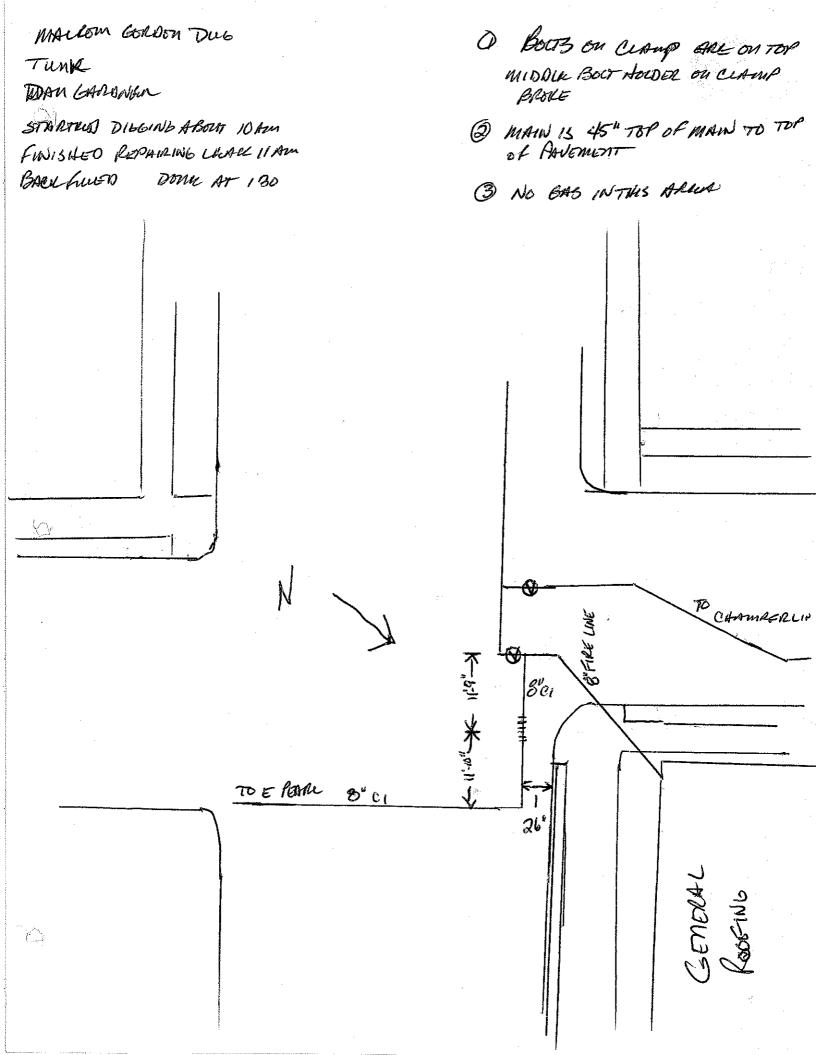
BEOCH AT THREADS REPISEED WITH MALE & COMP COMPACING COMPACING

12" COPPLE PIPE



2" GAS LINE IS 27" WIDER GRASS 10" TOWARDS STREET
FROM CURB BOX INSTALLAND A S.S. ROD AND NEW BOX

agency: IDCSV Hzo	EAK REPAIR REPORT
igency: 1000 INC	Date
V.O. No.: # 2	Foreman: D GANDWAN
EAK IDENTIFICATION	Map Reference:
lefer to Leak Discovery Report	Page and Coordinates:
iscovery Date: 2/9/16	Leak No.: 42 2016
ocation (include street name and number):	
OR MAIN AND SERVICE LATERAL LEAKS	ONLY
ketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
Street name; north arrow.	Straight down over leak or damage.
Meter number (if applicable). Mains and hydrants in shutdown	 Close-up of leak and damage. Any other photo which you feel will
area. All valves (give valve numbers and	help.
show which were closed during repair).	
Locate leak to nearest intersection or house with address.	
Show distances to property lines	
or street centerlines.	
eak Found? (Yes/No) /PE OF LEAK	en e
PE OF LEAK eter Leak Main Line Leai	Joint Leak
eter Spud Leak Service Latera	
eter Yoke Leak Fire Hydrant L	
urb Stop Leak Valve Leak	
ESCRIPTION OF REPAIR	
amaged part was: Repaired Rep	placed If replaced, what material was used?
,	
epaired, what repairs were made?	Repair Time $8 - 11^{30}$ (From/To)
Leak Clamp Repacked Valve	Crew Size 3 (persons)
Welded Recaulked Joint	Equipment Used for Repair
Other (describe)	
pair Costs:	Size of Leak:
terials \$	Measuredgpm
oor \$	Estimate 30 & gpm
uipment \$	
115uieu 6	Method Used: <u>V13 WAC</u> SUR FACULO
ner \$	CALL NO.



Agency: Werebrut WA	The Date 1/20/16
W.O. No.: 12016	Foreman: D GANDWILL
LEAK IDENTIFICATION	Map Reference:
Refer to Leak Discovery Report	Page and Coordinates:
Discovery Date: 1/20/16	Leak No.:
Location (include street name and number):	
FOR MAIN AND SERVICE LATERAL LEAKS	ONLY
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. 	1. Straight down over leak or damage. 2. Close-up of leak and damage. 3. Any other photo which you feel will help. SHUT DOWN EARLY AT BROOKLYN SHALLOW 2 VALVES AT EARLY TAND HAMKTON
Leak Found? (Yes/No)	
TYPE OF LEAK	
Meter Leak Main Line Lea	ık
Meter Spud Leak Service Latera	I Leak Other Leak
Meter Yoke Leak Fire Hydrant L	eak Describe
Curb Stop Leak Valve Leak	
DESCRIPTION OF REPAIR	
Damaged part was: V Repaired V Re	placed If replaced, what material was used?
If repaired, what repairs were made? Leak Clamp Repacked Valve Welded Recaulked Joint Other (describe)	Repair Time SAM 445P (From/To) Crew Size (persons) Equipment Used for Repair Backhoe Dumptruck
Repair Costs:	Size of Leak:
Materials \$	Measuredgpm
_abor \$	6UÉstimated 20-30 gpm
Equipment \$	Melhod Used: Callests
Other \$	

6" el Pipe 15 54" DEEP 11 CIRCUMPRANCE CRACK NORTH APPROX 8-4" FROM & OF HYDRANT EAST TAP FOR 35 EARLY CO APPROX 11'4" & OF HYDRAUT HYDRANT EAST blokk corp off AT 30x4" MAIN FOR 35 EARLY STEEL LINE LEPLACED WITH SANDLE CORP + SWING 90" 4' By copped + speich *8'3"-> A GAB ONTBOARD Of SIDIK WALLS ON PACK SIDE OF STREET FROM BROOKLYN TO HOWILLTON

		REPAIR REPORT
Agency: _/	NCSV HZU	Date 12-11-15
•	·	ELLEN OF GARDINELL
W.O. No.:		Polenian.
LEAK IDEN		Map Reference:
	an older or y this part	Page and Coordinates:
Discovery C	late: 12-11-15	LEAK NO.:
Location (in	clude street name and number):	MANNIBERUIN + NOILIG BOREVELLYN
FOR MAIN	AND SERVICE LATERAL LEAKS ONLY	
Sketch a ma	ap of the site including:	If Main or Service Leak, Attach Three Photos:
2. Mete	t name; north arrow. r number (il applicable).	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will
area.	s and hydrants in shutdown	help.
show	lives (give valve numbers and which were closed during repair).	
5. Loca	te leak to nearest intersection	
Shov	v distances to property lines	
	? (Yes/No)	
TYPE OF L		
Meter Leak		Joint Leak
Meter Spuc	Leak Service Lateral Lea	ok Other Leak
Meter Yoke	Leak Fire Hydrant Leak	Describe
Curb Stop	eak Valve Leak	
DESCRIPT	ION OF REPAIR	ed It replaced, what material was used? Frem CIR
Damaged p	part was: V Repaired Replace	ed If replaced, what material was used?
		Repair Time $\frac{2 20^{-4} / 30^{-6}}{2}$ (From/To)
	what repairs were made?	Repair Time 2 (From/To)
Leak	Clamp Repacked Valve	CIBW 0126
Weld	ded Recaulked Joint	Equipment Used for Repair
Othe	r (describe)	
<u></u>		Dumptruck
Repair Cos	ts:	Size of Leak:
Materials	\$	Measuredgpm
Labor	\$	Estimated 30-30 gpm
Equipment	\$	Melhod Used:
Other	s	
Total	\$	

N BROOKLYN 155 CHAMBEREIN GAS MAIN IS ON OBD SIDE OF STRUET WAGNE GET SOIL BAD CAURING PINNED IN DITCH CHUR IN CAUSED BY AUGTHER DITTER CLUST 155 FROMOBIO WATER MAIN 4 MUNY MAIN 65"DETER n proon GAS SIMULCAL NO STEAL LINE GAS METELS Post 6' Deel CHAMBARIA CHAME BELLIN

NOT TO SCALA

12-11-15 2 Am (Pouck)



	,
/.O. No.:	Foreman: D GAADWUR
EAK IDENTIFICATION	Map Reference:
efer to Leak Discovery Feport	Page and Coordinates:
scovery Date: 3/5/15	Leak No.: 1ST LEARL OF 2015
ocation (include street name and number):	
OR MAIN AND SERVICE LATERAL LEAKS	ONLY
etch a map of the site including:	If Main or Service Leak, Attach Three Photos:
Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will
area. All valves (give valve numbers and	help.
show which were closed during repair). Locate leak to nearest intersection or house with address.	
Show distances to property lines or street centerlines.	
ak Found? (Yes/No)	
PE OF LEAK	
eter Leak Main Line Leal	k 1 Joint Leak
ter Spud Leak Service Lateral	LeakOther Leak
ter Yoke Leak Fire Hydrant Le	eak Describe
rb Stop Leak Valve Leak	
SCRIPTION OF REPAIR	
maged part was: Pepaired Repaired Rep	placed If replaced, what material was used? Fixe Cité C
epaired, what repairs were made?	Repair Time 10 Am 6Pm (From/To)
Leak Clamp Repacked Valve	Crew Size (persons)
Welded Recaulked Joint	Equipment Used for Repair
Other (describe)	- Backhoe
	Dumptruck
pair Costs:	Size of Leak:
erials \$	Measuredgpm
or \$	Estimatedgpm
ipment \$	Method Used:

8"CI MAIN IS APPROX 44" DEED 32° CULB TO \$ OF8" C WANS * Cramo SPUT APPLOX VALUE TO STEVENS

	K REPAIR REPORT
Agency: WLSV IWATER	Date 3/12/15
W.O. No.: # 2	Foreman: D GAROWEL
LEAK IDENTIFICATION	Map Reference:
Refer to Leak Discovery Report	Page and Coordinates:
Discovery Date: 3/12/15	Leak No.:
Location (include street name and number):	
FOR MAIN AND SERVICE LATERAL LEAKS ON	JLY ,
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. 	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will help.
Leak Found? (Yes/No)	
TYPE OF LEAK	
Meter Leak Main Line Leak	Joint Leak
Meter Spud Leak Service Lateral Le	eak X Other Leak
Meter Yoke Leak Fire Hydrant Leak	k Describe
Curb Stop Leak Valve Leak	
DESCRIPTION OF REPAIR	
Damaged part was: Repaired Replace	ced If replaced, what material was used?
<u> </u>	
If repaired, what repairs were made?	Repair Time(From/To)
Leak Clamp Repacked Valve	Crew Size (persons)
Welded Recaulked Joint	Equipment Used for Repair
Other (describe)	Backhoe
	Dumptruck
Repair Costs:	Size of Leak:
Materials \$	Measuredgpm
abor \$	Estimated 7 GPM gpm
Equipment S	Melhod Used:
Other \$	
•	
Total \$	•

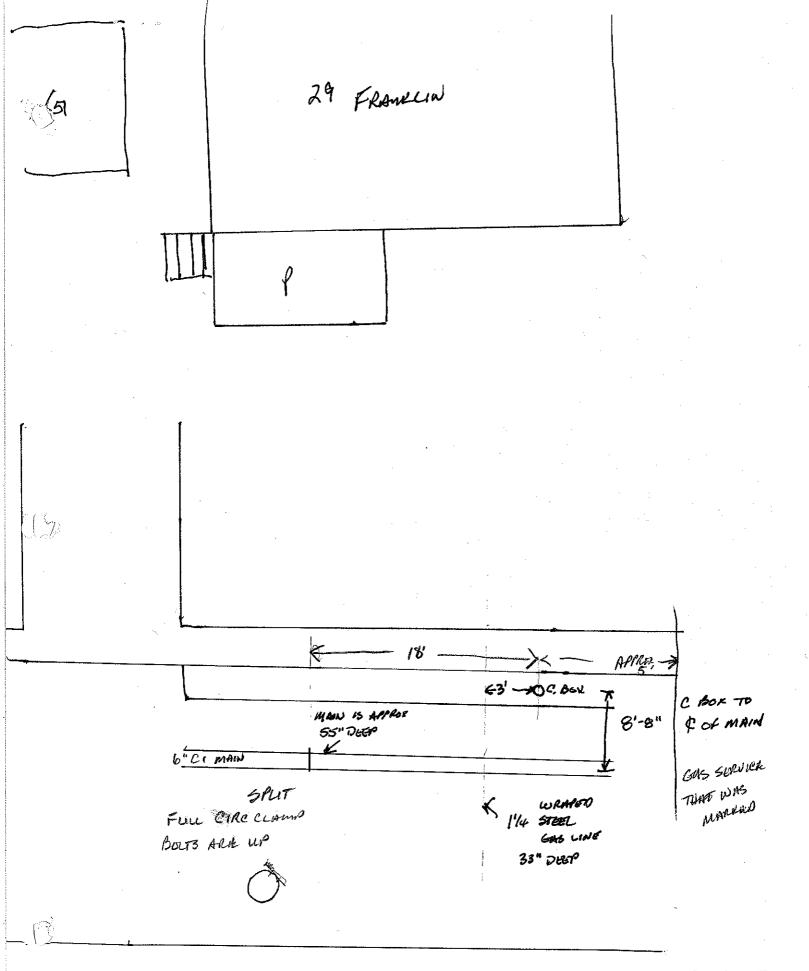
48'-8"

PLOOKS TO BE SI MAIN

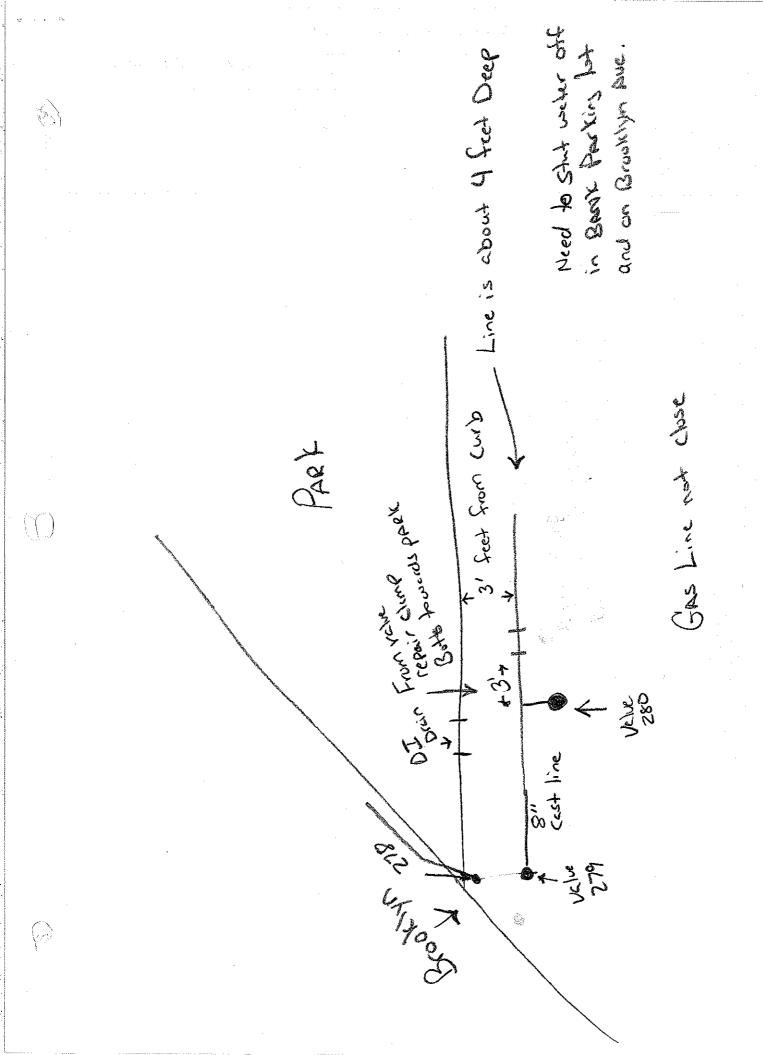
VALVE 316

- 1 PLASTIC GAS LINE NEXT TO CURB BOX (DID NOT SEE)
- (2) CIRON LINE LOOKS LIKE 8" NOT 6" DID NOT UNCOVAR 5'DEP
- 3) APPRO 48'-8" FROM 4 VALVE 316 OLD CORP. STOP SHOT OFF LEAD WHIP FAILED
- @ FROST WAS 5' IN GROWND

Agency: WLSV WATER	Date 3/14/15
W.O. No.: LEAR # 3	Foreman: D GARDNER
LEAK IDENTIFICATION	Map Reference:
Refer to Leak Discovery Report	Page and Coordinates:
1 1	Leak No.:
	ar):
FOR MAIN AND SERVICE LATERAL LE	
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during rep Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. 	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will help.
Leak Found? Yes/	(No)
TYPE OF LEAK	
Meter Leak Main Lin	ne Leak
Meter Spud Leak Service	Lateral Leak Other Leak
Meter Yoke Leak Fire Hyd	trant Leak Describe
Curb Stop Leak Valve Le	eak
DESCRIPTION OF REPAIR	
Damaged part was: Repaired	Replaced If replaced, what material was used?
if repaired, what repairs were made? Leak Clamp Repacked Value Welded Recaulked Join Other (describe)	nt Equipment Used for Repair
Repair Costs:	Size of Leak:
Materials \$	Measuredgpm
s	Estimated 20 6 gpm x 2 HRS
Equipment S	Melhod Used: Guess
Other \$	
Total \$	



LEAK Agency: WESV WHYMUN	REPAIR REPORT Date 3/34/5
W.O. No.: 43	Foreman: Sout Fry
LEAK IDENTIFICATION	Map Reference:
Refer to Leak Discovery Report	Page and Coordinates:
	Leak No.:
<i>,</i> ,	
FOR MAIN AND SERVICE LATERAL LEAKS ON	*
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. 	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will help.
Leak Found? (Yes/No)	
TYPE OF LEAK	
Meter Leak Main Line Leak	Joint Leak
Meter Spud Leak Service Lateral Le	eak Other Leak
Meter Yoke Leak Fire Hydrant Leak	Describe
Curb Stop Leak Valve Leak	
DESCRIPTION OF REPAIR	
Damaged part was: Repaired Replace	ced If replaced, what material was used?
If repaired, what repairs were made? Leak Clamp Repacked Valve Welded Recaulked Joint Other (describe)	Repair Time
Repair Costs:	Size of Leak:
Materials \$	Measuredgpm
Labor S	Estimatedgpm
Equipment \$	Method Used:
Other \$	
Total S	



	\$73. CANN		Date 5/4/15 TEARAD MATTISTA
W.O. No.:	Fo	oreman:	AGICAL PUNTY/1500C
LEAK IDENTIFICATION	Ma	ip Referenc	Ce:
Refer to Leak Discovery Report	Pa	ge and Co	ordinates:
Discovery Date:	Le	ak No.: _	
Location (include street name and	number):		
FOR MAIN AND SERVICE LATE	RAL LEAKS ONLY		
Sketch a map of the site including:		II Mai	in or Service Leak, Attach Three Photos:
 Street name; north arrow. Meter number (if applicable) Mains and hydrants in shutcarea. All valves (give valve numbershow which were closed dutable Locate leak to nearest intersor house with address. Show distances to property or street centerlines. 	town ers and ring repair). section lines	1. 2. 3.	Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will help.
Leak Found?	_ (Yes/No)		and the second s
TYPE OF LEAK	State of the	. /	•
Meter Leak N	Nain Line Leak		Joint Leak
Meter Spud Leak S	Service Lateral Leak		Other Leak
Meter Yoke Leak F	ire Hydrant Leak		Describe
Curb Stop Leak V	alve Leak		
DESCRIPTION OF REPAIR Damaged part was: Repaire	d Replaced	If rep	placed, what material was used? Fcare cu
f repaired, what repairs were made	: ?	Repa	ir Time(From/To)
Leak Clamp Repact	ked Valve	Crew	Size (persons)
Welded Recaul	ked Joint	Equip	ment Used for Repair
Other (describe)			Backhoe
			Dumptruck
Repair Costs:		Size o	f Leak:
		Measu	iredgpm
Materials \$	_		
•			atedgpm
Materials \$		Estima	aled gpm d Used:
Materials \$	<u>-</u>	Estima	

PONCER PAN HOUR LEAST A PISSEN CENTIC THAY CUCONENTED TO CONTINUES ON NEXT PAGE.

hebung wi Las Link y 4" for core com APPROX 5' DEEP E Genesie PIN HOLK IN BOTTOM of 4th CI PIPE BAD SPOT BURNOUT WHEN WEAKARD 4" MAIN + CLEVERS GAS IS BETTWEEN

STREATS CARW + WHYNE DUE THIS

CAST IRON HAS CHART CAKEN JOINTS

29"
Conc.
STORM

A W 18" RIGER

24" Conc. STORM

A 2"

S'A 2 DUE TO TIME CONSTRUNTS

CAS MON

DIO NIT SEE

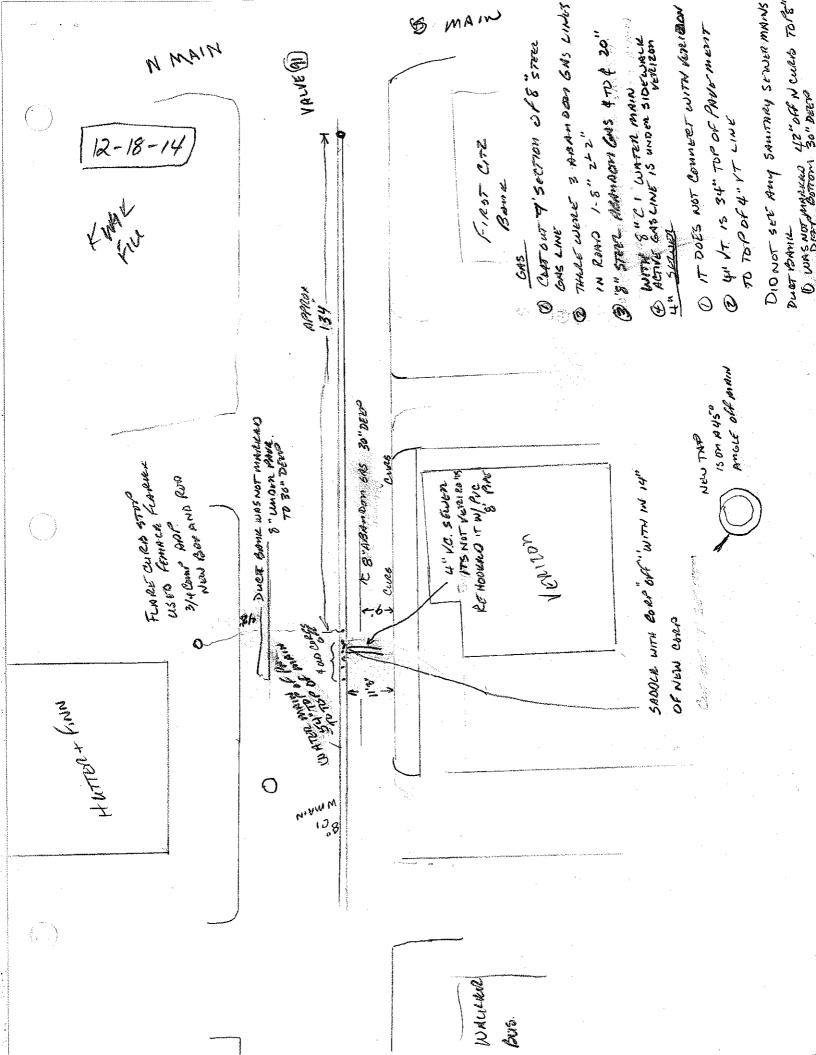
Dub up or 4 DARLING HYDRANT + RURALING WITH 6"K-81

FINE ST SERVICE LEASE 09-2015 FERREND 52 PINE

Agend	(h) 13N		Date 11/10/15
w.o.	No.:		nan: D GANDWAN
	CIDENTIFICATION		eference:
	to Leak Discovery Report	•	and Coordinates:
Dieses	was Data: 11/6/15	look N	No ·
Locati	ion (include street name and number):	1241	W STATE ST
	MAIN AND SERVICE LATERAL LEAKS		
Sketch	h a map of the site including:	•	If Main or Service Leak, Attach Three Photos:
2. 3. 4. 5.	Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines.		 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will help.
Leak F	Found? (Yes/No)		
TYPE	OF LEAK	•	•
Meter	Leak Main Line Lea	ik _	Joint Leak
Meter	Spud Leak Service Latera	al Leak _	Other Leak
	Yoke Leak Fire Hydrant L	.eak _	Describe
Curb S	Stop Leak Valve Leak	_	
	RIPTION OF REPAIR		
-,	ged part was: Repaired Re	placed	If replaced, what material was used?
if repa	aired, what repairs were made?		Repair Time(From/To)
X	Leak Clamp Repacked Valve	٠	Crew Size (persons)
	Welded Recaulked Joint	÷	Equipment Used for Repair
	Other (describe)	·	Backhoe Dumptruck
Repair	Costs:	٤	Size of Leak: SEE ATTACHED
Materia	als \$	IV	wiedsured gpin
abor	\$	ε	Estimatedgpm
Equipm	ment \$	1	Method Used:
Other	\$	-	
Total	s		

6" 68 4" DEBO SAW S.OR. OF 18 APPLAX 7'902 APPROR 6 8" 64

		LEAK REP	AIR REPORT			
Agency:	WELISTUR	WATER	Date 12-18-14			
J , _			7 (
W.O. No.:		Fore	man: D GARDNER			
Refer to Leak Discovery Report Discovery Date: 12-18-14			Map Reference:Page and Coordinates:			
						Leak No.:
			Location (in	iclude street name and nu	mber):	VICKTO ZZ W STATE ST
FOR MAIN	AND SERVICE LATERAL	LEAKS ONLY				
Sketch a m	ap of the site including:	SEE PACK	If Main or Service Leak, Attach Three Photos:			
1. Stree 2. Mete	et name; north arrow. er number (if applicable). es and hydrants in shutdow	·	 Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will 			
area			help.			
shov	which were closed during te leak to nearest intersec	g repair).				
or he	ouse with address.					
	w distances to property line reet centerlines.	35	en e			
Leak Found	d?(Yes/No)				
TYPE OF I	,					
Meter Leak	Mai	n Line Leak	Joint Leak			
Meter Spu	d Leak Ser	vice Lateral Leak	X Other Leak			
Meter Yok	Leak Fire	Hydrant Leak	Describe			
Curb Stop	Leak Val	ve Leak				
DESCRIPT	TON OF REPAIR	and the second	0			
Damaged	oan was: Repaired	Replaced	If replaced, what material was used? Bronz San			
			Repair Time 8 m - 980 Pm (From/To)			
If repaired, what repairs were made?			Crew Size 3 (persons)			
Leak Clamp Repacked Valve			Equipment Used for Repair			
Welded Recaulked Joint			Backhoe			
Oth	er (describe)		Dumptruck			
Repair Costs:			Size of Leak:			
Materials \$			Measuredgpm			
Labor	\$		Estimated/O			
Equipment	\$		Melhod Used: CSUESS			
Other	s					
Total	s	_				



	REPAIR REPORT		
Agency: WESV WATER			
W.O. No.:	Foreman: DAB GANDNER		
LEAK IDENTIFICATION	Map Reference:		
Refer to Leak Discovery Report	Page and Coordinates:		
Location (include street name and number):	Leak No.:STATE ST		
FOR MAIN AND SERVICE LATERAL LEAKS ON			
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:		
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlings. 	1. Straight down over leak or damage. 2. Close-up of leak and damage. 3. Any other photo which you feel will help. (N FRONT OF HYDRANT 5-1 WATEN PLAN ON STRIKKT		
Leak Found? (Yes/No)			
TYPE OF LEAK			
Meter Leak Main Line Leak	Joint Leak		
Meter Spud Leak Service Lateral Le	eak Other Leak		
Meter Yoke Leak Fire Hydrant Leak	Describe		
Curb Stop Leak Valve Leak			
DESCRIPTION OF REPAIR			
Damaged part was: 🔀 Repaired Replac	ced If replaced, what material was used?		
If repaired, what repairs were made?	Repair Time 8 HR3 (From/To)		
Leak Clamp Repacked Valve	Crew Size (persons)		
WeldedRecaulked Joint	Equipment Used for Repair		
Other (describe)			
	Dumptruck		
Repair Costs: O Ama	Size of Leak:		
Repair Costs: O Awr ³ Materials \$	Measuredgpm		
_	Estimatedgpm		
\$	Melhod Used:		
Equipment \$	Midifiloo Osed.		
Other \$			

Joset Aly Works

WATCH PLANT

BE" FROM CLUBS SO & OF MAIN

53" & OF HODRONTVALUE TO & MAIN

55" TOPOF MAIN TO FOPOF PAVELLENT

SPLIT WAS 65" & HID. LAT

TO SPLIT (EAST)

LIETEL PLANIE

SION WALK

POLE 47

HYDRANT

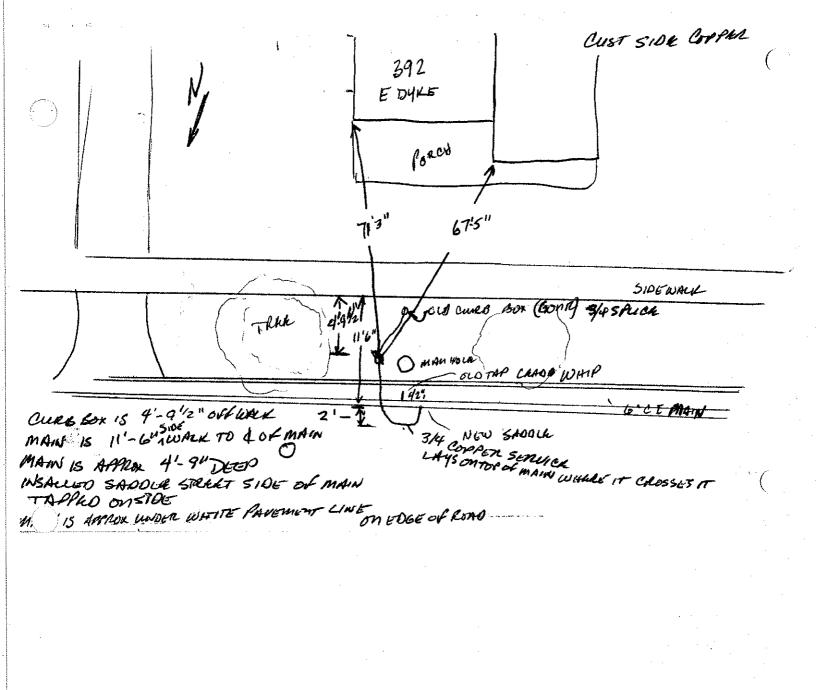
SPLIT (58" 53" 6



BOLTS ARK STICKING UP

LEAK REPAIR REPORT					
Agency: Wilso WATER					
W.O. No.:	Foreman: D Costal Div du				
LEAK IDENTIFICATION	Map Reference:				
Refer to Leak Discovery Report	Page and Coordinates:				
Discovery Date: 10/20/14	Leak No.:				
Location (include street name and number):	2 392 E DYKK IN FRONT OF LA CHA				
FOR MAIN AND SERVICE LATERAL LEAKS OF	· 11				
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:				
or house with address. 5 #0 Show distances to property lines 3/4	1. Straight down over leak or damage. 2. Close-up of leak and damage. 3. Any other photo which you feel will help. I'LOTO WHIP BLEW OUT USED NEW OUT WED NEW OUT WED NEW OUT WED NEW OUT SERTION OF COURS STOP W/BOX CUT SERTION OF				
or street centerlines. Leak Found? (Yes/No)	EATO OUT				
TYPE OF LEAK					
Meter Leak Main Line Leak	Joint Leak				
Meter Spud Leak Service Lateral L	Leak Other Leak				
Meter Yoke Leak Fire Hydrant Lea	ak Describe				
Curb Stop Leak Valve Leak					
DESCRIPTION OF REPAIR					
Damaged part was: Repaired X Repla	aced If replaced, what material was used? LopPen				
If repaired, what repairs were made? Leak Clamp Repacked Valve Welded Recaulked Joint Other (describe)	Repair Time PM 2 Am (From/To) Crew Size (persons) Equipment Used for Repair Backhoe Dumptruck				
Repair Costs:	Size of Leak:				
Materials \$	Measuredgpm				
Labor \$	Estimatedgpm				
Equipment \$	Method Used:				
Other \$	JOSH FRY				

RAJAN STISSEN



W.O. No.:	W.O. No.:		Foreman:		
LEAK IDENTIFICATION			Map Referen	ce:	
Refer to Leak Discovery/Report			Page and Coordinates:		
Location (in	nclude street name	and number): 18-	96 PLA	ubhr	
		ATERAL LEAKS ONL			
Sketch a m	ap of the site inclu	ding:	il Ma	ain or Service Leak, Attac	h Three Photos:
1. Street 2. Mete 3. Main area 4. All vi show 5. Loca or ho	et name; north arro er number (if applic is and hydrants in alves (give valve n w which were close te leak to nearest buse with address.	ow. rable). shutdown umbers and d during repair). intersection	1. 2. 3.	Straight down over leak Close-up of leak and da Any other photo which help.	amage.
	w distances to prop reet centerlines.	ery lines			
_eak Found	1?	(Yes/No)	•	we to the	
TYPE OF L	EAK		V	:	:
Meter Leak		Main Line Leak		Joint Leak	
Meter Spuc	d Leak	Service Lateral Le	ak	Other Leak	
Meter Yoke Leak Fire Hydrant Lea		Fire Hydrant Leak		_ Describe _	· · · · · · · · · · · · · · · · · · ·
Curb Stop I	Leak	Valve Leak		-	
	ION OF REPAIR part was: Re	paired Aeplac	ed If rep	17.7	used? 2° 6° PIPE
				air Time 3 - 9	2-1130
If repaired, what repairs were made?				2	(Persons)
Leak Clamp Repacked Valve			4.5	r Size	_ (persons)
Welded Recaulked Joint			. ,	•	
Other (describe)			<u> </u>	Backhoe Dumptruck	100 B1 "
Repair Cost	ls:		Size	of Leak: 1/2 Hour	-
Materials	\$		Meas	ured	gpm
abor .	S		Estima	ated	gpm

5-14" TOP OF PIPE

APPROX 18" ABOVE TRANSITE

2' TOWARDS CURB 6" CI PIPE

IT HAD A CAPPED END

CEMBER 8-10' OF IT

CAPLACED Z' OF TRANSITE (24-3/4")

CAS ON NORTH SIDE OF STREET

5'-4" TOP OF LINE
6"TRANSITE

APPROX
36'

72 WATER AUDITS AND LEAK DETECTION

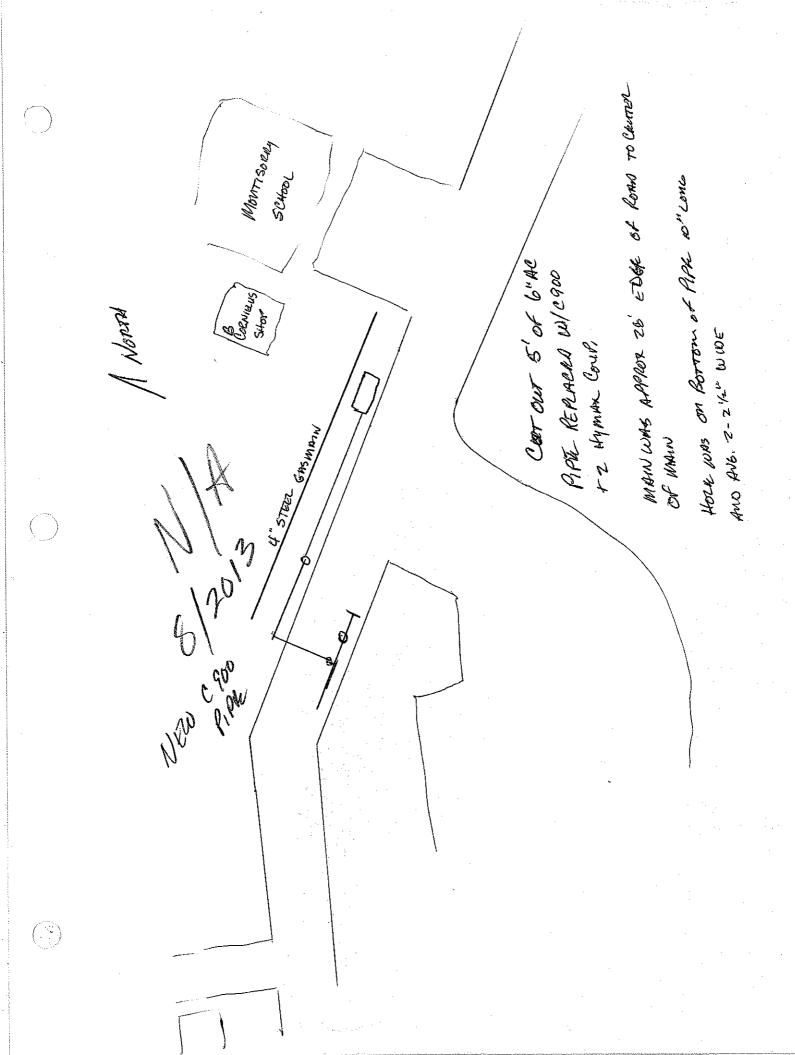
	REPAIR REPORT
gency: Walsville WATE	Date 6/9/14
	Dinama
N.O. No.:	Foreman: D GHAWNENC
A STELON	Map Reference:
EAR IDENTIFICATION	Page and Coordinates:
	Leak No.:
Discovery Date:	
ocation (include street name and number):	
FOR MAIN AND SERVICE LATERAL LEAKS ONL	LY
Sketch a map of the site including:	It Wall of Service Found / When
at the man both arrow.	 Straight down over leak or damage. Close-up of leak and damage.
A Moter number (if applicable).	Close-up of leak and dantage. Any other photo which you feel will
Mains and hydrants in shutdown	help.
area. 4. All valves (give valve numbers and	
show which were closed during repair.	
ar house with address.	
Show distances to properly lines or street centerlines.	
Leak Found? (Yes/No)	
TYPE OF LEAK Main Line Leak	Joint Leak
Meter Leak ———	Other Look
Meter Spud Leak Service Lateral	Dongriba
Meter Yoke Leak Fire Hydrant Lea	ak
Curb Stop Leak Valve Leak	
DESCRIPTION OF REPAIR	placed If replaced, what material was used?
Damaged part was: Repaired Rep	placed If replaced, what material was used?
Damaged part was.	
	Repair Time 48m, 850 pm (From/To)
If repaired, what repairs were made?	Crew Size 4 (persons)
Leak Clamp Repacked Valve	Equipment Used for Repair
Welded Recaulked Joint	
Other (describe)	Backhoe
	Dumptruck
	Size of Leak:
Repair Costs:	Size of Leak: Measured Shu To 536 Pu gpm
Materials \$	Estimatedgpm
Labor \$	Method Used:
Equipment \$	Method Used:
,	
Total S	

Form continues on next page.

						1
		1.00				
						· · · · · · · · · · · · · · · · · · ·
181 577	Ven 1S		104" DUP			
			lo4" neap Top of main		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
		C. Box		\bot	·	
	 	e/o	TAP	6		
			FOR 181 STEVENS			
		12			12'-4"	
		27				**************************************
			N	8	V	
					The state of the s	440.
						8 -0
					and the control of th	
		The state of the s				
					*	Market Country of the

				r L		
					· · · · · · · · · · · · · · · · · · ·	

Agency: WELLSVILLE WATER			D	ate 12/9/12	
W.O. No.:	Foreman:	Døn	Gires	Ukh	
LEAK IDENTIFICATION	Map Refere				
Refer to Leak Discovery Report	·				
Discovery Date: 12/9/12	_		-		
Location (include street name and number): 50	2HOOL ST	IN FRE	OUT DE	Bun Cornicin	3 86
FOR MAIN AND SERVICE LATERAL LEAKS ON					
Sketch a map of the site including:		dain or Serv	ice Leak A	ttach Three Photos:	
 Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdown area. 	1. 2. 3.	Straight Close-u	down over o of leak an	leak or damage.	
 All valves (give valve numbers and show which were closed during repair). Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. 					
Leak Found? YES (Yes/No)					
TYPE OF LEAK		/	-	•	
Meter Leak Main Line Leak	V	/ . Ji	oint Leak		
Meter Spud Leak Service Lateral L	eak	c	ther Leak	<u> </u>	
Meter Yoke Leak Fire Hydrant Lea	k	_	Describe		
Curb Stop Leak Valve Leak		~~~		7.14	
DESCRIPTION OF REPAIR					
Damaged part was: Repaired Repla	iced If r	eplaced, wh	at material	was used? <u>C 900</u>	
If repaired, what repairs were made?	Re	pair Time	7 H	(From/To)	
Leak Clamp Repacked Valve		,		(persons)	
Welded Recaulked Joint			ed for Repai		
Other (describe)		✓ Backh			
		Dumpi		y	
			= +-*		
Repair Costs:		of Leak:			
Materials \$				gpm	
_abor \$				gpm	
Equipment \$	Met	hod Used: _	VISUAL	<u>- </u>	
Other \$					



Agency: Waisville	
W.O. No.:	Foreman: D GANDWEL
LEAK IDENTIFICATION	Map Reference: N BROOKLYN NERT TO GEN ROOFIN
Refer to Leak Discovery Report	Page and Coordinates:
Discovery Date: 9/17/13	Leak No.:
Location (include street name and nu	mber):
FOR MAIN AND SERVICE LATERAL	
Sketch a map of the site including:	If Main or Service Leak, Attach Three Photos:
Street name; north arrow. Meter number (if applicable). Mains and hydrants in shutdow	Straight down over leak or damage. Close-up of leak and damage. Any other photo which you feel will
area. 4. All valves (give valve numbers	help.
show which were closed during	g repair).
or house with address.	
Show distances to property line or street centerlines.	es
Leak Found?	(Yes/No)
TYPE OF LEAK	
Meter Leak Mai	in Line Leak
Meter Spud Leak Ser	vice Lateral Leak Other Leak
Meter Yoke Leak Fire	Hydrant Leak Describe
Curb Stop Leak Val	ve Leak
DESCRIPTION OF REPAIR	
Darnaged part was: Repaired	Replaced If replaced, what material was used?
	110
If repaired, what repairs were made?	Repair Time 7 HL5 (From/To)
Leak Clamp Repacke	d Valve Crew Size (persons)
Welded Recaulke	ed Joint Equipment Used for Repair
Other (describe)	Backhoe
	Dumptruck
2	Size of Leak:
Repair Costs: Materials \$	10 80 to som
· ·	60-86 gpm
Labor \$	•
Equipment \$	•
Other \$	
Total \$	<u>.</u>

Culto 1 - 23"-

(8°) C1

Onart"

Car Contract Contract

REPAIRND WITH FOUR CARE. CLAMP

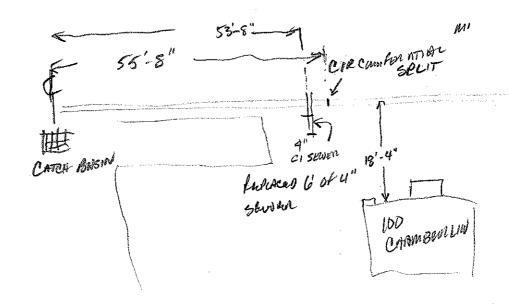
FOUND A STORM DRAIN (DAPAD OF)

8" DIPORCI IS ABOUT 5-6' DEEP

	K REPAIR REPORT
Agency: Walleville WATRO	Date 1/4/18
W.O. No.:	Foreman: D GANGWUN
LEAK IDENTIFICATION	Map Relevence: WALVES ZZL ZZ8
Refer to Leak Discovery/Report	Page and Coordinates:
Diamon Balan 1/1/1/13	Leak No.
Location (include street name and number): Aff	CRON 24 SOUTH OF FAMELIEW ON FAREIUM
FOR MAIN AND SERVICE LATERAL LEAKS ON	
	If Main or Service Leak, Attach Three Photos:
Sketch a map of the site including: 1. Street name; north arrow. 2. Meter number (if applicable). 3. Mains and hydrants in shutdown area. 4. All valves (give valve numbers and show which were closed during repair). 5. Locate leak to nearest intersection or house with address. Show distances to property lines or street centerlines. Leak Found? (Yes/No) TYPE OF LEAK Meter Leak Main Line Leak Meter Spud Leak Service Lateral L Meter Yoke Leak Fire Hydrant Lea Curb Stop Leak Valve Leak DESCRIPTION OF REPAIR Damaged part was: Repaired Replated	1. Straight down over leak or damage. 2. Close-up of leak and damage. 3. Any other photo which you feel will help. Joint Leak Leak
If repaired, what repairs were made? Leak Clamp Repacked Valve Welded Recaulked Joint Other (describe)	Repair Time 1-6fm (From/To) Crew Size 2-+1 (persons) Equipment Used for Repair
Materials \$	Measuredgpm
Labor \$	Estimatedgpm
Equipment \$	Method Used:
Other \$	
Univ	

FAIRVIEW LEAK APPLEX 24' South of Hank Q 226 TT USED FULL CARE WITH THE ONLY 228 THINK WE HAD USED CORP TO RUE OFF TAP. CORP IS FARANCE WEST (SIDE OF MAIN) 8" CI PIR IS ABOUT 5" TO TOPOR MANN J My W STERRARKITE D GARDWAR

WATER MAIN 64" PAVEET TO TOP OF READEN Shower APPROX 53'-8" FROM CENTER OF C.B.



Lyan ST16302 L maerary I Pay D GARAWAR APPENDIX N NYLD LEAK DETECTION SURVEY

NEW YORK LEAK DETECTION, INC.

Date: <u>04-11-2016</u>	te: <u>04-11-2016</u> Technician: <u>George Williams</u>			
Customer: HUNT Engineers, Architects & Land Surveyors, PC				
Site Address: Village of Wellsville	, 156 North Main Street, Wel	Isville, NY14895		
Contact Person: Brad Mattison (T	own of Wellsville) Ph	one: <u>585-610-8486</u> Phone	:	
Scope of Work: Leak Detection	Survey Village of Wellsville	on approximately 40 miles		
Type of Service:				
∠ Leak Detection	☐ Utility Location/GPR	☐ Video Inspection		
☐ Infrastructure Assessment	Utility Mapping/AutoCAL)		
Type of Equipment Used				
Profiler EMP 400	☐ <i>RD8000</i>	MetroTech Vivax vLocPro	2	
∠C2500 Leak Correlator	☐ Noggin 250 mHz	☐ PosiTector UTG G3		
⊠ S-30 Surveyor	☐ Noggin 500 mHz	☐ Video Inspection Ca	mera	
Sonde	☐ Conquest 1000 mHz	☐ Helium # Bot	tles	
Leica Robotic Total Station	☐ Leica GPS	☐ Traceable Rodder		
Marking Used				
□ Paint	☐ Flags	☐ Chalk		
☐ Updated existing maps onsite	Other:			
Instructions from Onsite Conta	ct: ACP, Plastic, Cast Iron a	nd Ductile pipe contained through	out system.	
Size of Pipe: Varying				
Ground Cover/Weather Conditi	ons: 35-55 degrees, heavy r	ain at times.		
Information Transfer				
	Brad Hand drawn to office for digital	map (forward	•	



NEW YORK LEAK DETECTION, INC.

Key

Blue	Water
Red	Power
Orange	Communications
Yellow	Gas/Flammable Fuel
White	Unknown
Green	Storm/Sanitary

<u>Notes/Testing Results:</u> Leak tested all hydrants throughout entire system using S-30 Surveyor as well as testing valves and curb boxes as needed. Correlations combined with surface acoustic testing utilized for leak locations. Continuity performed on areas containing asbestos concrete pipe. In areas where there were plastic mains scanned all accessible services for leak signal. Additionally found hydrants that have been moved or are not shown on provided mapping. Please see reports below for more information.

Areas Scanned with Plastic Mains:

- 1) Howard Street: Scanned all accessible services
- 2) School Street (C900): Scanned all accessible services
- 3) Sunset Avenue and Lee Place: Scanned all accessible services
- 4) Johnson Street: Scanned all available services
- 5) Oak Street: Scanned all accessible services
- 6) Main under creek near State highway 417 West and West State Street

In all cases no leak signal detected

Areas Scanned with Asbestos Concrete Mains:

- 1) Witter Avenue: scanned accessible services, continuity good
- 2) McDowell Avenue: Scanned accessible services, continuity good, note service to #11 McDowell is bent and inoperable.
- 3) Fairview Avenue: Continuity good
- 4) Rauber Street: Continuity good
- 5) West Hanover Street: Continuity good, this is an unknown however tone sounds similar to cast iron pipe.
- 6) Meadowbrook Court: Continuity good
- 7) Main which runs from Coats Street through the golf course and terminates at the municipal boundary on Riverside Drive: Continuity good
- 8) Main from the nursing home to Bolivar Road (417): Continuity good
- 9) North Highland Avenue to Seneca Street: Continuity good
- 10) West Dyke Street through the baseball field to Alfred State College: Continuity good

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-12-2016 Leak/Work Order: # 1

Leak Priority Classification: 1

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 25 Hamilton Street, Village of Wellsville

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

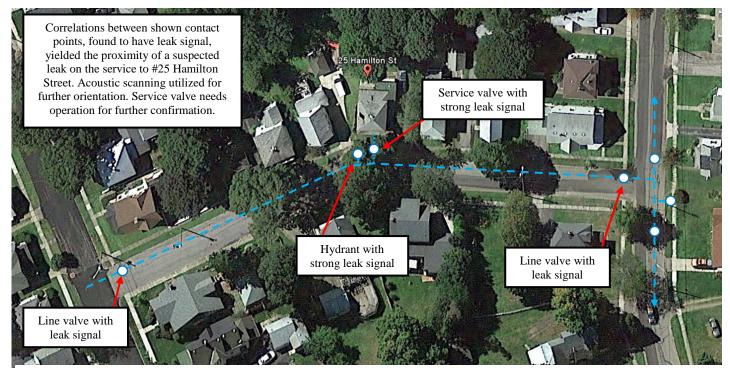
15,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	
Asphalt	X
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-12-2016

Leak/Work Order: #2

Leak Priority Classification: 2

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 252 Farnum Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

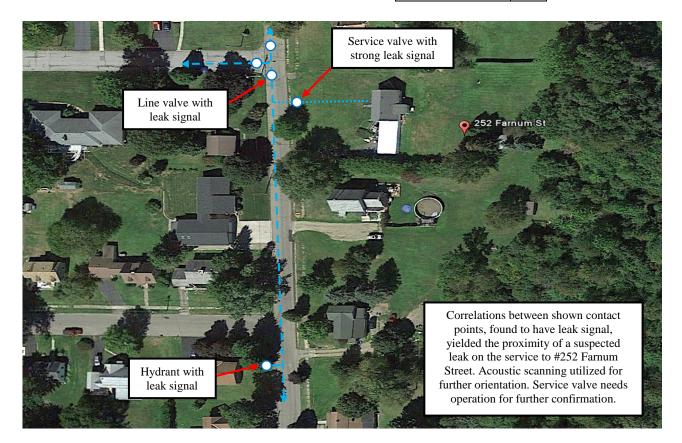
7,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	-
Asphalt	X
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Technician: George Williams

Date: 04-13-2016

Leak/Work Order: # 5
Leak Priority Classification: 1

Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: South Main Street, Wellsville NY, near car dealership

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

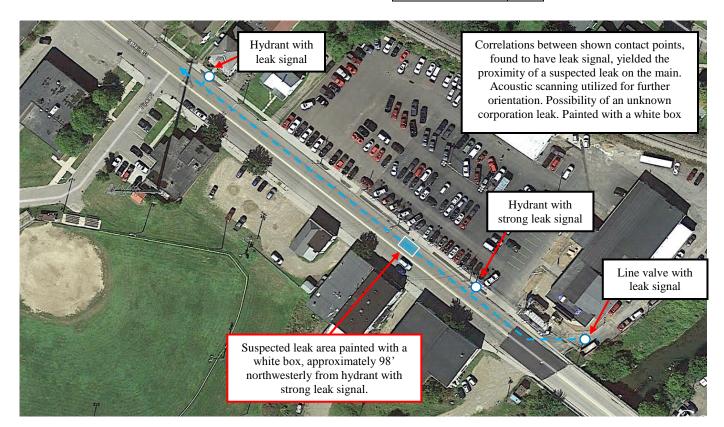
40,000

Main Valve	X
Curb Valve	
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	X
Service	
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	
Asphalt	X
Brick	
Gravel	
Soil	
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-13-2016 Leak/Work Order: # 6

Leak Priority Classification: 1

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 540 North Main Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

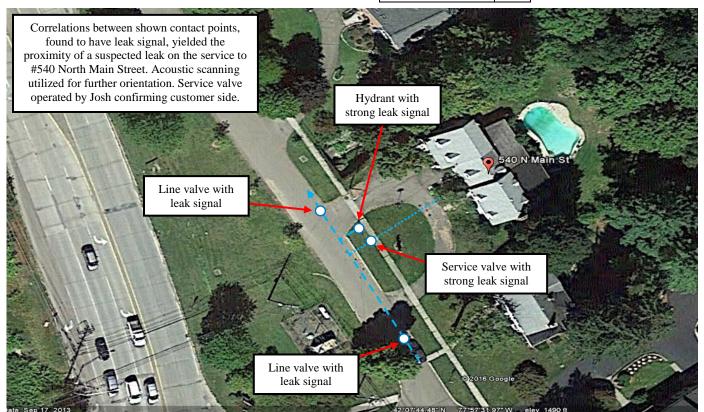
12,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	-
Asphalt	
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-13-2016

Leak/Work Order: #7

Leak Priority Classification: 2

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 47 Chestnut Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

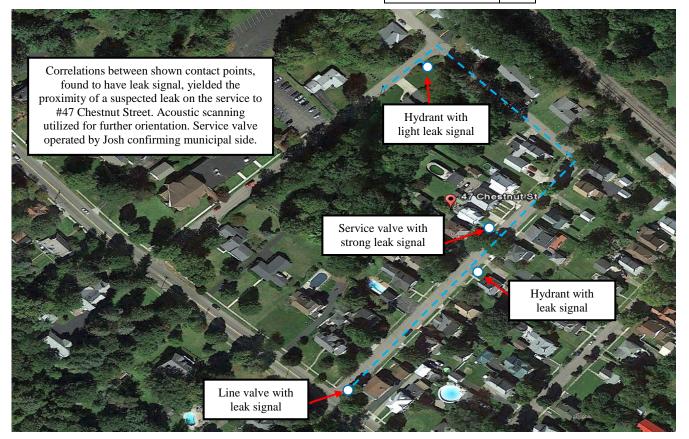
8,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	_
Asphalt	
Brick	
Gravel	
Soil	
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 08-13-2016

Leak/Work Order: # 8

Leak Priority Classification: 1

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 60 Maple Avenue, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

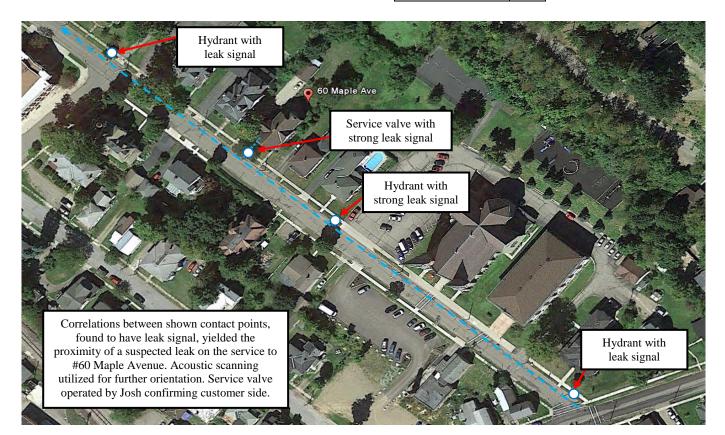
20.000

Main Valve	
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	
Asphalt	
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 08-14-2016

Leak/Work Order: #9

Leak Priority Classification: 2

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 47 Elm Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

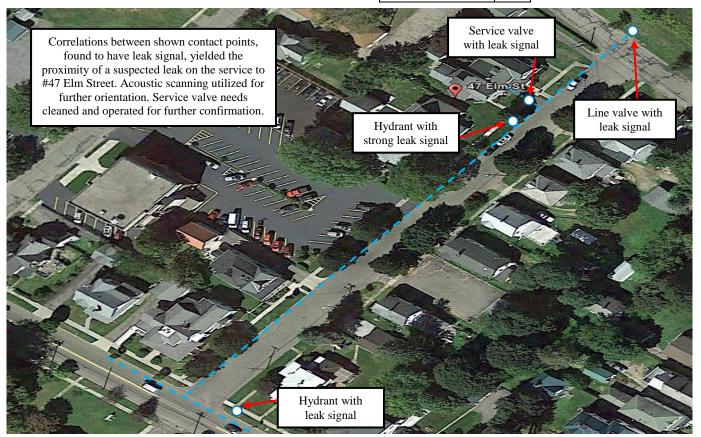
6,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	_
Asphalt	X
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 08-14-2016

Leak/Work Order: #10

Leak Priority Classification: 1

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 313 N. Main Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

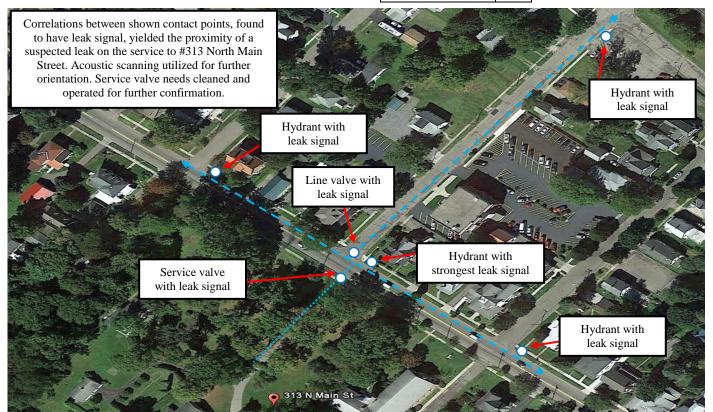
10,000

Main Valve	X
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	-
Asphalt	X
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-14-2016

Leak/Work Order: #11

Leak Priority Classification: 1

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 38 Rauber Street, Wellsville NY

Sonic	X
Surfaced Water	X
Other	

Estimation of Leak (GPD)

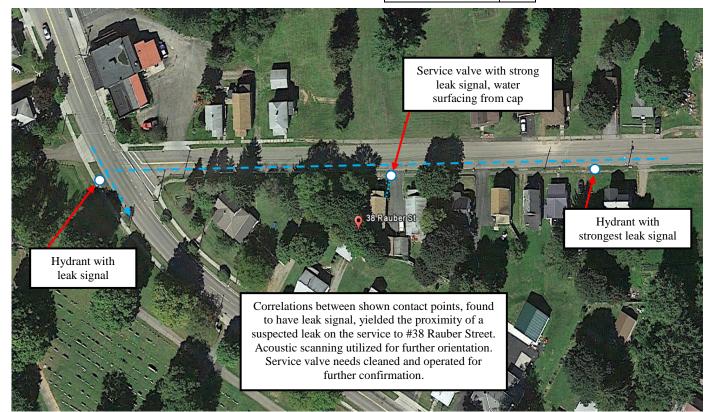
20,000

Main Valve	
Curb Valve	X
Meter Box	
Hydrant	X
Other	

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	
Asphalt	X
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

NEW YORK LEAK DETECTION, INC.

NYLD

New York Leak Detection, Inc.

LEAKAGE CONTROL REPORT

(Drawings Not To Scale)

Date: 04-15-2016

Leak/Work Order: # 12

Leak Priority Classification: 2

Technician: George Williams Client: HUNT Engineers, Architects & Land Surveyors, PC

Street Address: 131 Miller Street, Wellsville NY

Sonic	X
Surfaced Water	
Other	

Estimation of Leak (GPD)

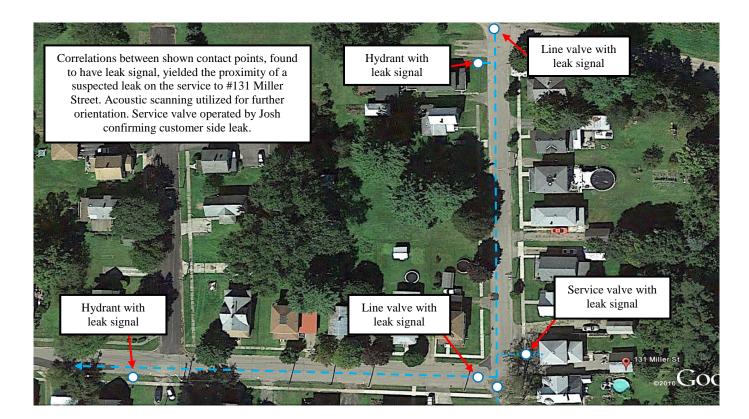
8,000

Main Valve	X			
Curb Valve	X			
Meter Box				
Hydrant	X			
Other				

Leak Appears To Be On

Main	
Service	X
Joint Connection	
Hydrant	
Valve	
Other	

Concrete	
Asphalt	
Brick	
Gravel	
Soil	X
Other	



Recognizing that underground leak detection and utilities locating is an art as well as a science, and that there are innumerable variables in achieving the desired results, NYLD does not guarantee accuracy in locating underground leaks or utilities and disclaims all liability for any damages based on information provided by NYLD.

APPENDIX O WATER METER TESTING REPORTS

Cold Spring Environmental

3248 Buffalo Rd., Varysburg, N.Y. 14167

Ph: 716-863-7052

May 17, 2016

Hunt Engineers Architects Surveyors Mr. Tim Steed
100 Hunt Center
Horseheads, NY 14845-1019

Ref: Wellsville Water Plant Flow Meter Report

Dear Mr. Steed,

Below are the meter description and results.

Calibration Date: May 13, 2016 Site location: Treated Water Flow meter Equipment Model: Water Specialties 7-TR15-S Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 20083093

Measuring device: 12 inch Pipe Output type: 4-20 mA: 0-3000 GPM

Totalizer multiplier: X1000, 1 revolution = 1000 gallons

Displayed level/flow rate: 1130-1140 GPM Measured Level/flow rate: 1120-1130 GPM

Percent Difference: ≤ 1%

Adjustment: No

Calibration Date: May 13, 2016

Site location: Clearwell Inlet Flow meter Equipment Model: Water Specialties TR15

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 20081718-12 Measuring device: 12 inch Pipe Output type: 4-20 mA: 0-2250 GPM

Totalizer multiplier: X1000, 1 revolution = 1000 gallons

Displayed level/flow rate: 1400-1410 GPM Measured Level/flow rate: 1390-1400 GPM

Percent Difference: ≤ 1%

Adjustment: No

Calibration Date: May 13, 2016

Site location: Treated Water Chart Recorder

Equipment Model: Honeywell DR45AT Equipment type: 7 day round chart Equipment S/N: C4000000466114

Measuring device: output from flow meter

Maximum flow rate: 2250 GPM

Input type: 4-20 mA

Totalizer multiplier: 100 gallons

Displayed level/flow rate Input: 1345 GPM Measured Level/flow rate Input: 1345 GPM

Percent Difference: 0%

Adjustment: No

Calibration Date: May 13, 2016

Site location: Clearwell Inlet Chart Recorder

Equipment Model: Honeywell DR45AT Equipment type: 7 day round chart Equipment S/N: C4000000466113

Measuring device: output from flow meter

Maximum flow rate: 2250 GPM

Input type: 4-20 mA

Totalizer multiplier: 100 gallons

Displayed level/flow rate Input: 1419 GPM Measured Level/flow rate Input: 1419 GPM

Percent Difference: 0%

Adjustment: No

Please contact me with any questions.

Sincerely, Jon Wolak

Jon Wolak

jonwolak@yahoo.com

Cold Spring Environmental

3248 Buffalo Rd., Varysburg, N.Y. 14167 Ph: 716-863-7052

June 16, 2016

Hunt Engineers Architects Surveyors Mr. Tim Steed 100 Hunt Center Horseheads, NY 14845-1019

Ref: Wellsville Flow Meter Report

Dear Mr. Steed,

Because of the varying flows we measured the totalizers over a 15 minute period, below are the meter description and results. The portable meter used was purchased new one month ago.

Verification Date: June 15, 2016

Site location: High School Flow meter, (across from water plant)

Equipment Model: Sensus 4"-6"

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 61294023

Measuring device: 4 inch copper pipe

Output type: none

Totalizer multiplier: X1, 1 revolution = 1 CF

Measured Total over 15 minutes (above meter): 28 CF Measured Total over 15 minutes (portable meter): 28 CF

Percent Difference: ≤ 1%

Verification Date: June 15, 2016

Site location: Elementary School Flow meter

Equipment Model: Sensus 4"-6"

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: No S/N

Measuring device: 4 inch copper pipe

Output type: none

Totalizer multiplier: X1, 1 revolution = 1 CF

Measured Total over 15 minutes (above meter): 44 CF Measured Total over 15 minutes (portable meter): 45 CF

Percent Difference: ≤ 1%

Verification Date: June 15, 2016

Site location: Argentari Laundry Flow meter

Equipment Model: Rockwell 4"

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 1313296

Measuring device: 4 inch galvanized pipe

Output type: none

Totalizer multiplier: X100, 1 revolution = 100 CF Measured Total over 15 minutes (above meter): 2 CF Measured Total over 15 minutes (portable meter): 45 CF

Percent Difference: greater than 100%

Note: no movement for a long time, less than one revolution

Verification Date: June 15, 2016 Site location: Hospital Flow meter

Equipment Model: Rockwell 4"

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 1313296

Measuring device: 6 inch copper pipe

Output type: none

Totalizer multiplier: X100, 1 revolution = 100 CF Measured Total over 15 minutes (above meter): 35 CF Measured Total over 15 minutes (portable meter): 135 CF

Percent Difference: greater than 100%

Note: no movement when the flow was below 2.7 CFM, The second meter in the Hospital was shut off, all valves around it were in the off position, only used for a by-pass for the above meter during repairs so we did not check this one

Verification Date: June 15, 2016

Site location: Manor Care Center Flow meter, (next to old runway)

Equipment Model: Spectrun 260, 2" meter

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 01-3045-17FT3

Measuring device: 4 inch copper pipe

Output type: none

Totalizer multiplier: X100, 1 revolution = 1 CF Measured Total over 15 minutes (above meter): 34 CF Measured Total over 15 minutes (portable meter): 34 CF

Percent Difference: ≤ 1%

Verification Date: June 15, 2016

Site location: Manor Care Center Flow meter, (behind WWTP)

Equipment Model: Rockwell 2"-3"

Equipment type: Closed Pipe Turbine Flow Meter

Equipment S/N: 1294518

Measuring device: 4 inch copper pipe

Output type: none

Totalizer multiplier: X10, 1 revolution = 10 CF
Measured Total over 15 minutes (above meter): 15 CF
Measured Total over 15 minutes (portable meter): 15 CF

Percent Difference: ≤ 1%

Please contact me with any questions.

Sincerely, Jon Wolak

Jon Wolak

jonwolak@yahoo.com

APPENDIX P COST ESTIMATES

Opinion of Probable Cost Water Meter Replacement



Village of Wellsville		Date: 1/30/2017			HUNT Project Number: 1861.034				
	9		Design Stage: PER			Calculated By: AEV			
Wellsville, NY		Revision: 1			Checked By: TKS				
Item No.	tana Na		Unit		1	COSTS		Subtotals	
item No.	. Item	Quantity	Offic		\$/Unit	Total		Sublotais	
	DIV 1 - GENERAL CONDITIONS							\$	2,328.24
1	Bonding (2.5%)	1	LS	\$	1,455.15	\$	1,455.15		
2	Supervision (0.5%)	1	LS	\$	291.03	\$	291.03		
3	Mobilization (1%)	1	LS	\$	582.06	\$	582.06		
	WATER METER REPLACEMENT							\$	58,206.00
4	Commercial Water Meter 2" Ultrasonic	2	EA	\$	1,716.00	\$	3,432.00		
5	Commercial Water Meter 3" Badger CSM	1	EA	\$	3,954.00	\$	3,954.00		
6	Commercial Water Meter 4" Badger FS with bypass	2	EA	\$	9,600.00	\$	19,200.00		
6	Commercial Water Meter 4" Badger Install Only	1	EA	\$	1,440.00	\$	1,440.00		
7	Commercial Water Meter 6" Badger FS with bypass	2	EA	\$	15,090.00	\$	30,180.00		
	Construction Contingency (20%)	1	LS	\$	12,106.85	\$	12,106.85	\$	12,106.85
	TOTAL CONSTRUCTION COST				\$	72,641.09			

Opinion of Probable Cost Treatment Plan Pump Replacement and Control Improvements



Village of Wellsville		Date: 1/5/2017					HUNT Project Number: 1861.034				
		Design Stage: PER				Calculated By: AEV					
	Wellsville, NY					Checked By: TKS					
Item No.	. Item	Quantity U	Unit		(COSTS			Subtotals		
item No.			Offic		\$/Unit		Total		Oublotais		
	DIV 1 - GENERAL CONDITIONS							\$	16,492.00		
1	Bonding (2.5%)	1	LS	\$	5,890.00	\$	5,890.00				
2	Supervision (2%)	1	LS	\$	4,712.00	\$	4,712.00				
3	Mobilization (2%)	1	LS	\$	4,712.00	\$	4,712.00				
4	Temporary Facilities (0.5%)	1	LS	\$	1,178.00	\$	1,178.00				
	WATER TREATMENT PUMP AND CONTROLS							\$	235,600.00		
5	25HP Raw Water Submersible Pump	2	EA	\$	23,000.00	\$	46,000.00				
6	15HP Raw Water	2	EΑ	\$	21,800.00	\$	43,600.00				
7	25HP VFD	2	EΑ	\$	9,050.00	\$	18,100.00				
8	15HP VFD	2	EΑ	\$	8,000.00	\$	16,000.00				
9	60HP VFD	1	EΑ	\$	13,300.00	\$	13,300.00				
10	125HP VFD	2	EΑ	\$	20,300.00	\$	40,600.00				
11	Custom Pump Control Panel for High Service	1	EA	\$	48,000.00	\$	48,000.00				
12	Removal of Existing Equipment and Relocations	1	LS	\$	10,000.00	\$	10,000.00				
	Construction Contingency (20%)	1	LS	\$	50,418.40	\$	50,418.40	\$	50,418.40		
	TOTAL CONSTRUCTION COST							\$	302,510.40		

Opinion of Probable Cost Water Replacement Madison Hill Rd.



Village of Wellsville		Date:	1/5/2	2017	7	HUNT Project Number: 1861.034				
9		Design Stage:	PER			Calculated By: AEV				
Wellsville, NY		Revision:					Checked By: TKS			
Item No.	Item	Quantity	Unit			COSTS			Subtotals	
itom ito.		Quantity	Onne		\$/Unit		Total			
	DIV 1 - GENERAL CONDITIONS							\$	50,484.60	
	Bonding (2.5%)	1		\$	13,285.42	\$	13,285.42			
2	Supervision (2%)	1		\$	10,628.34	\$	10,628.34			
	Mobilization (3%)	1		\$	15,942.51	\$	15,942.51			
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	7,971.25	\$	7,971.25			
5	Temporary Facilities (0.5%)	1	LS	\$	2,657.08	\$	2,657.08			
	EROSION AND SEDIMENT CONTROL							\$	2,000.00	
6	Inlet Protection	16	EA	\$	125.00	\$	2,000.00			
	EARTHWORK							\$	12,285.19	
7	Import and Place Topsoil	230	CY	\$	40.00	\$	9,185.19			
8	Fine Grade Topsoil	12400	SF	\$	0.25	\$	3,100.00			
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	181,914.67	
9	Heavy Duty Asphalt Pavement with Subbase	2521	SY	\$	52.00	\$	131,074.67			
10	Concrete Sidewalk with Subbase	2480	SF	\$	10.00	\$	24,800.00			
11	Granite Curb	620	LF	\$	38.00	\$	23,560.00			
12	Seeding & Mulching	12400	SF	\$	0.20	\$	2,480.00			
	WATER DISTRIBUTION							\$	335,217.00	
13	Connection to Existing System	7	EA	\$	2,000.00	\$	14,000.00			
14	8" Ductile Iron Watermain	1171	LF	\$	62.00	\$	72,602.00			
15	14" Ductile Iron Watermain	1140	LF	\$	121.00	\$	137,940.00			
17	8" Gate Valve and Box	2		\$	2,000.00	\$	4,000.00			
18	8" Elbow/Tee	5	EA	\$	695.00	\$	3,475.00			
19	14" Elbow/Tee	8	EA	\$	2,450.00	\$	19,600.00			
20	Fire Hydrant Unit	7	EA	\$	5,000.00	\$	35,000.00			
21	1-1/2" Water Service Direct Bury	1440	LF	\$	20.00	\$	28,800.00			
22	Coupler to Tie onto Existing Water Service	55	EA	\$	50.00	\$	2,750.00			
23	Curb Stop and Box	55		\$	310.00	\$	17,050.00			
	Construction Contingency (20%)	1	LS	\$	116,380.29	\$	116,380.29	\$	116,380.29	
	TOTAL CONSTRUCTION COST							\$	698,281.74	

Opinion of Probable Cost Water Replacement Brooklyn Ave.

HUNT

Village of Wellsville		Date: 1/5/2017					HUNT Project Number: 1861.034			
		Design Stage: PER					Calculated By: AEV			
Wellsville, NY		Revision:				Checked By: TKS				
				1		COST		1110		
Item No.	Item	Quantity	Unit		\$/Unit		Total	Subtotals		
	DIV 1 - GENERAL CONDITIONS							\$	57,354.22	
1	Bonding (2.5%)	1	LS	\$	15,093.21	\$	15,093.21			
2	Supervision (2%)	1	LS	\$	12,074.57	\$	12,074.57			
3	Mobilization (3%)	1	LS	\$	18,111.86	\$	18,111.86			
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	9,055.93	\$	9,055.93			
5	Temporary Facilities (0.5%)	1	LS	\$	3,018.64	\$	3,018.64			
	EROSION AND SEDIMENT CONTROL							\$	2,000.00	
6	Inlet Protection	16	EA	\$	125.00	\$	2,000.00			
	EARTHWORK							\$	15,059.26	
7	Import and Place Topsoil	281	CY	\$	40.00	\$	11,259.26			
8	Fine Grade Topsoil	15200	SF	\$	0.25	\$	3,800.00			
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	232,949.33	
9	Heavy Duty Asphalt Pavement with Subbase	3281	SY	\$	52.00	\$	170,629.33			
10	Concrete Sidewalk with Subbase	3040	SF	\$	10.00	\$	30,400.00			
11	Granite Curb	760	LF	\$	38.00	\$	28,880.00			
12	Seeding & Mulching	15200	SF	\$	0.20	\$	3,040.00			
	WATER DISTRIBUTION							\$	353,720.00	
13	Connection to Existing System	4	EA	\$	2,000.00	\$	8,000.00			
15	8" Ductile Iron Watermain	3632	LF	\$	65.00	\$	236,080.00			
17	8" Gate Valve and Box	4	EA	\$	2,000.00	\$	8,000.00			
18	8" Elbow/Tee	12	EA	\$	695.00	\$	8,340.00			
19	Fire Hydrant Unit	6	EA	\$	5,000.00	\$	30,000.00			
20	1-1/2" Water Service Direct Bury	1905	LF	\$	20.00	\$	38,100.00			
21	Coupler to Tie onto Existing Water Service	70		\$	50.00	\$	3,500.00			
22	Curb Stop and Box	70		\$	310.00	\$	21,700.00			
	Construction Contingency (20%)	1	LS	\$	132,216.56	\$	132,216.56	\$	132,216.56	
	TOTAL CONSTRUCTION COST							\$	793,299.37	

Opinion of Probable Cost Water Replacement Early St.

HUNT

			4 /5 /6	20.4	_		IT D : (N)	1001	20.4		
Village of Wellsville Wellsville, NY		Date: 1/5/2017					HUNT Project Number: 1861.034				
		Design Stage: PER					Calculated By: AEV				
		Revision:					Checked By:	IKS			
Item No.	Item	Quantity	Unit			COSTS		Subtotals			
		,			\$/Unit		Total		A 07 0 5 1 1		
	DIV 1 - GENERAL CONDITIONS			_				\$	37,637.11		
	Bonding (2.5%)	1	LS	\$	-	\$	9,904.50				
2	Supervision (2%)	1	LS	\$	7,923.60	\$	7,923.60				
	Mobilization (3%)	1	LS	\$	11,885.40	\$	11,885.40				
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	5,942.70	\$	5,942.70				
5	Temporary Facilities (0.5%)	1	LS	\$	1,980.90	\$	1,980.90				
	EROSION AND SEDIMENT CONTROL							\$	1,000.00		
6	Inlet Protection	8	EA	\$	125.00	\$	1,000.00				
	EARTHWORK							\$	13,474.07		
7	Import and Place Topsoil	252	CY	\$	40.00	\$	10,074.07				
8	Fine Grade Topsoil	13600	SF	\$	0.25	\$	3,400.00				
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	158,096.00		
9	Heavy Duty Asphalt Pavement with Subbase	1968	SY	\$	52.00	\$	102,336.00				
10	Concrete Sidewalk with Subbase	2720	SF	\$	10.00	\$	27,200.00				
11	Granite Curb	680	LF	\$	38.00	\$	25,840.00				
12	Seeding & Mulching	13600	SF	\$	0.20	\$	2,720.00				
	WATER DISTRIBUTION							\$	223,610.00		
13	Connection to Existing System	3	EA	\$	2,000.00	\$	6,000.00				
14	8" Ductile Iron Watermain	1992	LF	\$	65.00	\$	129,480.00				
15	8" Gate Valve and Box	2	EA	\$	2,000.00	\$	4,000.00				
16	8" Elbow/Tee	6	EA	\$	695.00	\$	4,170.00				
17	Fire Hydrant Unit	5	EA	\$	5,000.00	\$	25,000.00				
18	1-1/2" Water Service Direct Bury	1614	LF	\$	20.00	\$	32,280.00				
19	Coupler to Tie onto Existing Water Service	63	EA	\$	50.00	\$	3,150.00				
20	Curb Stop and Box	63	EA	\$	310.00	\$	19,530.00				
	Construction Contingency (20%)	1	LS	\$	86,763.44	\$	86,763.44	\$	86,763.44		
	TOTAL CONSTRUCTION COST							\$	520,580.62		

Opinion of Probable Cost Water Replacement S. Main St.



	Village of Wellsville	Date:	1/5/2	2017	7	1UH	NT Project Number:	1861.	.034
	<u> </u>	Design Stage:		•			Calculated By:		
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			cos			Subtotals
itom ito.		Quartity	Oint		\$/Unit		Total		
	DIV 1 - GENERAL CONDITIONS							\$	66,383.59
1	Bonding (2.5%)	1	LS	\$	12,293.26	\$	12,293.26		
2	Supervision (2%)	1	LS	\$	9,834.61	\$	9,834.61		
3	Mobilization (3%)	1	LS	\$	14,751.91	\$	14,751.91		
4	Maintenance and Protection of Traffic (5%)	1	LS	\$	24,586.51	\$	24,586.51		
5	Temporary Facilities (1%)	1	LS	\$	4,917.30	\$	4,917.30		
	EROSION AND SEDIMENT CONTROL							\$	2,350.00
6	Silt Fence	200	LF	\$	3.00	\$	600.00		
7	Inlet Protection	14	EA	\$	125.00	\$	1,750.00		
	EARTHWORK							\$	3,962.96
8	Import and Place Topsoil	74	CY	\$	40.00	\$	2,962.96		·
9	Fine Grade Topsoil	4000	SF	\$	0.25	\$	1,000.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	161,289.33
10	S-Duty Asphalt Pavement with Subbase	333	SY	\$	45.00	\$	15,000.00		
11	Heavy Duty Asphalt Pavement with Subbase	2061	SY	\$	52.00	\$	107,189.33		
12	Concrete Sidewalk with Subbase	2500	SF	\$	10.00	\$	25,000.00		
13	Granite Curb	350	LF	\$	38.00	\$	13,300.00		
14	Seeding & Mulching	4000	SF	\$	0.20	\$	800.00		
	WATER DISTRIBUTION							\$	324,128.00
15	Connection to Existing System	8	EA	\$	2,000.00	\$	16,000.00		
16	4" Ductile Iron Service	250	LF	\$	50.00	\$	12,500.00		
17	10" Ductile Iron Watermain	2392	LF	\$	84.00	\$	200,928.00		
18	4" Gate Valve and Box	10	EA	\$	1,200.00	\$	12,000.00		
20	10" Gate Valve and Box	2	EA	\$	4,500.00	\$	9,000.00		
21	10" Elbow/Tee	10	EA	\$	1,700.00	\$	17,000.00		
22	Fire Hydrant Unit	6	EA	\$	5,000.00	\$	30,000.00		
23	1-1/2" Water Service Direct Bury	705	LF	\$	20.00	\$	14,100.00		
24	Coupler to Tie onto Existing Water Service	35	EA	\$	50.00	\$	1,750.00		
25	Curb Stop and Box	35	EA	\$	310.00	\$	10,850.00		
	Construction Contingency (20%)	1	LS	\$	111,622.78	\$	111,622.78	\$	111,622.78
	TOTAL CONSTRUCTION COST							\$	669,736.66

Opinion of Probable Cost Water Replacement State St.

	Village of Wellsville	Date:	1/5/2	2017	7	HUN	NT Project Number:	1861	.034
	<u> </u>	Design Stage:	PER				Calculated By:		
	Wellsville, NY	Revision:	1				Checked By:	TKS	
Item No.	Item	Quantity	Unit			cos			Subtotals
		~~y	0		\$/Unit		Total		
	DIV 1 - GENERAL CONDITIONS							\$	64,442.42
1	Bonding (2.5%)	1		\$	12,392.77	\$	12,392.77		
2	Supervision (2%)	1	LS	\$	9,914.22	\$	9,914.22		
3	Mobilization (3%)	1	LS	\$	14,871.33	\$	14,871.33		
4	Maintenance and Protection of Traffic (5%)	1	LS	\$	24,785.55	\$	24,785.55		
5	Temporary Facilities (0.5%)	1	LS	\$	2,478.55	\$	2,478.55		
	EROSION AND SEDIMENT CONTROL							\$	1,500.00
6	Inlet Protection	12	EA	\$	125.00	\$	1,500.00		
	EARTHWORK							\$	10,898.15
7	Import and Place Topsoil	204	CY	\$	40.00	\$	8,148.15		
8	Fine Grade Topsoil	11000	SF	\$	0.25	\$	2,750.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	225,037.78
9	Heavy Duty Asphalt Pavement with Subbase	3391	SY	\$	52.00	\$	176,337.78		
10	Concrete Sidewalk with Subbase	2750	SF	\$	10.00	\$	27,500.00		
11	Granite Curb	500	LF	\$	38.00	\$	19,000.00		
12	Seeding & Mulching	11000	SF	\$	0.20	\$	2,200.00		
	WATER DISTRIBUTION							\$	258,275.00
13	Connection to Existing System	3	EA	\$	2,000.00	\$	6,000.00		
14	4"Ductile Iron Service	60	LF	\$	50.00	\$	3,000.00		
15	8" Ductile Iron Watermain	2611	LF	\$	65.00	\$	169,715.00		
16	4" Gate Valve and Box	1	EA	\$	1,200.00	\$	1,200.00		
17	8" Gate Valve and Box	2	EA	\$	2,000.00	\$	4,000.00		
18	8" Elbow/Tee	8	EA	\$	695.00	\$	5,560.00		
19	Fire Hydrant Unit	5	EA	\$	5,000.00	\$	25,000.00		
20	1-1/2" Water Service Direct Bury	1290	LF	\$	20.00	\$	25,800.00		
21	Coupler to Tie onto Existing Water Service	50	EA	\$	50.00	\$	2,500.00		
22	Curb Stop and Box	50		\$	310.00	\$	15,500.00		
	Construction Contingency (20%)	1	LS	\$	112,030.67	\$	112,030.67	\$	112,030.67
	TOTAL CONSTRUCTION COST							\$	672,184.02

Opinion of Probable Cost Water Replacement State St. East of Brooklyn



	East of Brooklyff								
	Village of Wellsville	Date:	1/5/2	2017	7	HUN	T Project Number:	1861.	.034
	•	Design Stage:	PER				Calculated By:		
	Wellsville, NY	Revision:	1				Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST			Subtotals
1.0111 110.		Quartity	Oim		\$/Unit		Total		
	DIV 1 - GENERAL CONDITIONS							\$	23,756.64
1	Bonding (2.5%)	1		\$	3,959.44	\$	3,959.44		
2	Supervision (2%)	1	LS	\$	3,167.55	\$	3,167.55		
3	Mobilization (3%)	1	LS	\$	4,751.33	\$	4,751.33		
4	Maintenance and Protection of Traffic (7%)	1	LS	\$	11,086.43	\$	11,086.43		
5	Temporary Facilities (0.5%)	1	LS	\$	791.89	\$	791.89		
	EROSION AND SEDIMENT CONTROL							\$	500.00
6	Inlet Protection	4	EA	\$	125.00	\$	500.00		
	EARTHWORK							\$	1,981.48
7	Import and Place Topsoil	37	CY	\$	40.00	\$	1,481.48		
8	Fine Grade Topsoil	2000	SF	\$	0.25	\$	500.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	68,711.11
9	Heavy Duty Asphalt Pavement with Subbase	1144	SY	\$	52.00	\$	59,511.11		
10	Concrete Sidewalk with Subbase	500	SF	\$	10.00	\$	5,000.00		
11	Granite Curb	100	LF	\$	38.00	\$	3,800.00		
12	Seeding & Mulching	2000	SF	\$	0.20	\$	400.00		
	WATER DISTRIBUTION							\$	87,185.00
13	Connection to Existing System	3	EA	\$	2,000.00	\$	6,000.00		
14	4"Ductile Iron Service	60	LF	\$	50.00	\$	3,000.00		
15	8" Ductile Iron Watermain	880	LF	\$	65.00	\$	57,200.00		
16	4" Gate Valve and Box	1	EA	\$	1,200.00	\$	1,200.00		
17	8" Gate Valve and Box	2	EA	\$	2,000.00	\$	4,000.00		
18	8" Elbow/Tee	3	EΑ	\$	695.00	\$	2,085.00		
19	Fire Hydrant Unit	1	EΑ	\$	5,000.00	\$	5,000.00		
20	1-1/2" Water Service Direct Bury	255	LF	\$	20.00	\$	5,100.00		
21	Coupler to Tie onto Existing Water Service	10	EA	\$	50.00	\$	500.00		
22	Curb Stop and Box	10	EA	\$	310.00	\$	3,100.00		
	Construction Contingency (20%)	1	LS	\$	36,426.85	\$	36,426.85	\$	36,426.85
	TOTAL CONSTRUCTION COST							\$	218,561.08

Opinion of Probable Cost Water Replacement Stevens St.

	Village of Wellsville	Date:		-	7	HUN	IT Project Number:	1861	.034
	Wellsville, NY	Design Stage:					Calculated By:		
	wellsville, iv t	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST			Subtotals
		Quantity	0		\$/Unit		Total		
	DIV 1 - GENERAL CONDITIONS							\$	41,672.50
1	Bonding (2.5%)	1	LS	\$	10,966.45	\$	10,966.45		
2	Supervision (2%)	1	LS	\$	8,773.16	\$	8,773.16		
3	Mobilization (3%)	1	LS	\$	13,159.74	\$	13,159.74		
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	6,579.87	\$	6,579.87		
5	Temporary Facilities (0.5%)	1	LS	\$	2,193.29	\$	2,193.29		
	EROSION AND SEDIMENT CONTROL							\$	1,250.00
6	Inlet Protection	10	EA	\$	125.00	\$	1,250.00		
	EARTHWORK							\$	13,672.22
7	Import and Place Topsoil	256	CY	\$	40.00	\$	10,222.22		
8	Fine Grade Topsoil	13800	SF	\$	0.25	\$	3,450.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	172,990.67
9	Heavy Duty Asphalt Pavement with Subbase	2239	SY	\$	52.00	\$	116,410.67		
10	Concrete Sidewalk with Subbase	2760	SF	\$	10.00	\$	27,600.00		
11	Granite Curb	690	LF	\$	38.00	\$	26,220.00		
12	Seeding & Mulching	13800	SF	\$	0.20	\$	2,760.00		
	WATER DISTRIBUTION							\$	250,745.00
13	Connection to Existing System	3	EΑ	\$	2,000.00	\$	6,000.00		
14	8" Ductile Iron Watermain	2308	LF	\$	65.00	\$	150,020.00		
15	8" Gate Valve and Box	2	EΑ	\$	2,000.00	\$	4,000.00		
16	8" Elbow/Tee	7	EA	\$	695.00	\$	4,865.00		
17	Fire Hydrant Unit	6	EA	\$	5,000.00	\$	30,000.00		
18	1-1/2" Water Service Direct Bury	1659	LF	\$	20.00	\$	33,180.00		
19	Coupler to Tie onto Existing Water Service	63	EA	\$	50.00	\$	3,150.00		
20	Curb Stop and Box	63	EA	\$	310.00	\$	19,530.00		
	Construction Contingency (20%)	1	LS	\$	96,066.08	\$	96,066.08	\$	96,066.08
	TOTAL CONSTRUCTION COST							\$	576,396.47
		T		_				•	

Opinion of Probable Cost Replace Asbestos Lines at Fairview and John St.

	Village of Wellsville	Date:	1/5/2	2017	7	HUN	T Project Number:	1861	.034
		Design Stage:	PER				Calculated By:	AEV	
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST	S		Subtotals
item No.	item	Quantity	Offic		\$/Unit		Total		Subiolais
	DIV 1 - GENERAL CONDITIONS							\$	16,288.08
1	Bonding (2.5%)	1	LS	\$	4,286.34	\$	4,286.34		
2	Supervision (2%)	1	LS	\$	3,429.07	\$	3,429.07		
3	Mobilization (3%)	1	LS	\$	5,143.60	\$	5,143.60		
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	2,571.80	\$	2,571.80		
5	Temporary Facilities (0.5%)	1	LS	\$	857.27	\$	857.27		
	EROSION AND SEDIMENT CONTROL							\$	750.00
6	Inlet Protection	6	EΑ	\$	125.00	\$	750.00		
	EARTHWORK							\$	4,161.11
7	Import and Place Topsoil	78	CY	\$	40.00	\$	3,111.11		
8	Fine Grade Topsoil	4200	SF	\$	0.25	\$	1,050.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	67,157.33
9	Heavy Duty Asphalt Pavement with Subbase	1275	SY	\$	52.00	\$	66,317.33		
10	Seeding & Mulching	4200	SF	\$	0.20	\$	840.00		
	WATER DISTRIBUTION							\$	99,385.00
11	Connection to Existing System	2	EΑ	\$	2,000.00	\$	4,000.00		
12	6" Ductile Iron Watermain	1283	LF	\$	50.00	\$	64,150.00		
13	8" Elbow/Tee (10% Increase Per Inch 6" = \$450)	5	EΑ	\$	695.00	\$	3,475.00		
14	Fire Hydrant Unit	2	EΑ	\$	5,000.00	\$	10,000.00		
15	1-1/2" Water Service Direct Bury	510	LF	\$	20.00	\$	10,200.00		
16	Coupler to Tie onto Existing Water Service	21	EA	\$	50.00	\$	1,050.00		
17	Curb Stop and Box	21	EA	\$	310.00	\$	6,510.00		
	Construction Contingency (20%)	1	LS	\$	37,548.30	\$	37,548.30	\$	37,548.30
	TOTAL CONSTRUCTION COST							\$	225,289.83

Opinion of Probable Cost Replace Asbestos Lines at King St.

	Village of Wellsville	Date:	1/5/2	2017		HUN	T Project Number:	1861	.034		
		Design Stage:	PER				Calculated By:	AEV			
	Wellsville, NY	Revision:					Checked By:	TKS			
Item No.	Itom	Quantity	Unit			COST	S		Subtotals		
item No.	ltem	Quantity	Unit		\$/Unit		Total		Subtotals		
	DIV 1 - GENERAL CONDITIONS							\$	6,979.22		
1	Bonding (2.5%)	1	LS	\$	1,836.64	\$	1,836.64				
2	Supervision (2%)	1	LS	\$	1,469.31	\$	1,469.31				
3	Mobilization (3%)	1	LS	\$	2,203.96	\$	2,203.96				
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	1,101.98	\$	1,101.98				
5	Temporary Facilities (0.5%)	1	LS	\$	367.33	\$	367.33				
	EROSION AND SEDIMENT CONTROL							\$	500.00		
6	Inlet Protection	4	EΑ	\$	125.00	\$	500.00				
	EARTHWORK							\$	1,981.48		
7	Import and Place Topsoil	37	CY	\$	40.00	\$	1,481.48				
8	Fine Grade Topsoil	2000	SF	\$	0.25	\$	500.00				
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	35,024.00		
9	Heavy Duty Asphalt Pavement with Subbase	482	SY	\$	52.00	\$	25,064.00				
10	Concrete Sidewalk with Subbase	500	SF	\$	10.00	\$	5,000.00				
11	Granite Curb	120	LF	\$	38.00	\$	4,560.00				
12	Seeding & Mulching	2000	SF	\$	0.20	\$	400.00				
	WATER DISTRIBUTION							\$	35,960.00		
13	Connection to Existing System	1	EA	\$	2,000.00	\$	2,000.00				
14	6" Ductile Iron Watermain	423	LF	\$	55.00	\$	23,265.00				
15	6" Gate Valve and Box	1	EΑ	\$	1,600.00	\$	1,600.00				
16	6" Elbow/Tee	1	EΑ	\$	695.00	\$	695.00				
17	1-1/2" Water Service Direct Bury	240	LF	\$	20.00	\$	4,800.00				
18	Coupler to Tie onto Existing Water Service	10	EA	\$	50.00	\$	500.00				
19	Curb Stop and Box	10	EA	\$	310.00	\$	3,100.00				
	Construction Contingency (20%)	1	LS	\$	16,088.94	\$	16,088.94	\$	16,088.94		
	TOTAL CONSTRUCTION COST							\$	96,533.64		

Opinion of Probable Cost Replace Asbestos Line at Trapping Brook

	Village of Wellsville	Date:	1/30	/201	7	HUN	T Project Number:	1861	.034		
	•	Design Stage:	PER				Calculated By:				
	Wellsville, NY	Revision:					Checked By:	TKS			
Item No.	Item	Quantity	Unit		-	COST	S		Subtotals		
nom No.		Quantity	Offic		\$/Unit		Total		Oubtotals		
	DIV 1 - GENERAL CONDITIONS							\$	9,050.46		
1	Bonding (2.5%)	1	LS	\$	2,154.87	\$	2,154.87				
2	Supervision (2%)	1	LS	\$	1,723.90	\$	1,723.90				
3	Mobilization (3%)	1	LS	\$	2,585.84	\$	2,585.84				
4	Maintenance and Protection of Traffic (2.5%)	1	LS	\$	2,154.87	\$	2,154.87				
5	Temporary Facilities (0.5%)	1	LS	\$	430.97	\$	430.97				
	EROSION AND SEDIMENT CONTROL							\$	250.00		
6	Inlet Protection	2	EΑ	\$	125.00	\$	250.00				
	EARTHWORK							\$	1,981.48		
7	Import and Place Topsoil	37	CY	\$	40.00	\$	1,481.48				
8	Fine Grade Topsoil	2000	SF	\$	0.25	\$	500.00				
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	41,673.33		
9	Heavy Duty Asphalt Pavement with Subbase	473	SY	\$	52.00	\$	24,613.33				
10	Concrete Sidewalk with Subbase	350	SF	\$	40.00	\$	14,000.00				
11	Granite Curb	70	LF	\$	38.00	\$	2,660.00				
12	Seeding & Mulching	2000	SF	\$	0.20	\$	400.00				
	WATER DISTRIBUTION							\$	42,290.00		
13	Connection to Existing System	2	EA	\$	2,000.00	\$	4,000.00				
14	6" Ductile Iron Watermain	500	LF	\$	55.00	\$	27,500.00				
15	6" Gate Valve and Box	1	EA	\$	1,600.00	\$	1,600.00				
16	6" Elbow/Tee	2	EA	\$	695.00	\$	1,390.00				
17	1-1/2" Water Service Direct Bury	264	LF	\$	20.00	\$	5,280.00				
18	Coupler to Tie onto Existing Water Service	7	EA	\$	50.00	\$	350.00				
19	Curb Stop and Box	7	EA	\$	310.00	\$	2,170.00				
	Construction Contingency (20%)	1	LS	\$	19,049.05	\$	19,049.05	\$	19,049.05		
	TOTAL CONSTRUCTION COST							\$	114,294.32		

Opinion of Probable Cost Replace Asbestos Lines at Meadowbrook Ct

	moddow brook ot								
	Village of Wellsville	Date:	1/5/2	2017	7	HUN	T Project Number:	1861	.034
	•	Design Stage:	PER				Calculated By:	AEV	
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST			Subtotals
item No.		Quantity	Offic		\$/Unit		Total		Oubtotals
	DIV 1 - GENERAL CONDITIONS							\$	11,956.99
1	Bonding (2.5%)	1	LS	\$	3,146.58	\$	3,146.58		
2	Supervision (2%)	1	LS	\$	2,517.26	\$	2,517.26		
3	Mobilization (3%)	1	LS	\$	3,775.89	\$	3,775.89		
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	1,887.95	\$	1,887.95		
5	Temporary Facilities (0.5%)	1	LS	\$	629.32	\$	629.32		
	EROSION AND SEDIMENT CONTROL							\$	500.00
6	Inlet Protection	4	EΑ	\$	125.00	\$	500.00		
	EARTHWORK							\$	2,774.07
7	Import and Place Topsoil	52	CY	\$	40.00	\$	2,074.07		
8	Fine Grade Topsoil	2800	SF	\$	0.25	\$	700.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	52,824.00
9	Heavy Duty Asphalt Pavement with Subbase	802	SY	\$	52.00	\$	41,704.00		
10	Concrete Sidewalk with Subbase	600	SF	\$	10.00	\$	6,000.00		
11	Granite Curb	120	LF	\$	38.00	\$	4,560.00		
12	Seeding & Mulching	2800	SF	\$	0.20	\$	560.00		
	WATER DISTRIBUTION							\$	69,765.00
13	Connection to Existing System	1	EΑ	\$	2,000.00	\$	2,000.00		
14	6" Ductile Iron Watermain	783	LF	\$	55.00	\$	43,065.00		
15	6" Gate Valve and Box	1	EΑ	\$	1,600.00	\$	1,600.00		
16	6" Elbow/Tee	4	EΑ	\$	695.00	\$	2,780.00		
17	Fire Hydrant Unit	2	EΑ	\$	5,000.00	\$	10,000.00		
18	1-1/2" Water Service Direct Bury	300	LF	\$	20.00	\$	6,000.00		
19	Coupler to Tie onto Existing Water Service	12	EA	\$	50.00	\$	600.00		
20	Curb Stop and Box	12	EA	\$	310.00	\$	3,720.00		
	Construction Contingency (20%)	1	LS	\$	27,564.01	\$	27,564.01	\$	27,564.01
	TOTAL CONSTRUCTION COST							\$	165,384.08

Opinion of Probable Cost Replace Asbestos Lines at Witter Ave.

	Ave.								
	Village of Wellsville	Date:	1/5/2	2017	•	HUN	T Project Number:	1861	.034
	_	Design Stage:	PER				Calculated By:	AEV	
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit		(COST	S		Subtotals
item No.	item	Quantity	Offic		\$/Unit		Total		Subtotals
	DIV 1 - GENERAL CONDITIONS							\$	8,077.80
1	Bonding (2.5%)	1	LS	\$	2,125.74	\$	2,125.74		
2	Supervision (2%)	1	LS	\$	1,700.59	\$	1,700.59		
3	Mobilization (3%)	1	LS	\$	2,550.88	\$	2,550.88		
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	1,275.44	\$	1,275.44		
5	Temporary Facilities (0.5%)	1	LS	\$	425.15	\$	425.15		
	EROSION AND SEDIMENT CONTROL							\$	500.00
6	Inlet Protection	4	EΑ	\$	125.00	\$	500.00		
	EARTHWORK							\$	2,377.78
7	Import and Place Topsoil	44	CY	\$	40.00	\$	1,777.78		
8	Fine Grade Topsoil	2400	SF	\$	0.25	\$	600.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	35,026.67
9	Heavy Duty Asphalt Pavement with Subbase	577	SY	\$	52.00	\$	29,986.67		
10	Granite Curb	120	LF	\$	38.00	\$	4,560.00		
11	Seeding & Mulching	2400	SF	\$	0.20	\$	480.00		
	WATER DISTRIBUTION							\$	47,125.00
12	Connection to Existing System	1	EΑ	\$	2,000.00	\$	2,000.00		
13	6" Ductile Iron Watermain	505	LF	\$	55.00	\$	27,775.00		
14	6" Gate Valve and Box	1	EΑ	\$	1,600.00	\$	1,600.00		
15	6" Elbow/Tee	2	EΑ	\$	695.00	\$	1,390.00		
16	Fire Hydrant Unit	1	EΑ	\$	5,000.00	\$	5,000.00		
17	1-1/2" Water Service Direct Bury	270	LF	\$	20.00	\$	5,400.00		
18	Coupler to Tie onto Existing Water Service	11	EA	\$	50.00	\$	550.00		
19	Curb Stop and Box	11	EA	\$	310.00	\$	3,410.00		
	Construction Contingency (20%)	1	LS	\$	18,621.45	\$	18,621.45	\$	18,621.45
	TOTAL CONSTRUCTION COST							\$	111,728.69

Opinion of Probable Cost Replace Asbestos Line at North End SR 19 and at Tracks



	End SR 19 and at Tracks						•		
	Town of Wellsville	Date:	1/5/2	2017	,	HUN	T Project Number:	1861.	.034
		Design Stage:	PER				Calculated By:	AEV	
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST	S		Subtotals
item No.		Quantity	Offic		\$/Unit		Total		Subtotals
	DIV 1 - GENERAL CONDITIONS							\$	63,513.22
1	Bonding (2.5%)	1	LS	\$	16,714.00	\$	16,714.00		
2	Supervision (2%)	1	LS	\$	13,371.20	\$	13,371.20		
3	Mobilization (3%)	1	LS	\$	20,056.81	\$	20,056.81		
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	10,028.40	\$	10,028.40		
5	Temporary Facilities (0.5%)	1	LS	\$	3,342.80	\$	3,342.80		
	EROSION AND SEDIMENT CONTROL							\$	2,500.00
6	Inlet Protection	20	EA	\$	125.00	\$	2,500.00		
	EARTHWORK							\$	8,718.52
7	Import and Place Topsoil	163	CY	\$	40.00	\$	6,518.52		
8	Fine Grade Topsoil	8800	SF	\$	0.25	\$	2,200.00		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	193,326.67
9	Heavy Duty Asphalt Pavement with Subbase	2567	SY	\$	52.00	\$	133,466.67		
10	Concrete Gutter with Subbase	390	LF	\$	40.00	\$	15,600.00		
11	Concrete Sidewalk with Subbase	4250	SF	\$	10.00	\$	42,500.00		
12	Seeding & Mulching	8800	SF	\$	0.20	\$	1,760.00		
	WATER DISTRIBUTION							\$	464,015.00
13	Connection to Existing System	2	EA	\$	2,000.00	\$	4,000.00		
14	10" Ductile Iron Watermain	5100	LF	\$	70.00	\$	357,000.00		
15	10" Gate Valve and Box	2	EA	\$	3,500.00	\$	7,000.00		
16	10" Elbow/Tee	5	EΑ	\$	1,295.00	\$	6,475.00		
17	Fire Hydrant Unit	7	EA	\$	5,000.00	\$	35,000.00		
18	1-1/2" Water Service Direct Bury	1110	LF	\$	20.00	\$	22,200.00		
19	1-1/2" Water Service (For HDD)	660	LF	\$	25.00	\$	16,500.00		
20	Coupler to Tie onto Existing Water Service	44	EA	\$	50.00	\$	2,200.00		
21	Curb Stop and Box	44		\$	310.00	\$	13,640.00		
	Construction Contingency (20%)	1	LS	\$	146,414.68	\$	146,414.68	\$	146,414.68
	TOTAL CONSTRUCTION COST							\$	878,488.08

Opinion of Probable Cost 8" Connect - Highland to Florida

	Village of Wellsville	Date	1/5/2	2017	7	HUN	T Project Number:	1861	.034
	•	Design Stage:					Calculated By:		
	Wellsville, NY	Revision					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST			Subtotals
itom ito.		Quantity	Onne		\$/Unit		Total		Captotals
	DIV 1 - GENERAL CONDITIONS							\$	9,042.40
1	Bonding (2.5%)	1	LS	\$	2,055.09	\$	2,055.09		
2	Supervision (2%)	1	LS	\$	1,644.07	\$	1,644.07		
3	Mobilization (3%)	1	LS	\$	2,466.11	\$	2,466.11		
4	Maintenance and Protection of Traffic (3%)	1	LS	\$	2,466.11	\$	2,466.11		
5	Temporary Facilities (0.5%)	1	LS	\$	411.02	\$	411.02		
	EROSION AND SEDIMENT CONTROL							\$	300.00
6	Silt Fence	100	LF	\$	3.00	\$	300.00		
	EARTHWORK							\$	7,709.72
8	Import and Place Topsoil	136	CY	\$	40.00	\$	5,422.22		
9	Fine Grade Topsoil	9150	SF	\$	0.25	\$	2,287.50		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	15,058.89
10	S-Duty Asphalt Pavement with Subbase	153	SY	\$	45.00	\$	6,900.00		
11	Heavy Duty Asphalt Pavement with Subbase	56	SY	\$	52.00	\$	2,888.89		
12	Concrete Sidewalk with Subbase	100	SF	\$	10.00	\$	1,000.00		
13	Concrete Curb	20	LF	\$	22.00	\$	440.00		
14	Seeding & Mulching	9150	SF	\$	0.20	\$	1,830.00		
15	Large Plants	8	EA	\$	250.00	\$	2,000.00		
	WATER DISTRIBUTION							\$	59,135.00
16	Connection to Existing System	2	EA	\$	2,000.00	\$	4,000.00		
17	8" Ductile Iron Watermain	820	LF	\$	62.00	\$	50,840.00		
18	6" Gate Valve and Box	1	EΑ	\$	1,600.00	\$	1,600.00		
19	8" Gate Valve and Box	1	EΑ	\$	2,000.00	\$	2,000.00		
20	8" Elbow/Tee	1	EA	\$	695.00	\$	695.00		
	Construction Contingency (20%)	1	LS	\$	18,249.20	\$	18,249.20	\$	18,249.20
	TOTAL CONSTRUCTION COST							\$	109,495.21

Opinion of Probable Cost



6" (Connect Under Madison St. Overpass at RR Tracks				70	J			
	Village of Wellsville	Date:	1/5/2	2017		HUN	T Project Number:	1861	.034
	•	Design Stage:	PER				Calculated By:		
	Wellsville, NY	Revision:					Checked By:	TKS	
Item No.	Item	Quantity	Unit			COST	S		Subtotals
ROITI IVO.	Rem	Quantity	Offic		\$/Unit		Total		- Cubiolais
	DIV 1 - GENERAL CONDITIONS							\$	15,786.67
1	Bonding (2.5%)	1	LS	\$	2,466.67	\$	2,466.67		
2	Supervision (2%)	1	LS	\$	1,973.33	\$	1,973.33		
3	Mobilization (3%)	1	LS	\$	2,960.00	\$	2,960.00		
4	Maintenance and Protection of Traffic (8%)	1	LS	\$	7,893.33	\$	7,893.33		
5	Temporary Facilities (0.5%)	1	LS	\$	493.33	\$	493.33		
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	4,666.67
6	Crusher Run Stone Surfacing	167	SY	\$	28.00	\$	4,666.67		
	WATER DISTRIBUTION							\$	94,000.00
7	Connection to Existing System	2	EΑ	\$	2,000.00	\$	4,000.00		
8	6" Watermain (For HDD)	360	LF	\$	250.00	\$	90,000.00		
	Construction Contingency (20%)	1	LS	\$	22,890.67	\$	22,890.67	\$	22,890.67
	TOTAL CONSTRUCTION COST							\$	137,344.00

Opinion of Probable Cost 6" Connect at S Broad St. Creek Crossing



Village of Wellsville Wellsville, NY	Date: 1/5/2017	HUNT Project Number: 1861.034				
	Design Stage: PER	Calculated By: AEV				
	Revision:	Checked By: TKS				

		110101011.				Checked By.	
Item No.	Item	Quantity	Unit		COSTS		Subtotals
1.0111 140.		Quartity		\$/Unit		Total	Odbiolais
	DIV 1 - GENERAL CONDITIONS						\$ 4,499.29
1	Bonding (2.5%)	1	LS	\$ 1,022.56	\$	1,022.56	
2	Supervision (2%)	1	LS	\$ 818.05	\$	818.05	
3	Mobilization (3%)	1	LS	\$ 1,227.08	\$	1,227.08	
4	Maintenance and Protection of Traffic (3%)	1	LS	\$ 1,227.08	\$	1,227.08	
5	Temporary Facilities (0.5%)	1	LS	\$ 204.51	\$	204.51	
	EROSION AND SEDIMENT CONTROL						\$ 600.00
6	Silt Fence	200	LF	\$ 3.00	\$	600.00	
	EARTHWORK						\$ 792.59
7	Import and Place Topsoil	15	CY	\$ 40.00	\$	592.59	
8	Fine Grade Topsoil	800	SF	\$ 0.25	\$	200.00	
	DIV 32 - EXTERIOR IMPROVEMENTS						\$ 4,760.00
9	Heavy Duty Asphalt Pavement with Subbase	80	SY	\$ 52.00	\$	4,160.00	
10	Concrete Curb	20	LF	\$ 22.00	\$	440.00	
11	Seeding & Mulching	800	SF	\$ 0.20	\$	160.00	
	WATER DISTRIBUTION						\$ 34,750.00
12	Connection to Existing System	2	EΑ	\$ 2,000.00	\$	4,000.00	
13	6" Watermain (For HDD)	205	LF	\$ 150.00	\$	30,750.00	
	Construction Contingency (20%)	1	LS	\$ 9,080.38	\$	9,080.38	\$ 9,080.38
	TOTAL CONSTRUCTION COST						\$ 54,482.25

Opinion of Probable Cost

Water Replacement Scott Ave

Village of Wellsville Wellsville, NY		Date: 1/5/2017				HUNT Project Number: 1861.034				
		Design Stage: PER			Calculated By: AEV					
		Revision:			Checked By: TKS					
Item No.	Item	Quantity	Unit			COST		Subtotals		
ileiii ivo.		Quantity	Oint		\$/Unit		Total		Subtotals	
	DIV 1 - GENERAL CONDITIONS							\$	31,208.85	
1	Bonding (2.5%)	1	LS	\$	•	\$	8,212.86			
2	Supervision (2%)	1	LS	\$	-,	\$	6,570.29			
3	Mobilization (3%)	1	LS	\$	9,855.43	\$	9,855.43			
4	Maintenance and Protection of Traffic (1.5%)	1	LS	\$	4,927.71	\$	4,927.71			
5	Temporary Facilities (0.5%)	1	LS	\$	1,642.57	\$	1,642.57			
	EROSION AND SEDIMENT CONTROL	1						\$	2,000.00	
6	Inlet Protection	16	EA	\$	125.00	\$	2,000.00		·	
	EARTHWORK	1						\$	6,142.59	
7	Import and Place Topsoil	115	CY	\$	40.00	\$	4,592.59		•	
8	Fine Grade Topsoil	6200	SF	\$	0.25	\$	1,550.00			
	DIV 32 - EXTERIOR IMPROVEMENTS							\$	122,226.67	
9	Heavy Duty Asphalt Pavement with Subbase	1907	SY	\$	52.00	\$	99,146.67			
10	Concrete Sidewalk with Subbase	1120	SF	\$	10.00	\$	11,200.00			
11	Granite Curb	280	LF	\$	38.00	\$	10,640.00			
12	Seeding & Mulching	6200	SF	\$	0.20	\$	1,240.00			
	WATER DISTRIBUTION							\$	198,145.00	
13	Connection to Existing System	2	EA	\$	2,000.00	\$	4,000.00			
14	8" Ductile Iron Watermain	2260	LF	\$	65.00	\$	146,900.00			
15	8" Gate Valve and Box	2	EA	\$	2,000.00	\$	4,000.00			
16	8" Elbow/Tee	3	EA	\$	695.00	\$	2,085.00			
17	Fire Hydrant Unit	3	EA	\$	5,000.00	\$	15,000.00			
18	1-1/2" Water Service Direct Bury	804	LF	\$	20.00	\$	16,080.00			
19	Coupler to Tie onto Existing Water Service	28	EA	\$	50.00	\$	1,400.00			
20	Curb Stop and Box	28	EA	\$	310.00	\$	8,680.00			
	Construction Contingency (20%)	1	LS	\$	71,944.62	\$	71,944.62	\$	71,944.62	
	TOTAL CONSTRUCTION COST							\$	431,667.74	

Opinion of Probable Cost

Water Replacement E Dyke and Miller Connection

Village of Wellsville	Date: 1/5/2017	HUNT Project Number: 1861.034
•	Design Stage: PER	Calculated By: AEV
Wellsville, NY	Revision:	Checked By: TKS
		00070

	<u> </u>	11011010111					S. Solida Dy.	
Item No.	ltem	Quantity	Unit			COSTS		Subtotals
		Quantity	Offic		\$/Unit		Total	Subiolais
	DIV 1 - GENERAL CONDITIONS							\$ 12,538.02
1	Bonding (2.5%)	1	LS	\$	2,849.55	\$	2,849.55	
2	Supervision (2%)	1	LS	\$	2,279.64	\$	2,279.64	
3	Mobilization (3%)	1	LS	\$	3,419.46	\$	3,419.46	
4	Maintenance and Protection of Traffic (3%)	1	LS	\$	3,419.46	\$	3,419.46	
5	Temporary Facilities (0.5%)	1	LS	\$	569.91	\$	569.91	
	EROSION AND SEDIMENT CONTROL	8						\$ 800.00
6	Silt Fence	100	LF	\$	3.00	\$	300.00	
7	Inlet Protection	4	EA	\$	125.00	\$	500.00	
	EARTHWORK							\$ 1,387.04
8	Import and Place Topsoil	26	CY	\$	40.00	\$	1,037.04	
9	Fine Grade Topsoil	1400	SF	\$	0.25	\$	350.00	
	DIV 32 - EXTERIOR IMPROVEMENTS							\$ 31,020.00
10	Heavy Duty Asphalt Pavement with Subbase	540	SY	\$	52.00	\$	28,080.00	
11	Granite Curb	70	LF	\$	38.00	\$	2,660.00	
12	Seeding & Mulching	1400	SF	\$	0.20	\$	280.00	
	WATER DISTRIBUTION							\$ 80,775.00
13	Connection to Existing System	2	EA	\$	2,000.00	\$	4,000.00	
14	8" Ductile Iron Watermain	670	LF	\$	65.00	\$	43,550.00	
15	8" Watermain (For HDD)	100	LF	\$	150.00	\$	15,000.00	
16	8" Gate Valve and Box	2	EA	\$	2,000.00	\$	4,000.00	
17	8" Elbow/Tee	3	EA	\$	695.00	\$	2,085.00	
18	Fire Hydrant Unit	1	EA	\$	5,000.00	\$	5,000.00	
19	1-1/2" Water Service Direct Bury	231	LF	\$	20.00	\$	4,620.00	
20	Coupler to Tie onto Existing Water Service	7	EA	\$	50.00	\$	350.00	
21	Curb Stop and Box	7	EA	\$	310.00	\$	2,170.00	
	Construction Contingency (20%)	1	LS	\$	25,304.01	\$	25,304.01	\$ 25,304.01
	TOTAL CONSTRUCTION COST							\$ 151,824.07

APPENDIX Q
PROJECT IMPROVEMENTS MAP

